



1 **A data-driven method coupling multiple physical constraints**
2 **for correcting structural errors in groundwater contaminant**
3 **transport models**

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10



11 **Abstract**

12 Model structural errors are pervasive in groundwater contaminant transport
13 modeling under complex environmental conditions, hindering accurate prediction of
14 contamination transport. Data-driven methods (DDMs) coupled with physical
15 constraints provide an effective approach for correcting structural errors and improving
16 prediction. However, in multicomponent reactive transport systems, multiple physical
17 mechanisms must be satisfied simultaneously, whereas existing DDMs have limited
18 capacity to effectively couple multiple physical constraints. To address this challenge,
19 this study proposes a general method for correcting structural errors in groundwater
20 models. A combined likelihood function is constructed and sub-likelihood weights are
21 dynamically updated to effectively couple multiple physical constraints. The method is
22 evaluated using a synthetic three-dimensional tetrachloroethylene reactive transport
23 simulation and a cadmium-phosphate cotransport sand column experiment. These tests
24 systematically assess the effects of coupling single versus multiple physical constraints
25 on structural error correction and predictive performance. The results show that
26 coupling multiple constraints can constrain parameter identification, reduce predictive
27 uncertainty, and more comprehensively improve model predictions. Appropriate
28 physical constraints function analogously to incorporating additional observations.
29 Moreover, coupling multiple physical constraints results in a simpler form of structural
30 error in the calibrated groundwater model, making it easier to characterize, thereby
31 enhancing prediction accuracy and physical consistency. The proposed dynamic
32 updating and stopping criterion of sub-likelihood weights maintains a balance between
33 multiple physical constraints and observations, improving the robustness of parameter
34 identification and constraint enforcement. Overall, the proposed DDM coupled with
35 multiple physical constraints provides a general framework for correcting structural
36 errors in complex groundwater contaminant transport models.

37 **Keywords:** Gaussian process regression; Model structural error; Multiple physical
38 constraints; Mass conservation



39 1. Introduction

40 Numerical groundwater models are essential tools for predicting contaminant
41 transport processes in groundwater and play a critical role in risk assessment and
42 remediation decision-making at contaminated sites (Kim et al., 2026; Wen et al., 2025).
43 With increasing demands for efficient and sustainable remediation of groundwater
44 contamination, improving the predictive accuracy of complex migration and
45 transformation processes of contaminants in aquifers, together with conducting reliable
46 uncertainty analysis, has become imperative (Zeeshan and Ruhl, 2023). Although
47 extensive studies have focused on parameter uncertainty (Hendrikx et al., 2026;
48 Engdahl, 2025), comparatively limited attention has been devoted to model structural
49 error. Owing to the complexity of groundwater systems and limitations in human
50 understanding, constructed groundwater models are often simplified or even
51 misspecified representations of real systems, resulting in the widespread presence of
52 structural error (Refsgaard et al., 2006; Neuman and Wierenga, 2003). Such structural
53 deficiencies lead to systematic errors in model predictions (Watson et al., 2013; Ye et
54 al., 2004), thereby reducing the reliability of prediction-informed decisions (Sun et al.,
55 2021; Gupta et al., 2012).

56 In recent years, data-driven methods (DDMs) have been widely employed to
57 correct model structural errors and enhance the predictive performance of physics-
58 based models (Yu et al., 2022; Hu et al., 2021; Kasiviswanathan et al., 2019; Pathiraja
59 et al., 2018). These approaches construct an error-correcting complementary model to
60 explicitly account for systematic discrepancies in model predictions, such that the
61 corrected predictions consist of the physical model, the structural error model, and the
62 measurement error (Xu et al., 2017; Xu and Valocchi, 2015). Previous studies have
63 demonstrated that applying DDMs to correct structural error can substantially improve
64 the predictive accuracy of groundwater head, soil moisture, and contaminant
65 concentration (Pan et al., 2023; Sun et al., 2021; Zhang et al., 2019). However, the error-
66 correction models constructed by these approaches remain essentially black-box
67 statistical models that lack necessary physical constraints. Consequently, they may



68 yield predictions that violate fundamental physical principles, such as contaminant
69 concentrations exceeding physically plausible ranges or violating mass conservation,
70 thereby weakening the plausibility and reliability of the predictions.

71 Incorporating physical constraints into DDMs provides an effective means of
72 enhancing the physical consistency of predictions. Previous studies have parameterized
73 physical constraints and formulated them as constraint conditions in optimization
74 problems to achieve coupling of physical mechanisms (Ayensa-Jiménez et al., 2018;
75 He and Xiu, 2016). However, when multiple physical constraints are considered
76 simultaneously, the number of unknown parameters may exceed the number of physical
77 constraints, thereby increasing predictive uncertainty. Wu et al. (2024) incorporated the
78 energy conservation constraint into parameter identification by jointly considering the
79 accumulated violation of the energy conservation constraint and prediction error.
80 Nevertheless, under multiple physical constraints, different physical constraints may
81 compete with one another, making it difficult for predictions to simultaneously satisfy
82 all physical mechanisms. In the groundwater field, Xu and Valocchi (2015) coupled
83 model predictions with the water balance equation through recalibration. However,
84 imposing multiple physical constraints through recalibration may drive the physical
85 model parameters to overfit in order to satisfy all constraints and result in numerical
86 instability or infeasible solutions, thereby limiting the applicability of the approach.
87 Overall, although existing methods can couple a single physical constraint, there
88 remains a lack of a general method capable of simultaneously and effectively coupling
89 multiple physical constraints.

90 The rapid development of physics-informed machine learning (PIML) offers a
91 potential pathway for coupling multiple physical constraints. PIML typically
92 incorporates physical mechanisms at three stages: during data preparation, by selecting
93 or augmenting training data based on physical constraints (Melching et al., 2023; Zhang
94 et al., 2021); during model construction, by introducing physics-constrained layers
95 (Mendenhall et al., 2024; Saha et al., 2021); and during model training, by designing
96 specific loss functions or inference algorithms to enforce physical constraints (Zhang



97 et al., 2023; Guo et al., 2022; Zhang et al., 2022). To date, PIML has been widely
98 applied in hydrology, engineering, and materials science (Cao and Weng, 2024; Cooper
99 et al., 2023; Lu et al., 2022), demonstrating its potential to integrate multiple physical
100 constraints simultaneously, such as mass balance, monotonicity, and boundary
101 conditions. However, existing PIML approaches still face limitations in correcting
102 model structural errors. This is primarily because PIML typically constrains only the
103 error-correction model itself rather than the entire error-correction process. As a result,
104 the corrected predictions, which consist of the physical model, the structural error
105 model, and measurement error, may still violate fundamental physical principles.
106 Moreover, existing PIML approaches lack effective strategies for selecting appropriate
107 physical constraints tailored to model structural errors, further limiting their potential
108 to enhance predictive reliability and physical consistency.

109 By constructing a combined likelihood function within a Bayesian uncertainty
110 analysis framework, Tian et al. (2026) successfully coupled Gaussian process
111 regression with a mass conservation constraint to correct structural errors in
112 groundwater models. However, in complex groundwater systems involving
113 multicomponent reactive transport processes, it is often necessary to simultaneously
114 constrain the physical mechanisms associated with multiple components. In such cases,
115 because different physical constraints represent distinct physical meanings, the
116 magnitudes of their corresponding likelihood functions may differ by several orders of
117 magnitude. Simply combining these likelihood functions may cause certain constraints
118 to dominate or be neglected, thereby hindering the effective coupling of multiple
119 physical constraints. Therefore, developing a general structural error correction method
120 capable of coupling multiple physical constraints is essential for improving the
121 accuracy and reliability of transport simulations in complex groundwater contamination
122 systems.

123 This study proposes a general physics-constrained DDM framework that
124 simultaneously embeds multiple physical constraints into the structural error correction
125 process. The constructed combined likelihood function consists of two components: a



126 conventional likelihood function that quantifies the fit between predictions and
127 observations, and a second component comprising multiple sub-likelihood functions
128 that measure the extent to which predictions satisfy individual physical constraints. In
129 addition, an adaptive weight-updating strategy with a stopping criterion is introduced.
130 During parameter identification, the sub-likelihood function weights are updated
131 according to their relative magnitudes, thereby achieving a reasonable balance between
132 data fitting and multiple physical constraints. The effectiveness of the proposed method
133 is evaluated using two groundwater contaminant transport simulation cases: a synthetic
134 three-dimensional tetrachloroethylene (PCE) reactive transport simulation and a
135 cadmium-phosphate cotransport sand column experiment. The results indicate that the
136 proposed physics-constrained DDM framework effectively couples multiple physical
137 constraints, significantly improving predictive performance of groundwater models and
138 providing a reliable basis for risk assessment and remediation decision-making at
139 complex contaminated sites.

140 The remainder of this paper is organized as follows. Section 2 presents the
141 Bayesian data-driven framework, the formulation for coupling multiple physical
142 constraints, and the weight-updating strategy with a stopping criterion. Sections 3 and
143 4 describe the configurations of the two case studies and the implementation of the
144 physics-constrained DDM framework, respectively. Section 5 presents and discusses
145 the results. Section 6 summarizes the main conclusions.

146 **2. Methods**

147 **2.1. Bayesian Data-driven Methods**

148 For a groundwater system, the observed data D can be expressed as (Kennedy and
149 O'Hagan, 2001):

$$150 \quad D = f(\theta) + b(x, \varphi) + \varepsilon \quad (1)$$

151 where f denotes the groundwater model and θ represents the model parameters. $b(x, \varphi)$
152 denotes the systematic prediction error described by Gaussian process regression (GPR),



153 where x and φ represent the inputs and hyperparameters of the GPR model, respectively.
154 ε denotes random measurement error.

155 In this study, Gaussian process regression (GPR) is employed to construct the
156 structural error correction model $b(x, \varphi)$. It is assumed that $b(x, \varphi)$ follows a
157 multivariate Gaussian distribution $N(\mu, C)$, which is fully characterized by the mean
158 function $\mu(x) = E(b(x))$ and the covariance function
159 $k(x, x') = E((b(x) - \mu(x))(b(x') - \mu(x')))$. The mean function μ is set to zero, and a
160 Matérn covariance kernel with smoothness parameter $\nu=5/2$ is adopted (Rasmussen and
161 Williams, 2005):

$$162 \quad C_{i,j} = k(x_i, x_j) = \sigma^2 \left(1 + \frac{\sqrt{5}(x_i - x_j)^T (x_i - x_j)}{\lambda} + \frac{5((x_i - x_j)^T (x_i - x_j))^2}{3\lambda^2} \right) \quad (2)$$
$$\quad \times \exp\left(-\frac{(x_i - x_j)^T (x_i - x_j)}{\lambda^2}\right) + \sigma_\varepsilon^2 I$$

163 where I denotes an indicator function that equals 1 if $i = j$ and 0 otherwise. The
164 hyperparameter φ consists of λ , σ^2 and σ_ε^2 , where λ denotes the characteristic
165 length scale, σ^2 represents the marginal variance of $b(x)$, and σ_ε^2 denotes the
166 variance of measurement error.

167 The groundwater model parameters θ and the GPR hyperparameters φ are
168 commonly assumed to be independent in the prior distribution. Accordingly, θ and
169 $\varphi(\lambda, \sigma^2, \sigma_\varepsilon^2)$ can be jointly calibrated (Xu and Valocchi, 2015; Xu et al., 2017; Yue
170 Pan et al., 2020). Based on Bayes' theorem, the posterior distribution can be expressed
171 as:

$$172 \quad p(\theta, \varphi | D) \propto p(D | \theta, \varphi) p(\theta) p(\varphi) \quad (3)$$

173 where $p(\theta)$ and $p(\varphi)$ denote the prior distributions of the physical parameters θ
174 and the hyperparameters φ , respectively, and $p(D | \theta, \varphi)$ represents the likelihood
175 function. In this study, a commonly used Gaussian likelihood function is adopted, and
176 its logarithmic form can be written as:



177
$$\log(p(D|\theta, \varphi)) = -\frac{1}{2}(D - f - \mu)^T \Sigma^{-1}(D - f - \mu) - \frac{1}{2} \log |\Sigma| - \frac{N}{2} \log 2\pi \quad (4)$$

178 where μ denotes the mean of the systematic prediction error, Σ represents the error
179 covariance matrix, and N is the number of observations.

180 Since the posterior distribution $p(\theta, \varphi|D)$ is generally intractable to evaluate
181 analytically, Markov chain Monte Carlo (MCMC) simulation is employed to generate
182 posterior samples of the physical parameters θ and the GPR hyperparameters φ . In
183 this study, the Differential Evolution Adaptive Metropolis (DREAMzs) algorithm is
184 adopted for MCMC sampling, which has been widely applied in groundwater inverse
185 modeling and uncertainty analysis.

186 Based on the posterior samples of θ and φ obtained from MCMC simulation,
187 predictions of the physical model f and the structural error model b can be generated for
188 a new input x^* , denoted as $f^*(\theta)$ and $b^*(x^*, \varphi)$, respectively. The latter can be
189 expressed as:

190
$$b^* | D - f, D, D^* \sim N(\overline{b^*}, C_b(b^*)) \quad (5)$$

191 where,

192
$$b^* = \mu^* + C^{*T} C^{-1} (D - f - \mu) \quad (6)$$

193
$$C_b(b^*) = C^{**} - C^{*T} C^{-1} C^* \quad (7)$$

194 where $\overline{b^*}$ denotes the posterior mean, $C_b(b^*)$ represents the posterior covariance
195 matrix, and μ^* denotes the prior mean $\mu(x^*, \varphi)$. The prior covariance matrices C^*
196 and C^{**} are defined as $C_{i,j}^* = f(x_i, x_j^*)$ and $C_{i,j}^{**} = f(x_i^*, x_j^*)$, respectively.

197 The final corrected prediction D^* can be expressed as:

198
$$D^* = f^* + b^* + \varepsilon^* \quad (8)$$

199 where f^* , b^* and ε^* denote the predictions of the groundwater model, the
200 structural error model, and the measurement error at x^* , respectively.



201 **2.2. Likelihood Function-based Coupling of Physical Constraints**

202 In practical applications, predictions of groundwater contaminant transport
203 simulations are typically required to satisfy multiple key physical constraints, such as
204 contaminant mass conservation and non-negativity of concentration. In this study, each
205 physical constraint is represented by a set of functions:

206
$$G(y) = [g_1(y), g_2(y), \dots, g_{n_c}(y)]^T = [0, 0, \dots, 0]^T \quad (9)$$

207
$$Z(y) = [z_1(y), z_2(y), \dots, z_{n_f}(y)]^T = [0, 0, \dots, 0]^T \quad (10)$$

208 where y denotes the corrected prediction (i.e., the sum of the physical model, the
209 structural error model, and the measurement error), which is a function of the
210 parameters θ and φ , i.e., $y = F(\theta, \varphi)$. $g_i(y), i = 1, 2, \dots, n_c$ represent the equality
211 constraints, where n_c denotes the number of equality constraints. $z_i(y), i = 1, 2, \dots, n_f$
212 represent the inequality constraints, where n_f denotes the number of inequality
213 constraints. The zero matrix on the right-hand side indicates that the corrected
214 predictions are required to satisfy the corresponding physical constraints at each
215 observation time.

216 The equality constraints are assumed to follow a zero-mean Gaussian distribution,
217 and Eq. (9) can therefore be expressed as:

218
$$G(y) = G(F(\theta, \varphi)) \sim N(0, \Sigma_c) \quad (11)$$

219 where Σ_c is a diagonal covariance matrix used to control the strictness of constraints
220 (Wu et al., 2019).

221 Therefore, a likelihood function can be constructed to quantify the extent to which
222 the corrected predictions satisfy the equality constraints, conditional on the parameters
223 θ and φ :

224
$$p(G(y) = 0 | \theta, \varphi) = \frac{1}{\sqrt{(2\pi)^{n_c} |\Sigma_c|}} \exp\left(-\frac{1}{2} G(F(\theta, \varphi))^T \Sigma_c^{-1} G(F(\theta, \varphi))\right) \quad (12)$$

225 For the inequality constraints, Eq. (10) can be transformed into the corresponding
226 equality constraints as:



227
$$\min(0, Z(y)) = 0 \tag{13}$$

228 Similarly, under the assumption of a zero-mean Gaussian distribution, Eq. (13) can
229 be written as:

230
$$\min(0, Z(y)) \sim N(0, \Sigma_f) \tag{14}$$

231 The corresponding likelihood function can be constructed as:

232
$$p(Z(y) \geq 0 | \theta, \varphi) = \frac{1}{\sqrt{(2\pi)^{n_f} |\Sigma_f|}} \exp\left(-\frac{1}{2} [\min(0, Z(y))]^T \Sigma_f^{-1} \min(0, Z(y))\right) \tag{15}$$

233 Accordingly, the posterior distribution conditional on both the observational data
234 and multiple physical constraints (including the equality and inequality constraints
235 described above) can be expressed as:

236
$$\begin{aligned} p(\theta, \varphi | D, G(y) = 0, Z(y) \geq 0) &\propto p(D, G(y) = 0, Z(y) \geq 0 | \theta, \varphi) p(\theta) p(\varphi) \\ &= p(D | \theta, \varphi) p(G(y) = 0 | \theta, \varphi) p(Z(y) \geq 0 | \theta, \varphi) p(\theta) p(\varphi) \end{aligned} \tag{16}$$

237 where $p(D | \theta, \varphi)$ corresponds to the likelihood function that quantifies the fit between
238 the predictions and the observational data, as given in Eq. (4), $p(G(y) = 0 | \theta, \varphi)$ and
239 $p(Z(y) \geq 0 | \theta, \varphi)$ correspond to the likelihood functions incorporating the equality and
240 inequality constraints defined in Eqs. (12) and (15), respectively.

241 This formulation can be further generalized, and Eq. (16) is equally applicable
242 when additional physical constraints are incorporated. In this case, the total log-
243 likelihood function can be expressed as the sum of the log-likelihoods of the individual
244 sub-likelihood functions:

245
$$L_T = L_1 + L_2 + L_3 \dots + L_n \tag{17}$$

246 where L_T denotes the total likelihood, L_1 represents the likelihood associated with
247 the predictions, and L_2, L_3, \dots, L_n denote the likelihood functions corresponding to
248 different physical constraints.

249 Therefore, MCMC sampling is performed based on the combined likelihood
250 function (Eq. 17) to jointly identify the posterior distributions of the groundwater model
251 parameters θ and the GPR hyperparameters φ .



2.3. Dynamic Weight-updating Strategy with a Stopping Criterion

Because different sub-likelihood functions correspond to distinct physical meanings, their magnitudes may differ substantially, leading to unequal competition among the sub-likelihood functions. Therefore, it is necessary to assign weights to the sub-likelihood functions and dynamically update them according to their relative magnitudes. In this study, an inverse-magnitude weighting method is adopted to weight the sub-likelihood functions as follows (Guger et al., 2020; Li et al., 2018):

$$L_{T,i} = w_{1,i} \cdot L_{1,i} + w_{2,i} \cdot L_{2,i} + \dots + w_{n,i} \cdot L_{n,i} \quad (18)$$

where $L_{1,i}, L_{2,i}, \dots, L_{n,i}$ denote the n sub-likelihood functions obtained at the i -th iteration of the Bayesian updating process under the scenario with multiple physical constraints, and $w_{1,i}, w_{2,i}, \dots, w_{n,i}$ denote the corresponding weights. The dynamic weight-updating procedure is described as follows:

First, the mean of each sub-likelihood function is calculated as follows:

$$L_{avg,i} = \frac{\sum_{j=1}^n L_{j,i}}{n} \quad (19)$$

Then, the initial unnormalized weights are computed:

$$w'_{j,i} = \frac{L_{avg,i}}{L_{j,i}}, j = 1, 2, \dots, n \quad (20)$$

Finally, the weights are normalized:

$$w_{j,i} = \frac{w'_{j,i}}{\sum_{k=1}^n w'_{k,i}}, j = 1, 2, \dots, n \quad (21)$$

During the dynamic weight-updating process, determining when to terminate weight updating is crucial for ensuring the stability and convergence of MCMC sampling. In this study, a stopping criterion derived from multi-objective evolutionary algorithms is introduced (Abu Doush et al., 2023; Kadhar and Baskar, 2018), whereby weight updating is terminated when the variance of weight changes remains stable within a specified iteration window. Because the MCMC simulation involves multiple chains, the stopping criterion must account for weight updates across different Markov



277 chains. The procedure is as follows:

278 (1) After the i -th iteration, compute the weight $w_{j,i}^r$ of the j -th sub-likelihood
279 function for the r -th chain.

280 (2) Compute the change in each weight between two consecutive iterations
281 $\Delta w_{j,i}^r = |w_{j,i}^r - w_{j,i-1}^r|$.

282 (3) Over the most recent γ iterations, compute the variance of the weight changes
283 for each sub-likelihood function and each chain as
284 $V_{j,\gamma}^r = \text{Variance}(\Delta w_{j,i-\gamma+1}^r, \Delta w_{j,i-\gamma+2}^r, \dots, \Delta w_{j,i}^r)$. The maximum value $MV_{j,\gamma}^{r*}$ is then
285 recorded, and the corresponding weight change is denoted as $M\Delta w_{j,i}^{r*}$, which is then
286 used in the subsequent χ^2 test.

287 (4) A χ^2 test is conducted to statistically assess whether $M\Delta w_{j,i}^{r*}$ over the
288 recent γ iterations is smaller than the prescribed tolerance variance (Tol_{fun}). The test
289 statistic is given by:

$$290 \quad \chi(M\Delta w_{j,i}^{r*}) = \frac{\text{Variance}(\Delta w_{j,i-\gamma+1}^{r*}, \Delta w_{j,i-\gamma+2}^{r*}, \dots, \Delta w_{j,i}^{r*})(\gamma-1)}{Tol_{fun}^2} \quad (22)$$

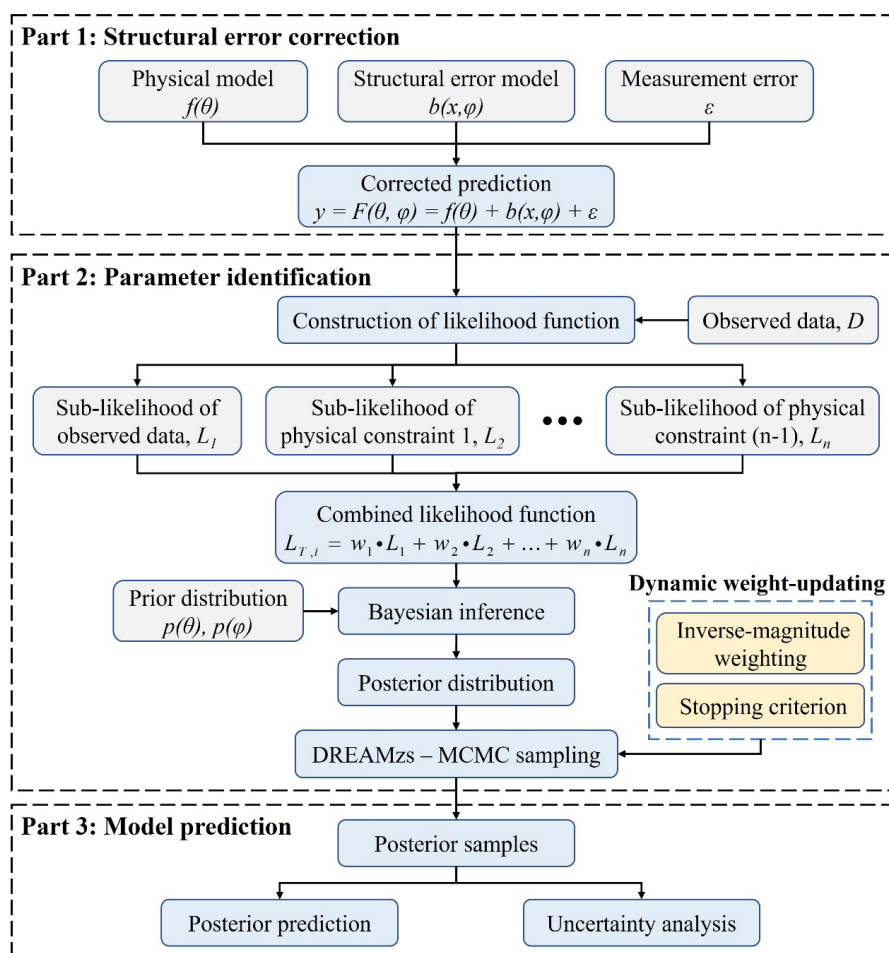
291 The corresponding probability is:

$$292 \quad P(M\Delta w_{j,i}^{r*}) = \chi^2 \text{test}[\chi(M\Delta w_{j,i}^{r*}), (\gamma-1)] \quad (23)$$

293 When $P(M\Delta w_{j,i}^{r*}) > 99\%$, the weight changes are considered statistically
294 insignificant, and the weight-updating process is terminated.

295 In summary, the overall workflow of the proposed framework for structural error
296 correction coupled with multiple physical constraints is illustrated in Fig. 1.

297



298

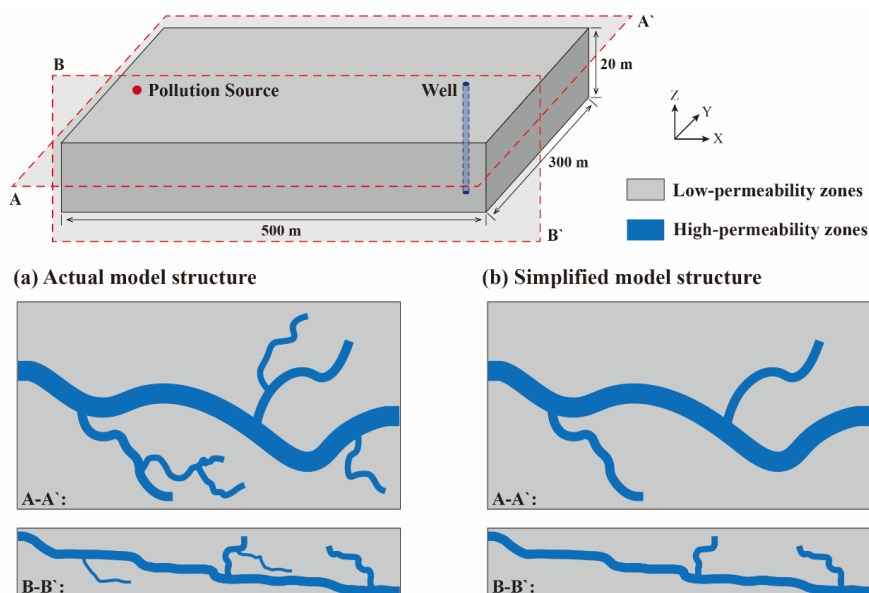
299 **Figure 1.** Overall workflow of the proposed framework.

300 **3. Descriptions of the Two Case Studies**

301 Two case studies were used in this study: A synthetic three-dimensional PCE
 302 reactive transport simulation and a cadmium–phosphate cotransport sand column
 303 experiment. In each case, the constructed groundwater model represents a structural
 304 simplification of the real groundwater system and therefore exhibits significant
 305 structural bias.



306 **3.1. Synthetic Groundwater PCE Reactive Transport Model**



307

308 **Figure 2.** Schematic illustration for the synthetic groundwater PCE reactive transport
309 model.

310 Case study 1 constructed a synthetic three-dimensional groundwater model to
311 simulate the reactive transport of PCE. As illustrated in Fig. 2, the aquifer is 500 m in
312 length, 300 m in width, and 20 m in thickness. The left and right boundaries are
313 specified-head boundaries corresponding to a hydraulic gradient of 0.12, whereas the
314 remaining boundaries are assigned as no-flow boundaries. The aquifer medium is
315 horizontally isotropic, with a vertical hydraulic conductivity anisotropy of 3 and a
316 porosity of 0.3. The overall permeability of the aquifer is relatively low, while dendritic
317 high-permeability preferential flow pathways are distributed within it (Fig. 2a).

318 A PCE leakage source is located near the left boundary at the top of the aquifer (x
319 = 75 m, y = 150 m, z = 20 m), with a release rate of 25 kg/d. During transport, PCE
320 undergoes sequential reductive dechlorination to form trichloroethylene (TCE),
321 dichloroethylene (DCE), and vinyl chloride (VC). A pumping well is located near the
322 right boundary at the bottom of the aquifer (x = 475 m, y = 150 m, z = 7 m) with a
323 pumping rate of 30 m³/d, where the concentrations of PCE, TCE, DCE, and VC are



324 continuously monitored.

325 In practice, detailed structural information of the aquifer is often unavailable, and
 326 simplification of the aquifer structure is therefore typically required. In particular, for
 327 preferential pathways with hierarchical branching structures, smaller branch channels
 328 are often neglected. In this case, only the main preferential pathways are retained (Fig.
 329 2b), and the PCE reactive transport model is constructed based on the simplified aquifer
 330 structure. The model is numerically solved using MODFLOW-2005 and RT3D.
 331 Gaussian process regression (GPR) is further employed to correct the structural error
 332 introduced by simplification of high-permeability preferential pathways.

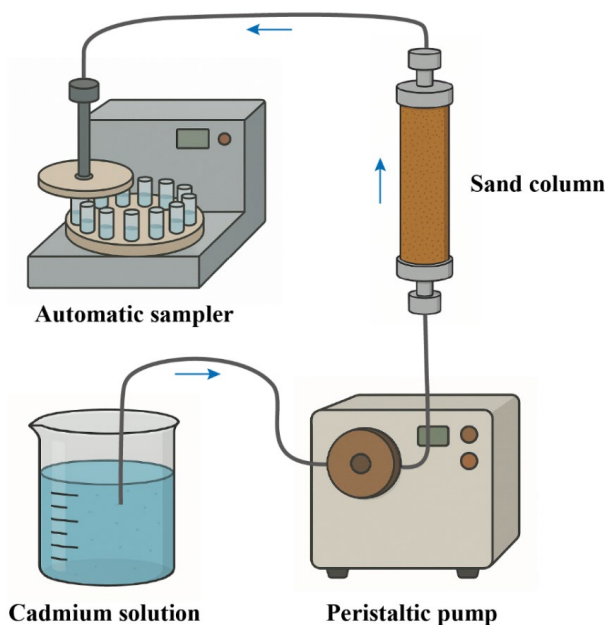
333 Except for the representation of the preferential pathways, the simplified model is
 334 identical to the actual model in all other respects. The total simulation period is 500
 335 days with a time step of 10 days. The parameter settings of the actual model are listed
 336 in Table 1. The contaminant concentrations simulated at the pumping well are treated
 337 as observational data. Gaussian white noise with a mean of 0 and a variance of 0.01 is
 338 added to the simulated concentrations, yielding a total of 50 concentration observations,
 339 of which the first 30 are used for parameter identification and the remaining 20 for
 340 model validation.

341 **Table 1.** Parameter configuration for the synthetic groundwater PCE reactive transport
 342 model with the actual model structure.

Parameters	Unit	Value
Porosity (ϕ)	-	0.3
Horizontal anisotropy of hydraulic conductivity (A_{Kh})	-	1.0
Vertical anisotropy of hydraulic conductivity (A_{Kv})	-	3.0
Horizontal anisotropy of dispersivity (A_{Dh})	-	0.3
Vertical anisotropy of dispersivity (A_{Dv})	-	0.3
Horizontal hydraulic conductivity of low-permeability zones (K_l)	$\text{m}\cdot\text{d}^{-1}$	1.5
Horizontal hydraulic conductivity of high-permeability zones (K_h)	$\text{m}\cdot\text{d}^{-1}$	8.0
Longitudinal dispersivity of low-permeability zones (D_l)	m	1.5
Longitudinal dispersivity of high-permeability zones (D_h)	m	8



343 **3.2. Sand Column Cadmium-phosphate Cotransport Model**



344

345 **Figure 3.** Schematic illustration of the sand column Cd-phosphate cotransport
346 experiment.

347 To investigate the influence of phosphate on cadmium (Cd) transport in natural
348 groundwater environments, a one-dimensional cadmium–phosphate cotransport sand
349 column experiment was conducted (Wu et al., 2022) (Fig. 3). A polypropylene column
350 with a height of 5 cm and an inner diameter of 1.5 cm was used. Stainless steel screens
351 with a pore size of 10 μm were placed at both ends of the column to support the porous
352 medium. The column was uniformly packed with dry soil, and the average porosity
353 (0.36) and bulk density (1.69 g/cm^3) were determined using the gravimetric method. A
354 peristaltic pump was used to drive the solution upward through the column at a flow
355 rate of 0.2 mL/min. After complete saturation, the column was flushed with background
356 solution for 12 h, followed by a tracer test to determine flow and solute transport
357 parameters. Once geochemical conditions stabilized, 420 mL of a mixed solution
358 containing 0.1 mM Cd and 1 mM phosphate was injected into the column using the



359 peristaltic pump. After completion of the mixed-solution injection, the influent was
360 switched to background solution without Cd. During the experiment, effluent samples
361 were collected every 90 min using an automatic fraction collector, and Cd
362 concentrations were measured, yielding a total of 58 concentration observations.

363 In real-world groundwater contamination scenarios, the spatial distribution of
364 phosphate is often uncertain and its influence on Cd transport is therefore commonly
365 neglected. Accordingly, case study 2 does not account for the cotransport mechanism
366 between Cd and phosphate. A one-dimensional groundwater model for Cd transport
367 was developed using HYDRUS-1D (Nkedikizza et al., 1984; Vengenuchten, 1980), in
368 which a two-site sorption model coupled with a Freundlich isotherm was employed to
369 describe Cd transport. The parameter settings of the physical model are listed in Table
370 2. In addition, Gaussian process regression (GPR) was employed to correct the
371 structural error arising from neglecting the Cd-phosphate cotransport mechanism. Of
372 the 58 observations, the first 32 were used for parameter identification and the
373 remaining 26 for validation.

374 **Table 2.** Parameter configuration of the sand column Cd-phosphate cotransport model.

Parameters	Unit	Value
Residual Water Content (θ_r)	$\text{cm}^3 \cdot \text{cm}^{-3}$	0.0
Saturated Water Content (θ_s)	$\text{cm}^3 \cdot \text{cm}^{-3}$	0.4
Saturated Hydraulic Conductivity (K_s)	$\text{cm} \cdot \text{min}^{-1}$	0.1132
Pore connectivity factor (P_L)	-	0.5
Bulk density (R_0)	$\text{g} \cdot \text{cm}^{-3}$	1.24
Longitudinal dispersivity (D_V)	cm	0.74

375 4. Application of the Methods

376 In each case study, contaminant transport simulations are first performed using a
377 groundwater model with structural error, after which the model is corrected within the
378 DDM framework. In addition, three scenarios are considered: without physical
379 constraints, with a single physical constraint, and with multiple physical constraints.



380 **4.1. DDM for Synthetic Groundwater PCE Reactive Transport Model**

381 As described in Section 3.1, case study 1 constructed a groundwater model with a
 382 simplified representation of preferential pathways in the aquifer to simulate PCE
 383 reactive transport. In this model, the hydraulic conductivities and dispersivities of the
 384 low- and high-permeability zones are treated as random variables with uniform prior
 385 distributions. Meanwhile, the GPR hyperparameters σ , λ and σ_ε are assigned exponential,
 386 gamma, and uniform priors, respectively. Table 3 summarizes the prior information for
 387 the physical parameters and the hyperparameters.

388 **Table 3.** Prior distributions of parameters for the synthetic groundwater PCE reactive
 389 transport model.

Parameter	Ranges	Type
K_l [m/d]	[1.0, 5.0]	Uniform
K_h [m/d]	[5.0, 12.0]	Uniform
D_l [m]	[1.0, 5.0]	Uniform
D_h [m]	[5.0, 12.0]	Uniform
σ	[0.001, 0.16]	Exponential
λ	[0.1, 3.0]	Gamma
σ_ε	[0.001, 0.12]	Uniform

390 In this case study, because carbon and chlorine are the key constituent elements of
 391 PCE and its dechlorination products, carbon and chlorine mass conservation constraints
 392 are incorporated into the DDM. Under the single-constraint scenario, only the carbon
 393 mass conservation constraint is coupled, whereas under the multiple-constraint scenario
 394 both carbon and chlorine mass conservation constraints are coupled simultaneously. For
 395 the entire system, the total mass of leaked carbon/chlorine is equal to the total
 396 carbon/chlorine mass contained in PCE, TCE, DCE, and VC. Moreover, at any time,
 397 the total mass of leaked carbon/chlorine m_l should equal the sum of three components:
 398 the mass remaining in the aquifer m_r , the mass leaving the aquifer through its
 399 boundaries m_b , and the mass extracted by the pumping well m_w . The mass balance
 400 error is defined as M_{error} , and the corresponding mass conservation constraints for
 401 carbon/chlorine can be expressed as:



402
$$M_{error} = m_l - (m_r + m_b + m_w) = 0 \quad (24)$$

403 Since the observation intervals are much larger than the simulation time step of
404 the physical model, the physical constraints at different observation times are assumed
405 to be independent and enforced with equal strength. The confidence on the accuracy of
406 each physical constraint ($\sigma_{c,i}^2$) is set to 0.2. Accordingly, the likelihood function
407 associated with the carbon/chlorine mass conservation constraints can be expressed as:

408
$$p(M_{error} = 0 | \theta, \varphi) = \frac{1}{\sqrt{(2\pi)^n |\Sigma_c|}} \exp\left(-\frac{1}{2} [m_l - (m_r + m_b + m_w)]^T \Sigma_c^{-1} [m_l - (m_r + m_b + m_w)]\right) \quad (25)$$

409 In addition, following Kadhar and Baskar (2018), the sliding window length used
410 for dynamic weight updating is set to $\gamma = 10$, and the minimum tolerance variance is
411 set to $Tol_fun = 10^{-4}$. The dynamic sub-likelihood weight-updating strategy with a
412 stopping criterion is implemented as described in Section 2.3. Three scenarios are
413 considered: without physical constraints, with only the carbon mass conservation
414 constraint, and with both carbon and chlorine mass conservation constraints.

415 After specifying the prior distributions and constructing the combined likelihood
416 function, MCMC simulations were conducted to infer the posterior distributions of the
417 parameters. Both unconstrained and constrained DDM configurations use the same
418 MCMC settings based on the DREAMzs algorithm, with three parallel Markov chains,
419 each running for 9000 iterations, and the first 3000 iterations designated as burn-in.
420 Finally, based on the identified posterior parameter distributions, the predictive
421 performance of the DDM for the concentrations of the four chlorinated hydrocarbons
422 was evaluated under the three scenarios.

423 **4.2. DDM for Sand Column Cd-phosphate Cotransport Model**

424 As described in Section 3.2, case study 2 developed a simplified Cd transport
425 model that does not consider the cadmium-phosphate cotransport mechanism. The
426 proportion coefficient of equilibrium sorption f , the Freundlich isotherm coefficient K_F ,



427 and the isotherm shape parameter β of the physical model are treated as random
 428 variables with uniform prior distributions. The GPR hyperparameters σ , λ and σ_ε are
 429 assigned exponential, gamma, and uniform prior distributions, respectively. The prior
 430 information for the physical model parameters and the GPR hyperparameters is
 431 summarized in Table 4.

432 **Table 4.** Prior distributions of parameters for the sand column Cd-phosphate
 433 cotransport model.

Parameter	Ranges	Type
f	[0.4, 0.6]	Uniform
K_F [cm ³ /mg]	[30.0, 40.0]	Uniform
β	[0.6, 1.2]	Uniform
σ	[0.01, 0.11]	Exponential
λ	[0.1, 1.0]	Gamma
σ_ε	[0.0001, 0.01]	Uniform

434 In this case study, Cd mass conservation and Cd concentration bound constraints
 435 are incorporated into the DDM. Under the single-constraint scenario, only the Cd mass
 436 conservation constraint is coupled. At any given time t_i , the total mass of Cd entering
 437 the column is assumed to be equal to the sum of the Cd masses present in different
 438 states. Specifically, before Cd breakthrough ($t_i \leq t_0$), the total mass consists of the
 439 dissolved-phase mass m_d and the mass adsorbed onto the solid phase m_a . After
 440 breakthrough ($t_i > t_0$), the total mass additionally includes the Cd mass in the effluent,
 441 m_{out} . Accordingly, the Cd mass conservation constraint is defined as:

$$442 \quad M_{error,t_i} = \begin{cases} m_{in,t_i} - (m_{d,t_i} + m_{a,t_i}) = 0, t_i \leq t_0 \\ m_{in,t_i} - (m_{d,t_i} + m_{a,t_i} + m_{out,t_i}) = 0, t_i > t_0 \end{cases} \quad (26)$$

443 where m_{in,t_i} denotes the total Cd mass entering the column; m_{d,t_i} , m_{a,t_i} and m_{out,t_i}
 444 denote the Cd mass in the dissolved phase, sorbed phase, and effluent, respectively; and
 445 t_0 represents the time of first Cd breakthrough.

446 Consistent with case study 1, the confidence on the accuracy of each physical
 447 constraint ($\sigma_{c,i}^2$) is set to 0.2, and the corresponding likelihood function can be written
 448 as:



$$\begin{aligned}
 & p(M_{error} = 0 | \theta, \varphi) \\
 449 \quad & = \begin{cases} \frac{1}{\sqrt{(2\pi)^{n_0} |\Sigma_c|}} \exp\left(-\frac{1}{2} [m_{in} - (m_d + m_a)]^T \Sigma_c^{-1} [m_{in} - (m_d + m_a)]\right) \\ \frac{1}{\sqrt{(2\pi)^{(n-n_0)} |\Sigma_c|}} \exp\left(-\frac{1}{2} [m_{in} - (m_d + m_a + m_{out})]^T \Sigma_c^{-1} [m_{in} - (m_d + m_a + m_{out})]\right) \end{cases} \quad (27)
 \end{aligned}$$

450 where n is the total number of observations and n_0 is the number of observations
 451 before Cd breakthrough.

452 Under the multiple-constraint scenario, the Cd concentration bound constraint is
 453 further coupled. The experimental breakthrough curve indicates that the upper bound
 454 of Cd concentration does not exceed 0.75 mg/cm³. Therefore, the Cd concentration
 455 bound constraint is defined as:

$$456 \quad 0 \leq C_{Cd} \leq 0.75 \quad (28)$$

457 This constraint is transformed into an equivalent equality constraint and assumed
 458 to follow a Gaussian distribution $N(0, \Sigma_f)$, which can be expressed as:

$$459 \quad Z(y) = [\max(0, -C_{Cd}) + \max(0, C_{Cd} - 0.75)] = 0 \quad (29)$$

460 Accordingly, the likelihood function associated with the Cd concentration bound
 461 constraint can be expressed as:

$$462 \quad p(Z(y) = 0 | \theta, \varphi) = \frac{1}{\sqrt{(2\pi)^n |\Sigma_f|}} \exp\left(-\frac{1}{2} [Z(y) = 0]^T \Sigma_f^{-1} [Z(y) = 0]\right) \quad (30)$$

463 As in case study 1, the sliding window length ($\gamma = 10$) and minimum tolerance
 464 variance ($Tol_fun = 10^{-4}$) are used for dynamic sub-likelihood weight updating and
 465 the stopping criterion. As in case study 1, three scenarios are considered: without
 466 physical constraints, with only the Cd mass conservation constraint, and with both the
 467 Cd mass conservation and Cd concentration bound constraints.

468 MCMC simulations were then conducted to infer the posterior distributions of the
 469 parameters. Under all three scenarios, identical MCMC settings based on the
 470 DREAMzs algorithm are employed, with three parallel Markov chains, each running
 471 for 9000 iterations and the first 3000 iterations designated as burn-in. Based on the



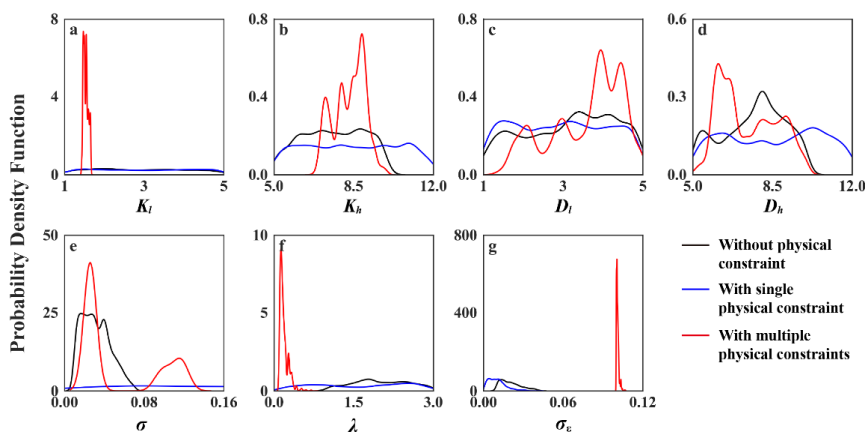
472 identified posterior parameter distributions, the predictive performance of the DDM for
473 Cd concentrations is evaluated under the three scenarios.

474 **5. Results and Discussions**

475 **5.1. Parameter Estimations**

476 **5.1.1. Synthetic Groundwater PCE Reactive Transport Model**

477 Fig. 4 presents the posterior distributions of the PCE reactive transport model
478 parameters under the three scenarios, where the first row shows the physical model
479 parameters and the second row showing the GPR hyperparameters. The results indicate
480 that both the presence and the number of coupled physical constraints significantly
481 influence the posterior distributions. Under the unconstrained and single-constraint
482 scenarios, the posterior distributions are generally similar, exhibiting relatively wide
483 ranges and lower peak probability densities, indicating greater parameter uncertainty.
484 In contrast, when multiple physical constraints are coupled simultaneously, the
485 posterior distributions contract substantially. Specifically, the posterior ranges become
486 substantially narrower and the peak densities increase markedly. Notably, the posterior
487 distributions of the physical model parameter K_l and the GPR hyperparameters σ , λ and
488 σ_ε show particularly pronounced changes in terms of range, mean value, and peak
489 density (Figs. 4a and 4e-4g). In addition, the physical model parameters K_h , L_l and L_h
490 exhibit more pronounced multimodal characteristics (Figs. 4b-4d), suggesting the
491 presence of multiple plausible parameter combinations.

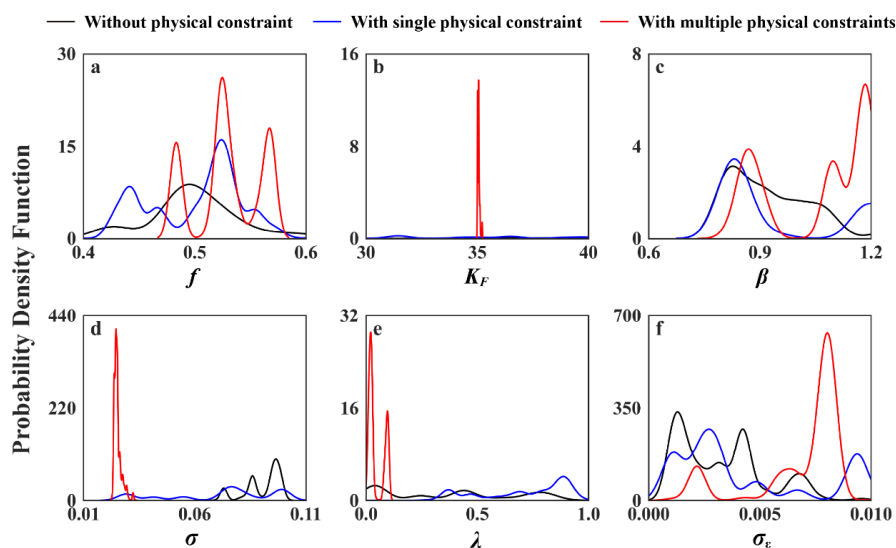


492

493 **Figure 4.** Parameter posterior distributions for the synthetic groundwater PCE reactive
 494 transport model under the three scenarios.

495 **5.1.2. Sand Column Cd-phosphate Cotransport Model**

496 Fig. 5 presents the posterior distributions of the parameters of the Cd-phosphate
 497 cotransport model under the three scenarios, where the first and second rows correspond
 498 to the physical model parameters and the GPR hyperparameters, respectively. Under
 499 the unconstrained and single-constraint scenarios, most parameters exhibit wide
 500 posterior ranges and low peak probability densities, indicating greater parameter
 501 uncertainty. Except for the relatively flat probability density curves for the physical
 502 model parameter K_F and the GPR hyperparameter λ (Figs. 5b and 5e), the posterior
 503 distributions of the remaining parameters display varying degrees of multimodality. In
 504 contrast, when multiple physical constraints are simultaneously coupled, the posterior
 505 distributions of all parameters exhibit substantial contraction and markedly higher peak
 506 densities. Among them, the changes are particularly pronounced for the physical model
 507 parameter K_F and the GPR hyperparameters σ and λ (Figs. 5b and 5d-5e).



508

509 **Figure 5.** Parameter posterior distributions for the sand column Cd-phosphate
510 cotransport model under the three scenarios.

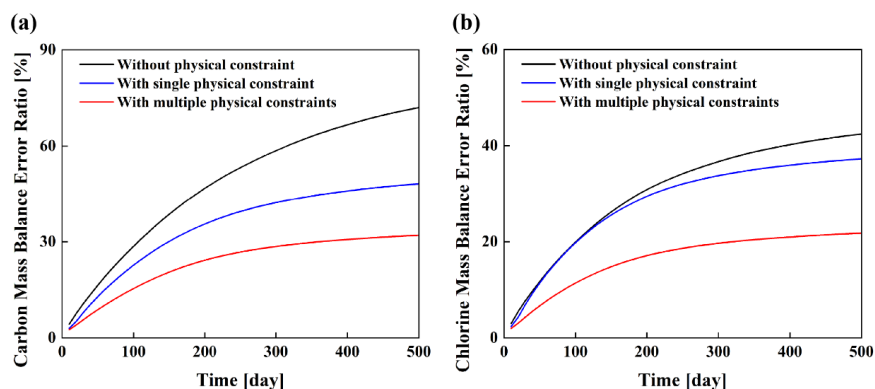
511 5.2. Mass Conservation Evaluation

512 5.2.1. Synthetic Groundwater PCE Reactive Transport Model

513 Fig. 6 illustrates the temporal variation in the carbon and chlorine mass balance
514 error rates (ERs) predicted by the DDM under the three scenarios for case study 1.
515 During the simulation, the contamination source continuously releases PCE at a rate of
516 25 kg/d, containing 3.63 kg of carbon and 21.37 kg of chlorine. The mass balance error
517 rate (ER) for carbon/chlorine is defined as:

$$518 \quad ER = \frac{M_T - M_S}{M_T} \times 100\% \quad (31)$$

519 where M_T and M_S denote the true total mass of carbon/chlorine in the system and the
520 corresponding total mass predicted by the DDM, respectively.



521

522 **Figure 6.** Variation in the carbon/chlorine mass balance error rate for the synthetic
523 groundwater PCE reactive transport model under the three scenarios.

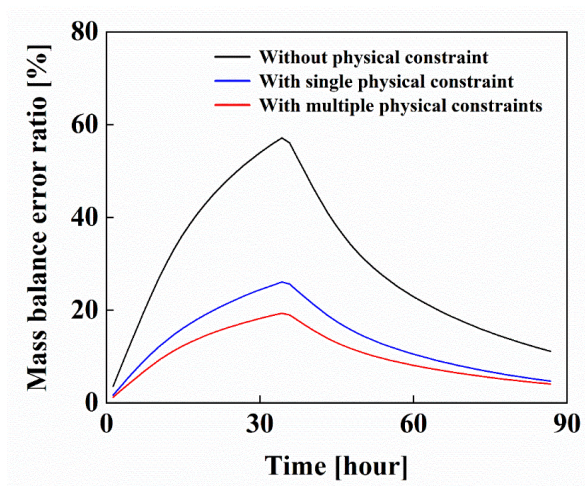
524 The results indicate that coupling physical constraints significantly reduces the
525 mass conservation error of the DDM predictions. Under the unconstrained scenario, the
526 mean mass conservation ERs for carbon and chlorine reach 48.62% and 30.65%,
527 corresponding to average mass errors of 1.76 kg and 6.55 kg, respectively. After
528 coupling the carbon mass conservation constraint, the mean ER for carbon decreases to
529 35.03%, with the average mass error reduced to 1.27 kg. Meanwhile, the mean ER for
530 chlorine decreases slightly to 28.27%, with an average mass error of 6.04 kg. When
531 both carbon and chlorine mass conservation constraints are simultaneously coupled, the
532 mean ERs for carbon and chlorine further decrease to 23.63% and 16.49%, with the
533 corresponding average mass errors reduced to 0.86 kg and 3.52 kg, respectively.
534 Moreover, compared with the unconstrained scenario, coupling physical constraints
535 results in a more gradual increase in the carbon/chlorine ER over time, followed by
536 stabilization in the later stages of the simulation.

537 5.2.2. Sand Column Cd-phosphate Cotransport Model

538 Fig. 7 presents the temporal variation of the Cd mass conservation error rate (ER)
539 predicted by the DDM under the three scenarios in case study 2. Over the entire
540 experiment period, a total of 4.72 mg of Cd is introduced into the soil column. Under
541 the unconstrained scenario, the mean mass conservation ER for Cd is 30.14%, with an
542 average mass error of 1.42 mg. The peak ER reaches 57.14%, corresponding to a



543 maximum mass error of 2.70 mg. When only the Cd mass conservation constraint is
544 coupled, the mean and peak ERs decrease markedly to 13.64% and 26.07%,
545 respectively, with the corresponding average and maximum mass errors substantially
546 reduced to 0.64 mg and 1.23 mg. When both the Cd mass conservation and
547 concentration bound constraints are simultaneously coupled, the mean and peak ERs
548 further decrease to 10.36% and 19.33%, respectively, with the corresponding average
549 and maximum mass errors reduced to 0.49 mg and 0.91 mg.



550
551 **Figure 7.** Variation in the cadmium mass balance error rate for the sand column Cd-
552 phosphate cotransport model under the three scenarios.

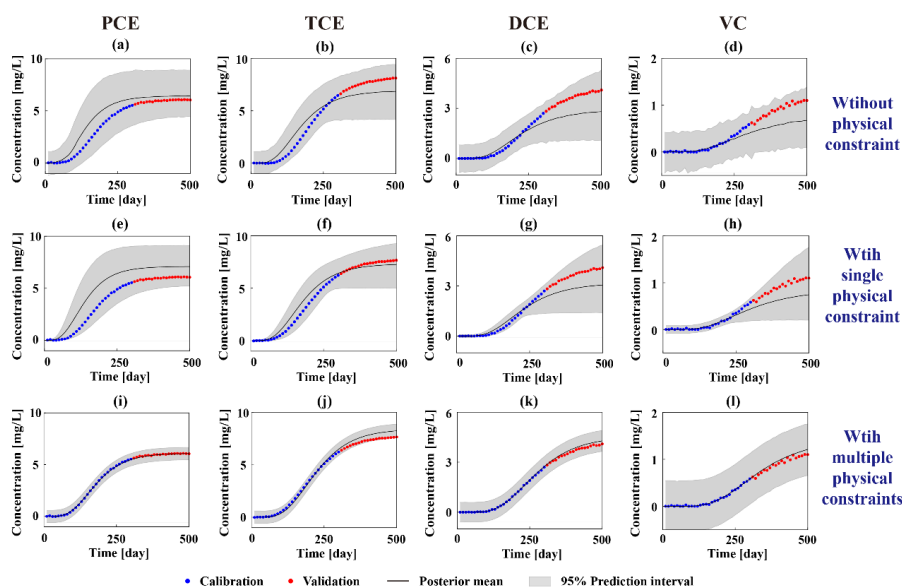
553 5.3. Prediction Performance

554 5.3.1. Synthetic Groundwater PCE Reactive Transport Model

555 Fig. 8 presents the temporal variations in concentrations of four chlorinated
556 hydrocarbons predicted by the DDM under the three scenarios in case study 1. Under
557 the unconstrained scenario, the model captures the overall trends of chlorinated
558 hydrocarbon concentrations. However, noticeable discrepancies remain between the
559 posterior means and the observations during the validation period (Figs. 8a-8d),
560 particularly for DCE and VC (Figs. 8c and 8d). After coupling a single physical
561 constraint, the DDM shows improved predictive performance. The predicted TCE
562 concentrations become closer to the observations, whereas substantial deviations from



563 the observations remain for the other chlorinated hydrocarbons during the validation
 564 period. In contrast, when multiple physical constraints are simultaneously coupled, the
 565 predictive accuracy for all four chlorinated hydrocarbons improves markedly, with
 566 particularly pronounced improvements during the validation period (Figs. 8i-8l).



567

568 **Figure 8.** DDM predictions of chlorinated ethenes concentrations under the three
 569 scenarios.

570 Moreover, under the unconstrained scenario, the 95% prediction intervals are
 571 generally wider, especially during the validation period (Figs. 8a-8d). When a single
 572 physical constraint is coupled, the predictive intervals narrow during the calibration
 573 period but remain wide during the validation period, particularly for DCE and VC (Figs.
 574 8g and 8h). In contrast, simultaneously coupling multiple physical constraints markedly
 575 reduces the prediction intervals for all chlorinated hydrocarbons (Figs. 8i-8l), indicating
 576 a substantial reduction in predictive uncertainty.

577



578 **Table 5.** Evaluation of predictions for the synthetic groundwater PCE reactive transport
579 model under the three scenarios.

Constraints	Calibration/ Validation	Species	NSE	MAE	RMSE
Without physical constraint	Calibration	PCE	0.7300	0.9444	1.0728
		TCE	0.9129	0.15615	0.6978
		DCE	0.9410	0.1709	0.2313
		VC	0.8732	0.0427	0.0708
	Validation	PCE	-13.9883	0.4076	0.4111
		TCE	-0.6106	1.10108	1.0313
		DCE	-8.5719	1.0405	1.0561
		VC	-4.2188	0.3369	0.3467
With single physical constraint	Calibration	PCE	0.4866	1.3393	1.4795
		TCE	0.9282	0.5057	0.6043
		DCE	0.9642	0.1383	0.1801
		VC	0.9230	0.0351	0.0552
	Validation	PCE	-96.8477	1.0494	1.0505
		TCE	0.2689	0.2481	0.2762
		DCE	-4.9220	0.8148	0.8307
		VC	-2.5998	0.2777	0.2879
With multiple physical constraints	Calibration	PCE	0.9994	0.0417	0.0496
		TCE	0.9955	0.1241	0.1505
		DCE	0.9991	0.0216	0.0285
		VC	0.9964	0.0091	0.0119
	Validation	PCE	0.8126	0.0356	0.0460
		TCE	0.6444	0.4863	0.4946
		DCE	0.7920	0.1471	0.1557
		VC	0.7728	0.0655	0.0723

580 Table 5 summarizes the quantitative evaluation of DDM predictive performance
581 under the three scenarios in case study 1. Compared with the unconstrained scenario,
582 coupling a single physical constraint leads to a modest improvement in the NSE of the
583 predicted concentrations for the four chlorinated hydrocarbons during the validation
584 period, although the improvement remains limited. Except for PCE, for which MAE
585 and RMSE increase slightly, MAE and RMSE decrease for the other chlorinated
586 hydrocarbons. Specifically, the MAE and RMSE of TCE decrease by 77.47% and
587 73.22%, respectively, whereas those of DCE and VC decrease by only about 20%.
588 When multiple physical constraints are coupled, the predictive performance of the



589 DDM improves markedly relative to the single-constraint scenario. During the
590 validation period, TCE behaves differently from the other chlorinated hydrocarbons: its
591 NSE increases from 0.2689 to 0.6444 (approximately 139.64%), whereas its MAE and
592 RMSE increase slightly. In contrast, the NSE values of the other chlorinated
593 hydrocarbons increase markedly from negative values to approximately 0.8,
594 accompanied by substantial reductions in MAE and RMSE. Specifically, both MAE
595 and RMSE decrease by approximately 95% for PCE, 81% for DCE, and 75% for VC.

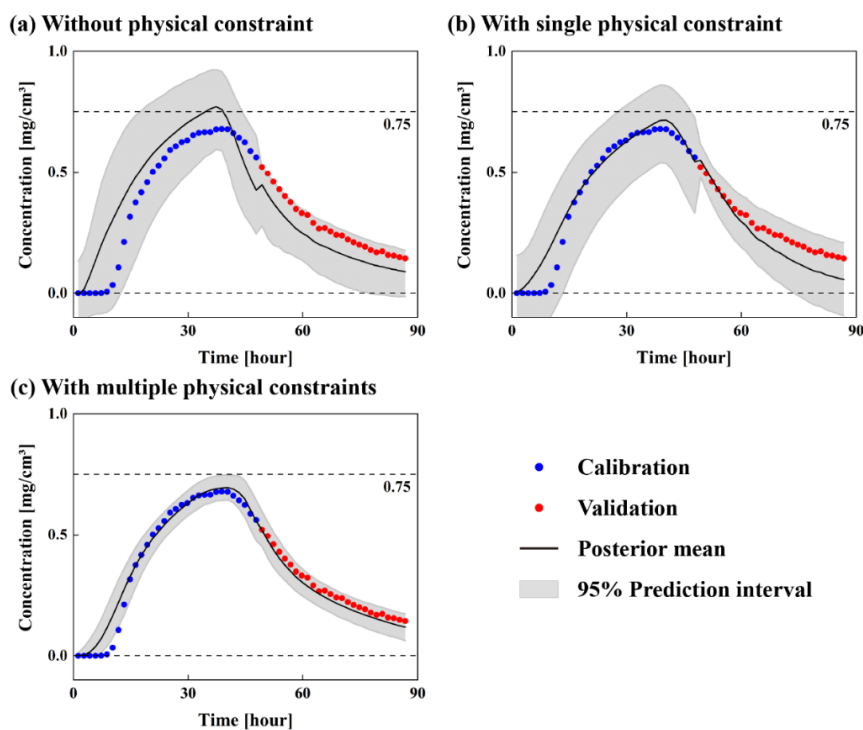
596 **5.3.2. Sand Column Cd-phosphate Cotransport Model**

597 Fig. 9 presents the temporal variation in Cd concentration predicted by the DDM
598 under the three scenarios in case study 2. Under the unconstrained scenario, the model
599 captures the overall trend of Cd concentration, which first increases and then decreases.
600 However, noticeable discrepancies remain between the posterior means and the
601 observations, with the concentration peak overestimated and concentrations during the
602 validation period markedly underestimated (Fig. 9a). After coupling the Cd mass
603 conservation constraint, the DDM predictions become more consistent with the
604 observations. Except for residual deviations at the early and late stages of the simulation,
605 the posterior mean aligns more closely with the observations at most time points (Fig.
606 9b). When multiple physical constraints are simultaneously coupled, the predictive
607 accuracy of the DDM improves markedly, with particularly evident improvements at
608 the concentration peak and during the validation period (Fig. 9c).

609 In addition, predictive uncertainty differs substantially among the scenarios. Under
610 the unconstrained scenario, the 95% prediction intervals are relatively wide (Fig. 9a).
611 After coupling a single physical constraint, the width of the prediction intervals changes
612 only marginally. Notably, some posterior predictions still fall outside the physically
613 plausible range of Cd concentration, with the predicted maximum exceeding the highest
614 observed value and the predicted minimum concentration dropping below zero (Fig.
615 9b). In contrast, when the Cd concentration bound constraint is additionally coupled,
616 all posterior predictions remain within the physically admissible range, and the
617 prediction intervals become substantially narrower, indicating a marked reduction in



618 predictive uncertainty.



619

620 **Figure 9.** DDM predictions of cadmium concentration under the three scenarios.

621 Table 6 presents the quantitative evaluation of the DDM predictive performance
 622 under the three scenarios in case study 2. Compared with the unconstrained scenario,
 623 coupling a single physical constraint increases the NSE of Cd concentration predictions
 624 during the validation period from 0.4874 to 0.6241 (an improvement of approximately
 625 28.05%), while the MAE and RMSE decrease by approximately 22.91% and 14.33%,
 626 respectively. When multiple physical constraints are simultaneously coupled, the NSE
 627 during the validation period further increases from 0.6241 to 0.9419 (an improvement
 628 of approximately 50.92%) relative to the single-constraint scenario, while the MAE and
 629 RMSE decrease by approximately 56.52% and 60.61%, respectively, indicating a
 630 substantial improvement in predictive performance.

631



632 **Table 6.** Evaluation of predictions for the sand column Cd-phosphate cotransport model
633 under the three scenarios.

Constraints	Calibration/ Validation	NSE	MAE	RMSE
Without physical constraint	Calibration	0.7696	0.1018	0.1198
	Validation	0.4874	0.0716	0.0726
With single physical constraint	Calibration	0.9460	0.0381	0.0580
	Validation	0.6241	0.0552	0.0622
With multiple physical constraints	Calibration	0.9704	0.0269	0.0429
	Validation	0.9419	0.0240	0.0245

634 **5.4. Discussions**

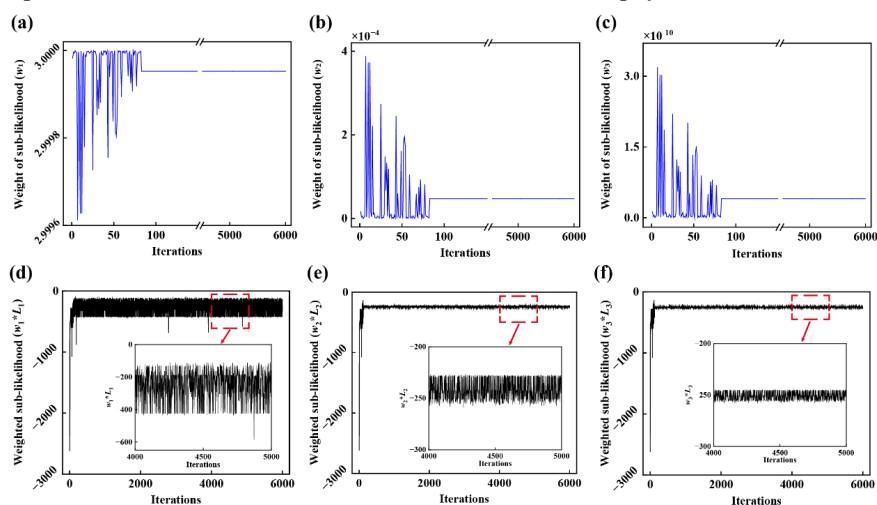
635 **5.4.1. Updating of Sub-likelihood Function Weights**

636 Since sub-likelihood functions associated with different physical constraints
637 represent distinct physical mechanisms, their numerical scales often differ by several
638 orders of magnitude. In case study 1, sub-likelihood values differ by up to six orders of
639 magnitude, whereas in case study 2 they differ by approximately one order of
640 magnitude. Such scale disparities cause smaller sub-likelihoods to be overshadowed by
641 larger ones during parameter identification, thereby diminishing the influence of certain
642 physical constraints in Bayesian inference and making it difficult for the corrected
643 predictions to simultaneously satisfy all constraints. Therefore, dynamically updating
644 sub-likelihood weights in the combined likelihood function is essential to ensure
645 appropriate contributions from each sub-likelihood during parameter identification.

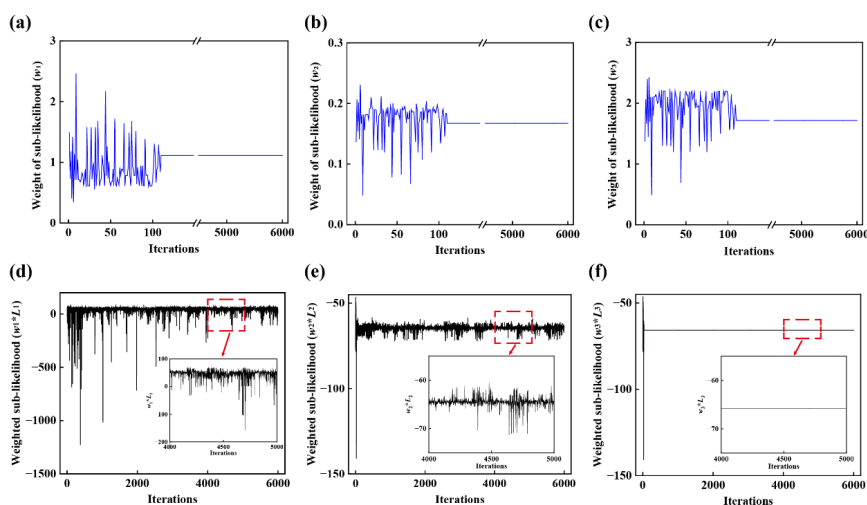
646 In both case studies, the evolution of sub-likelihood weights over the MCMC
647 iterations indicates that the proposed dynamic weight-updating strategy with a stopping
648 criterion plays a critical role in Bayesian inference (Figs. 10 and 11). After substantial
649 adaptive adjustment in the early iterations, the weights rapidly converge to stable values,
650 effectively rescaling sub-likelihoods that initially differed by several orders of
651 magnitude to a comparable scale. This ensures that all sub-likelihoods exert meaningful
652 influence on parameter identification while avoiding the subjectivity inherent in
653 manually assigned weights. Notably, the stable weighting patterns obtained in the two
654 case studies differ markedly, suggesting that fixed weights are unlikely to be suitable



655 across varying scenarios. In contrast, the proposed dynamic weight-updating strategy
 656 is well suited to combined likelihood functions that integrate multiple sub-likelihoods
 657 associated with distinct physical mechanisms. It maintains a balance between multiple
 658 physical constraints and observational information, thereby enhancing the robustness
 659 of parameter identification and the effective enforcement of physical constraints.



660
 661 **Figure 10.** Dynamic updating of sub-likelihood weights (a-c) and weighted sub-
 662 likelihoods (d-f) for the synthetic groundwater PCE reactive transport model.



663
 664 **Figure 11.** Dynamic updating of sub-likelihood weights (a-c) and weighted sub-
 665 likelihoods (d-f) for the sand column Cd-phosphate cotransport model.
 666



667 **5.4.2. Impact of Physical Constraints on Posterior Model Parameters and**
668 **Predictions**

669 The presence and the number of coupled physical constraints significantly
670 influence the posterior distributions of DDM parameters (Figs. 4 and 5). Without
671 physical constraints, the structural error correction process lacks the constraints
672 imposed by physical mechanisms, causing both the physical model parameters θ and
673 the GPR hyperparameters φ to be excessively adjusted in order to maximize agreement
674 with observations. Although coupling a single physical constraint can partially alleviate
675 this over-adjustment, it is generally insufficient to effectively constrain all parameters.
676 When multiple physical constraints are simultaneously coupled, the posterior
677 distributions of all parameters are confined to physically consistent ranges, thereby
678 effectively mitigating parameter overfitting.

679 The GPR hyperparameters exhibit greater sensitivity to physical constraints
680 because GPR lacks inherent physical regulation, whereas the parameters of the physical
681 model (groundwater model) are already subject to internal constraints such as mass
682 conservation. Consequently, the hyperparameters are more prone to excessive
683 adjustment than the physical model parameters. The DDM coupled with multiple
684 physical constraints primarily constrains the identification of GPR hyperparameters,
685 leading to substantially more concentrated posterior distributions and effectively
686 reducing parameter uncertainty.

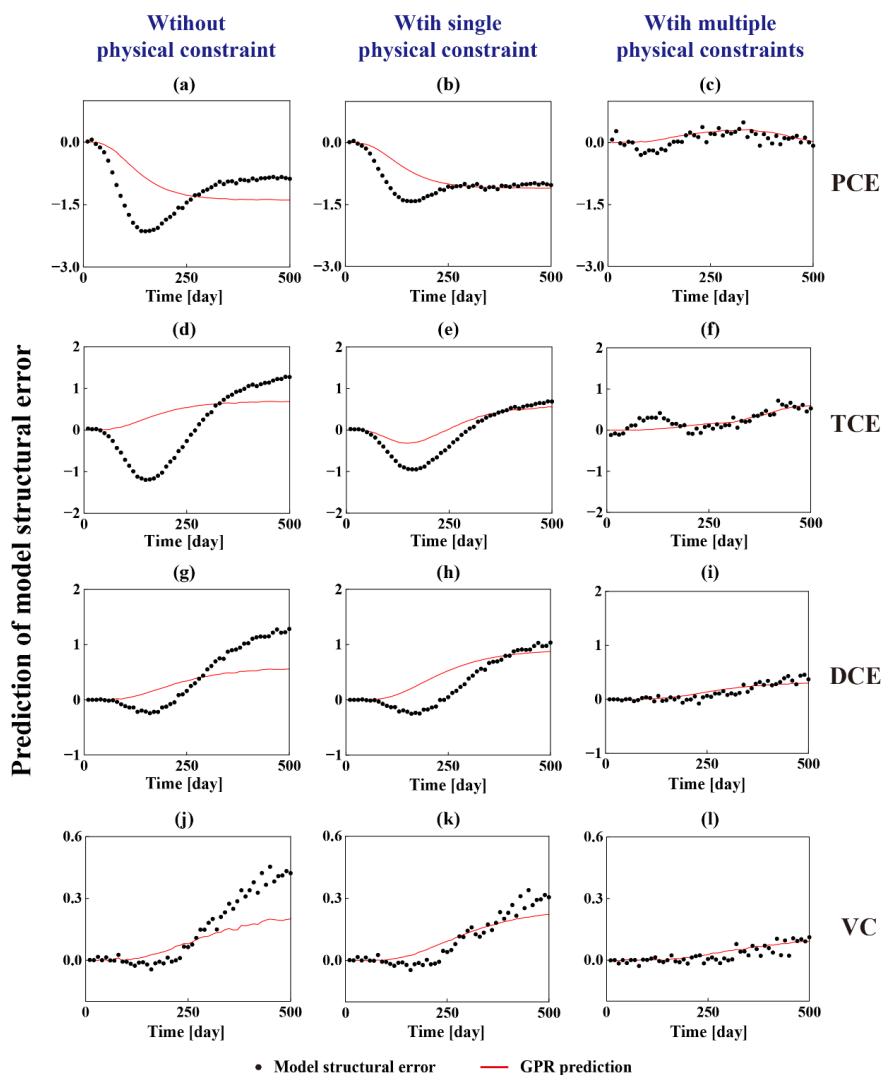
687 Notably, the observational data available for parameter identification are limited
688 in both case studies, with only 50 and 58 observations in case studies 1 and 2,
689 respectively. Under limited data availability and structural bias in the physical model,
690 parameter identification is subject to substantial uncertainty, resulting in pronounced
691 equifinality. In this context, incorporating appropriate physical constraints effectively
692 restricts the parameter identification process. Within the Bayesian uncertainty analysis,
693 these physical constraints function analogously to additional observations, thereby
694 further reducing parameter uncertainty.

695 The analysis of mass balance errors (Figs. 6 and 7) highlights the necessity of



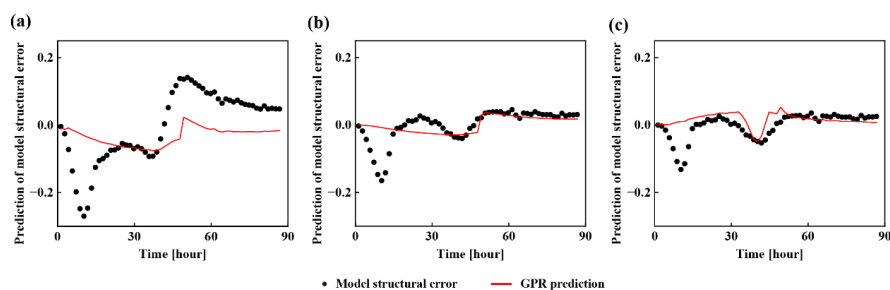
696 coupling multiple physical constraints during structural error correction. In case study
697 1, when only the carbon mass conservation constraint is coupled, the mass balance error
698 rate (ER) of chlorine remains at a relatively high level, whereas coupling both carbon
699 and chlorine mass conservation constraints substantially reduces the ERs of both
700 components. In case study 2, although the Cd mass conservation constraint reduces the
701 ER for Cd, the ER decreases further after the concentration bound constraint is
702 additionally coupled. These results indicate that a single physical constraint typically
703 regulates only the predicted variables directly associated with it, whereas predictions of
704 unconstrained variables may still violate physical principles. In contrast, coupling
705 multiple physical constraints constrains the structural error correction process at the
706 level of distinct physical mechanisms, thereby enhancing the physical consistency of
707 DDM predictions.

708 The presence and the number of coupled physical constraints also significantly
709 influence the forms of structural error (Figs. 12 and 13). Without physical constraints,
710 structural errors exhibit large amplitudes and greater complexity, making them difficult
711 for the GPR model to characterize accurately. Coupling a single physical constraint
712 partially suppresses the amplitude of structural error variations. In contrast, when
713 multiple physical constraints are coupled simultaneously, the posterior distributions of
714 model parameters are further constrained, leading to substantially smaller and more
715 stable structural error fluctuations. As a result, the complexity of structural error is
716 markedly reduced, enabling the GPR model to predict structural errors more accurately.



717

718 **Figure 12.** Predictions of model structural errors for the synthetic groundwater PCE
 719 reactive transport model without physical constraints (a, d, g, j), with single physical
 720 constraint (b, e, h, k) and with multiple physical constraints (c, f, i, l).
 721



722

723 **Figure 13.** Predictions of model structural errors for the sand column Cd-phosphate
724 cotransport model without physical constraints (a), with single physical constraint (b)
725 and with multiple physical constraints (c).

726 5.4.3. Limitations of this Study

727 This study explicitly incorporates multiple physical constraints into the structural
728 error correction process through a combined likelihood function, together with a
729 dynamic sub-likelihood weight-updating strategy and associated stopping criterion,
730 thereby overcoming the limitations of existing approaches in coupling multiple
731 constraints. Unlike PIML approaches, the proposed method preserves the physical
732 model predictions, with the corrected predictions consisting of the physical model,
733 structural error, and measurement error, enhancing the robustness of extrapolative
734 predictions. The method demonstrates broad applicability and enables the simultaneous
735 coupling of constraints associated with distinct physical mechanisms, thereby
736 improving the physical consistency of corrected predictions. Overall, this study
737 provides a generalizable framework for enhancing predictive performance in
738 simulations of contaminant reaction and transport within complex groundwater systems.

739 When the proposed method is applied to structural error correction in
740 multicomponent reactive transport systems, multiple physical constraints typically need
741 to be coupled simultaneously. In this context, the development of appropriate and
742 effective physical constraints is essential. In general, the introduced constraints should
743 exhibit clear mechanistic relevance to model structural error and observational data,
744 and directly represent key physical processes that have been neglected or simplified.
745 For instance, in more complex non-isothermal or multiphase flow systems, additional
746 constraints may be needed to explicitly represent temperature-dependent transport and



747 reaction processes, or the mass exchange and conservation of components across
748 different phases. The constraints should also possess explicit physical meaning and
749 quantifiable formulations to avoid imposing inappropriate restrictions. Furthermore,
750 when multiple constraints are incorporated, their complementarity and potential
751 competition should be carefully evaluated to prevent redundant or conflicting
752 constraints from adversely influencing parameter identification and compromising the
753 physical consistency of predictions.

754 In addition, an increase in the number of constraints inevitably increases the
755 complexity of the combined likelihood function, resulting in higher computational cost
756 and potentially affecting MCMC convergence. Future research may therefore explore
757 approaches to internally quantify and correct structural errors within groundwater
758 models, thereby reducing reliance on externally imposed physical constraints and
759 improving the applicability and stability of structural error correction.

760 **6. Conclusions**

761 Using DDM to correct model structural errors provides an effective approach for
762 improving the predictive performance of groundwater contaminant transport models.
763 This study proposes a general structural error correction framework capable of coupling
764 multiple physical constraints. By constructing a combined likelihood function and
765 dynamically updating sub-likelihood weights, the method explicitly incorporates
766 multiple physical constraints into the structural error correction process, thereby
767 substantially enhancing predictive accuracy and physical consistency of corrected
768 predictions. The framework is demonstrated through two case studies: a synthetic three-
769 dimensional PCE reactive transport simulation and a cadmium-phosphate cotransport
770 sand column experiment. The main conclusions are summarized as follows:

771 (1) An innovative data-driven framework is developed that couples multiple
772 physical constraints through a combined likelihood function and dynamic updating of
773 sub-likelihood weights. The structural error correction method exhibits broad
774 applicability to complex multicomponent reactive transport systems.



775 (2) The proposed dynamic weight-updating strategy with a stopping criterion
776 maintains a balance between multiple physical constraints and observational
777 information, thereby improving the robustness of parameter identification and
778 constraint enforcement. Consequently, it enables the simultaneous coupling of distinct
779 physical mechanism constraints.

780 (3) The DDM coupled with multiple physical constraints effectively regulates
781 parameter identification during structural error correction and suppresses excessive
782 parameter adjustment. Consequently, the forms of structural errors in calibrated
783 groundwater models become less complex and easier to characterize, leading to
784 improved predictive accuracy and enhanced physical consistency of the corrected
785 predictions.

786 (4) Within the DDM framework for structural error correction, introducing
787 appropriate physical constraints functions analogously to incorporating additional
788 observations, thereby further reducing predictive uncertainty.

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793 Center (HPCC) of Nanjing University for performing the simulations in this paper.

794 **Data Availability**

795 Version 5.0 of HYDRUS used for simulating the cadmium transport in the
796 laboratory sand column cadmium-phosphate cotransport case is preserved at
797 <https://www.pc-progress.com/en/Default.aspx?hydrus>. (Šimůnek, 2022)

798 Version 1.12.00 of MODFLOW-2005 used for simulating the groundwater flow in
799 the synthetic three-dimensional tetrachloroethylene reactive transport case is preserved
800 at <http://dx.doi.org/10.5066/F7RF5S7G>. (Harbaugh, 2017)

801 Version 1.1.00 of MT3D-USGS used for simulating the contaminant transport in



802 the synthetic three-dimensional tetrachloroethylene reactive transport case is preserved
803 at <http://dx.doi.org/10.3133/tm6A53>. (Bedekar, 2016)

804 Data used for the two case studies are publicly available at,
805 <https://www.hydroshare.org/resource/60f2db9c87914233b7fac0dcbe1f4f7b/>. (Tian,
806 2026)

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