

Influence of rainfall event characteristics and antecedent conditions on subsurface stormflow response of two forested hillslopes

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Abstract. Subsurface stormflow (SSF) is a critical runoff-producing mechanism in many upland and mountainous environments, yet the complex relationships between antecedent conditions, rainfall characteristics and SSF response are still not fully understood. Worldwide, the small number of SSF collection systems (trenches), as well as the generally small number of investigated SSF events limit our ability to generalize the findings and explore the influence of a broader range of storm sizes, intensities, antecedent wetness conditions and different hydrogeologic settings. In this study, we present a comprehensive analysis of SSF event characteristics in combination with rainfall event characteristics (depth and intensity), and antecedent conditions. The analysis is based on data collected over a 2-year period at two forested hillslope sites. Our results show that SSF volume is primarily controlled by total rainfall (P_{tot}) and antecedent wetness, with volumes being up to three orders of magnitude larger under wet initial conditions. For events with wet antecedent conditions, the rate at which the event SSF volume increased with rainfall clearly differed between the two trenched hillslopes. The peak SSF flow rates of smaller events were correlated with P_{tot} and antecedent conditions, but for larger events ($P_{\text{tot}} > \text{ca. } 20 \text{ mm}$), rainfall intensity and rainfall amount preceding peak rainfall intensity were more influential predictors than antecedent conditions. The steepness of the rising limb of the SSF hydrograph was correlated with P_{tot} and rainfall intensity. The antecedent soil moisture index (ASI) together with P_{tot} showed a high correlation with most SSF characteristics. The seasonal analysis revealed that, statistically, the largest SSF volumes occurred in winter and spring, while the highest peak flows were observed in spring and summer. Our results highlight the complex interactions among SSF responses, rainfall characteristics, and antecedent wetness conditions, underscoring the value of long-term monitoring across different seasons.

1 Introduction

Subsurface stormflow (SSF) is a key runoff-producing mechanism in many natural landscapes, often contributing significantly to the storm hydrograph of streams (McGuire et al., 2024; Peters et al., 1995; Weiler et al., 2006). Therefore, understanding the controls and processes governing SSF is essential for the future development of more robust rainfall-runoff models, particularly in light of increasingly extreme climatic conditions (Kirchner, 2006; Tabari, 2020; Weiler and McDonnell, 2007). Subsurface flow contributing to storm runoff (i.e. SSF) typically develops when percolating water encounters a downslope-dipping horizon or interface with reduced saturated hydraulic conductivity that limits vertical drainage; when the downward flow exceeds the percolation capacity across this horizon/interface, water perches and is diverted laterally towards the stream (Weiler et al., 2006). Alternatively, SSF can be generated through the “transmissivity feedback mechanism”, in which the rise of the water table into a more permeable zone induces a faster, lateral, stream-reaching flow (Bishop et al., 1990; Weiler and McDonnell, 2004). The quick subsurface flow response to rainfall can be coupled to high flow velocities along preferred pathways (preferential flow) as well as to the rapid increase of a more diffuse flow (matrix flow) (Blume and van Meerveld,

2015; Weiler et al., 2006). Preferential SSF occurs when water flows along larger interconnected voids in the matrix (e.g. soil pipes or fractures), through matrix zones characterized by a higher permeability than their surroundings, or in a channelized fashion dictated by the irregular topography of an impeding layer (Anderson et al., 2009; Weiler et al., 2006).

Several factors govern SSF generation, including the characteristics of the macropore and fracture networks, the spatial variation in soil thickness, the topography of surface and impeding layer (i.e. depression storage) and the vertical variation in hydraulic conductivity, which controls the vertical percolation, leakage rate through the impeding layer and the possible onset of faster lateral flow within upper more permeable layers (Hopp and McDonnell, 2009; McGuire et al., 2024). Antecedent wetness conditions further modulate the SSF responses. Wetter soils promote SSF, as they require less additional rainfall to reach saturated or close-to-saturated conditions and initiate lateral flow (Guo et al., 2014; Penna et al., 2011; Uchida et al., 2005). Nevertheless, dry conditions can lead to the formation of soil cracks and support hydrophobicity of soils, both favouring preferential SSF (Buttle and Turcotte, 1999; Peters and Ratcliffe, 1998; Tromp-van Meerveld and McDonnell, 2006a). Rainfall characteristics—including total depth, intensity, and temporal distribution—also play a crucial role. Greater rainfall amounts increase the overall water content and hydraulic head, promoting the connectivity of saturated areas across the hillslope, which enhances SSF generation (Hopp and McDonnell, 2009; Tromp-van Meerveld and McDonnell, 2006b). Hence, the contributing area expands with increasing rainfall amounts, leading to a more uniform hillslope response (Hrnčír et al., 2010; Šanda and Císlerová, 2009; Tromp-van Meerveld and McDonnell, 2006b).

The relationship between total event rainfall and resulting SSF volume is often nonlinear, with threshold-like behaviour observed in many studies (Tromp-van Meerveld and McDonnell, 2006a; Uchida et al., 2005; Weiler et al., 2006). SSF can be initiated by a relatively small precipitation amount ("activation" threshold), but significant SSF volumes are typically generated only after the rainfall amount exceeds a higher second threshold, which in hillslope studies ranges between ca. 18 and 60 mm (Fu et al., 2013; Tromp-van Meerveld and McDonnell, 2006a; Weiler et al., 2006). Beyond this higher threshold, some studies reported a linear relationship between SSF volume and total event rainfall, which was most evident for events with wet antecedent conditions, as then differences in soil moisture deficits are minimal (Du et al., 2016; Fu et al., 2013; Noguchi et al., 2001). To account for the influence of the antecedent conditions, some authors studied the relationship with SSF volume considering a combined variable, comprising rainfall amount and antecedent soil moisture index (ASI), which facilitated the threshold detection and increased the observed correlation (Fu et al., 2013; Han et al., 2020). Similar results were also observed at the catchment scale for the total stormflow volume (Detty and McGuire, 2010; Wei et al., 2020; Zwartendijk et al., 2023).

At the hillslope scale, SSF volume is generally found to be uncorrelated with precipitation intensity (Fu et al., 2013; Tromp-van Meerveld and McDonnell, 2006a). Nevertheless, rainfall intensity—especially peak intensity—is often, but not always (e.g. Tromp-van Meerveld and McDonnell, 2006a) correlated with peak flow. Noguchi et al. (2001) observed how for macropore flow this relationship was strong for some macropores but weak for others, whereas it was good for the matrix flow. In both cases, the correlation decreased for events characterized by drier initial conditions. Fu et al. (2013) report that, if events with small intensities and large ASI were excluded, a threshold between the max. 30-min intensity and peak flow was present. Moreover, Uchida et al. (2005) found that once pipe flow occurred, its maximum flow rate was sensitive to the maximum rainfall intensity. The lag time between rainfall (centroid or peak intensity) and peak flow depends on multiple factors, including peak rainfall intensity, cumulative rainfall preceding the peak, antecedent conditions, soil characteristics including macropores, as well as the runoff producing mechanism. Haga et al. (2005) observed that once the upslope saturated area was large, the lag time decreased with increasing rainfall intensity, indicating that the lag time is also affected by the timing of the peak rainfall (i.e. the state of the system preceding the higher intensities). The size of the contributing area also affects the stormflow response, as smaller areas were associated with a quicker response (Haga et al., 2005). On the other hand, the area drained by the SSF-collecting trench was shown to be influenced by rainfall intensity, rainfall amount and initial conditions (Hrnčír et al., 2010). A quick response can also be generated through fast macropore flow, which depending on the networks characteristics and its connection to the surface can be more or less influenced by the initial conditions and rainfall

intensities (Buttle and Turcotte, 1999; Uchida et al., 2005; Weiler and Naef, 2003). Zhang et al. (2021) observed a negative power function between peak intensity and lag time, with variability in lag times increasing when rainfall intensities were below a critical threshold. All the presented studies highlight the complex relationships between the SSF response, rainfall characteristics and antecedent conditions.

Despite our improved understanding of the controls of SSF, the limited number of study sites hinders our ability to further explore SSF and generalize the findings. At the hillslope scale, SSF has been investigated using non-invasive geophysical methods such as electrical resistivity tomography (Hübner et al., 2017; Uhlemann et al., 2024) and ground-penetrating radar (Guo et al., 2014), hydrometric measurements (wells, piezometers, tensiometers, soil-moisture sensors) (Du et al., 2016; Heller and Kleber, 2016), dye staining and excavation (Anderson et al., 2009), and, in particular, trenches excavated perpendicular to the slope (Buttle and Turcotte, 1999; Fu et al., 2013; McGlynn et al., 2002; Netto et al., 1999; Tromp-van Meerveld and McDonnell, 2006a; Uchida et al., 2005). Dyes, salts and stable water isotopes are commonly used as tracers to gain insights into flow-network characteristics, travel-time distributions and the interactions between matrix and preferential flow (Angermann et al., 2017; Kienzler and Naef, 2008; Tsuboyama et al., 1994).

Numerical modelling, when combined with experimental findings and conceptualizations, whether used to simulate real events (Meerveld and Weiler, 2008; Wienhöfer and Zehe, 2014) or to conduct virtual experiments (Hopp and McDonnell, 2009; Weiler and McDonnell, 2004), provides a complementary way to explore how surface and subsurface properties, antecedent conditions, and rainfall characteristics control the SSF response. Despite the many approaches available to explore SSF, trenched hillslopes remain a cornerstone of SSF research, as they are the only way to measure SSF directly and as a continuous time series of flow. However, due to the high installation costs and time-consuming maintenance, only a handful of research trenches have been excavated worldwide (Blume et al., 2025; Blume and van Meerveld, 2015). Moreover, the small number of SSF events analysed in most studies further limits the ability to explore the influence of a broader range of storm sizes, intensities and antecedent conditions. Especially the number of large, threshold-exceeding events is usually small. Additionally, many existing studies focus primarily on SSF volume and its relationship with rainfall amount, rainfall intensity and antecedent conditions, while peakflow is not always considered and other metrics as rising rate and response/lag time are often neglected in the analysis. As a consequence, to the best of the authors' knowledge, a comprehensive correlation analysis between all these metrics has not been carried out so far. Thus, this work aims to expand the knowledge of how rainfall event characteristics, antecedent conditions and SSF event characteristics (including peakflow, rising rate and response/lag time) are correlated, by analysing two years' worth of data collected at two trenches with contrasting hydrogeological settings (shallow and deep bedrock). The main research questions tackled in this study are: (1) What is the influence of rainfall depth and intensity on the subsurface stormflow response? (2) To what degree do the initial conditions (initial soil moisture and initial trenchflow rate) control the SSF response? (3) What is the relationship between different SSF event characteristics? (4) Does the SSF response vary seasonally?

2 Methods

2.1 Site description

Three trenches to capture SSF were installed in a headwater catchment in the Black Forest, southwest Germany (47°57' N, 7°50' E). For this analysis, we focussed on the two trenches with the most comprehensive data set: trench 1 and trench 3 (T1 and T3). The two trenches were excavated on two hillslopes located in a zero- and a first order sub-catchment of the research catchment. T1 and T3 are each situated near small perennial streams (Figure 1); the trench elevations are ca. 370 and ca. 400 m asl, respectively. The hillslope of T1 is predominantly planar, while the one at T3 exhibits cross-slope concavity. The average slope of the instrumented area, which extends up to ca. 25 m above the trench, is 19° for T1 and 27° for T3. The

topographically derived catchment area is ca. 3000 m² for T1 and ca. 27000 m² for T3. However, the actual catchment size might differ, as it is not only determined by surface topography, but also by unknown subsurface structures.

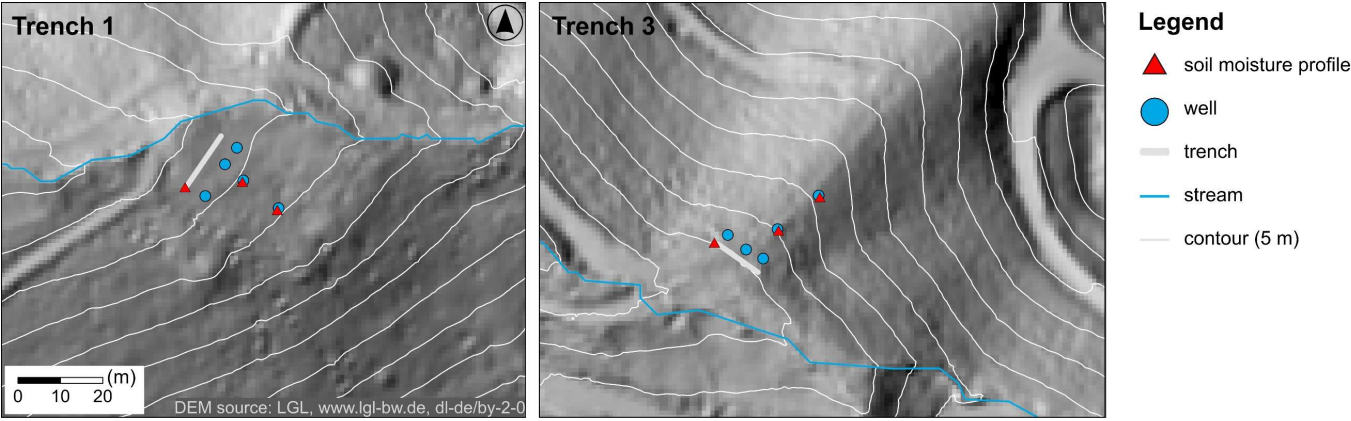


Figure 1. Location of the trenches, wells, soil moisture profiles and streams.

The climate is temperate, without a dry season and characterized by warm summers according to the Köppen classification (Cfb) (Peel et al., 2007). The mean annual temperature and precipitation are 11°C and 836 mm/yr, respectively (2003–2023; weather station of the German Meteorological Service (DWD) in Freiburg located ca. 7 km north from the study area). The summer months May to August are characterized by the highest mean monthly rainfall as well as the highest mean rainfall intensity (convective storms) (2003–2023; DWD Freiburg).

The bedrock is formed by Migmatite and Paragneiss at T1 and by Migmatite at T3 (LGRB-Kartenviewer, 2024). In both locations the bedrock is overlain by unconsolidated periglacial slope deposits in which Cambisols have developed. T1 was excavated into the slope deposits and does not reach the bedrock. From the surface down to 0.6 m below ground level (BGL) (depth of deepest soil sample), the soil is characterized by a high concentration of angular granule to cobble-sized clasts (i.e. diameter 2–256 mm) supported by a loam matrix (see Table 1 for soil properties). From 0.6 m BGL to the base of the trench at 2.7–2.9 m BGL the visual inspection showed an overall homogeneous material. Of particular interest is the soil colour of the trench face, which is not homogeneous. Besides the expected darker brown upper layer, a vertical ca. 2–3 m wide zone located on the southwest (right) side of the trench is characterized by a distinct dark bluish-grey colour. This coloration, which continues northeast (left) along the bottom portion of the trench, is a clear indication of waterlogged soils (Smirnova and Kozlov, 2023). The colour is typical of iron compounds that form in saturated soils under anaerobic (reducing) conditions (Schmidt and Ahn, 2019). The presence of such a long-lasting saturated zone matches well with our trenchflow and water level observations as well as the presence of a small intermittent spring located a few meters upslope of the trench.

At T3 the top 0.4 to 0.6 m of the soil is very poorly sorted and mainly composed of large (up to cobble-sized) angular clasts with a sandy loam matrix (see Table 1). Below this zone the amount and size of larger clasts gradually decreases (inverse grading) and the soil becomes more homogeneous and richer in silt (loam). This textural change is also supported by the water-retention analysis and soil density measurements of soil sampled at the trench face. The base of the unconsolidated slope deposits is defined by the weathered bedrock. At the trench, the interface between slope deposits and bedrock dips towards northwest (left trench side) while the elevation of the ground surface remains somewhat constant. This translates to an increase in thickness of the slope deposits from a few decimetres on the right side of the trench to more than 2 m on the left side of it. The thickness of the coarser upper layer remains fairly constant throughout the trench. Drilling samples of boreholes located ca. 12 and 21 m upslope of the trench also showed the presence of this coarse upper layer (thickness ca. 0.25–0.6 m) overlying a finer, more homogeneous sediment. This suggests that the hillslope directly above the trench is covered by a mostly continuous coarse-grained and thus highly permeable layer overlying a finer, less permeable one. The bedrock’s permeability is classified as very low according to thematic geological maps (LGRB-Kartenviewer, 2024) and thus likely acts as an aquitard/aquiclude in this hydrogeological setting.

The two hillslopes are covered by a mixed forest. The canopy above T1 is dominated by hornbeam (*Carpinus*) and spruce (*Picea*), interspersed with a few ash (*Fraxinus*) and beech (*Fagus*) trees. With higher elevation the proportion of coniferous trees increases giving rise to a coniferous-dominated forest. Young spruce trees make up most of the understory which is moderate above the trench and thins out further upslope. Strong winds toppled a few trees (mostly hornbeam) on the outer northern part of the hillslope during the observation period.

At T3 a mix of predominantly hornbeam (*Carpinus*) and fir (*Abies*) is found with a few robinia (*Robinia*) and ash (*Fraxinus*) on the lower part of the hillslope. The understory is thin and made up mainly of young fir trees. On both hillslopes the forest floor is covered by litter of deciduous and needle leaves as well as by some deadwood.

Table 1. Soil Properties at trench 1 and 3

depth (m BGL)	gravel (%)		sand / silt / clay (%)		bulk density (g cm ⁻³)		loss on ignition (%)		porosity (-)	
	T1	T3	T1	T3	T1	T3	T1	T3	T1	T3
0.1	65	56	43.3 / 31.7 / 25.0	57.5 / 25.7 / 16.8	1.1	1.2	7.4	6.0	0.57	0.54
0.3	63	65	41.6 / 34.2 / 24.2	66.2 / 20.6 / 13.2	1.3	1.3	4.2	6.0	0.52	0.51
0.6	77	55	51.7 / 33.7 / 14.6	54.6 / 26.5 / 19.1	1.7	1.2	2.2	3.8	0.34	0.52
1.5		77		44.6 / 41.9 / 13.5		1.9		1.7		0.29
2.0		77		52.4 / 34.6 / 13.0		1.9		2.1		0.25

The reported depths represent the midpoint of the 5 cm sampling interval. Analysis method: gravel = dry sieve, sand = wet sieve, silt and clay = laser diffraction, porosity = from fitted water retention curve obtained from HYPROP 2 and WP4C (METER-Group) measurements. Grain size classification: gravel > 2 mm, sand 2000–63 µm, silt 63–5.9 µm, clay < 5.9 µm. The silt-clay cut-off of 5.9 µm was used instead of 2 µm to align laser diffraction results with those obtained from traditional sedimentation analyses (Faé et al., 2019).

2.2 Experimental design

Two slope-perpendicular trenches, measuring 11 and 14 m in length and up to 2–3 m in depth, were installed to quantify SSF (Table 2). Photographs of the trenches during construction are provided in Appendix A. As illustrated in Figure 2 and documented in Appendix A, SSF was collected from two different depth horizons using drainage mats positioned along the trench face. At the bottom of the drainage mat a pond liner was inserted 5–10 cm into the trench face with an upward angle. The pond liner was then folded around a drainage pipe, forcing the collected water into the pipe (see cross section in Figure 2). The pipes were slightly tilted to route the water to the measuring equipment. Once the drainage system was installed, the trench was backfilled. The geometry of the two trenches (see Table 2) differs due to the different site characteristics.

The trench discharge was gauged using a set of two custom-built tipping buckets. Each tipping bucket is able to measure flowrates of up to ca. 450–500 l/h. Based on gravimetric reference measurements (bucket weighing and timed collection), the combined standard (1σ) uncertainty for discharges < 300 l/h is ca. ± 4 l/h. For higher discharges, uncertainty was not quantified but is expected to increase and discharge may be underestimated due to systematic errors (e.g. continuous inflow into the cavity of the tipping bucket while it is tipping; see Schwambach et al., 2022). Since discharges greater than 450 l/h were sporadically observed at the bottom section of T3, a custom-made V-Weir, built following the design proposed by Stewart et al. (2015), along with a vented pressure transducer (Truebner GmbH), was installed towards the end of the study period from 27 Mai 2024 to 25 June 2024. Along with the flow rate, the temperature and electrical conductivity of the trenchflow was also continuously measured. Above the trench, the shallow groundwater level was monitored using five observation wells; three wells were aligned 2–5 m above the trench and an additional two were located ca. 10 m and 20 m further upslope. The 6 cm-wide well boreholes were drilled using a hand-held percussion hammer. The depths of the boreholes vary between 1 to 4 m, as difficult drilling conditions (e.g. large boulders) sometimes limited the drilling depth (tool refusal depth). All boreholes were equipped with 5 cm diameter, fully slotted PVC pipes wrapped in geotextile. Where possible, the annular space between pipe and borehole wall was filled with quartz sand (≤ 2 mm). Each well was equipped with a capacitance water level sensor or a pressure transducer (Truebner GmbH) to monitor the water level. The water content and temperature of the soil was measured in three vertical profiles with TDT sensors (SMT100, Truebner GmbH). The deepest profile (2–2.5 m) was installed

along the edge of the trench-face a few decimetres away from the drainage mat (Table 2). The two shallower profiles were set up near the two upper wells and had sensors installed at 10, 30 and 60 cm BGL. All trench data (trenchflow rate, electrical conductivity, temperature, water level and water content) were continuously measured, predominantly using a 10-min logging interval.

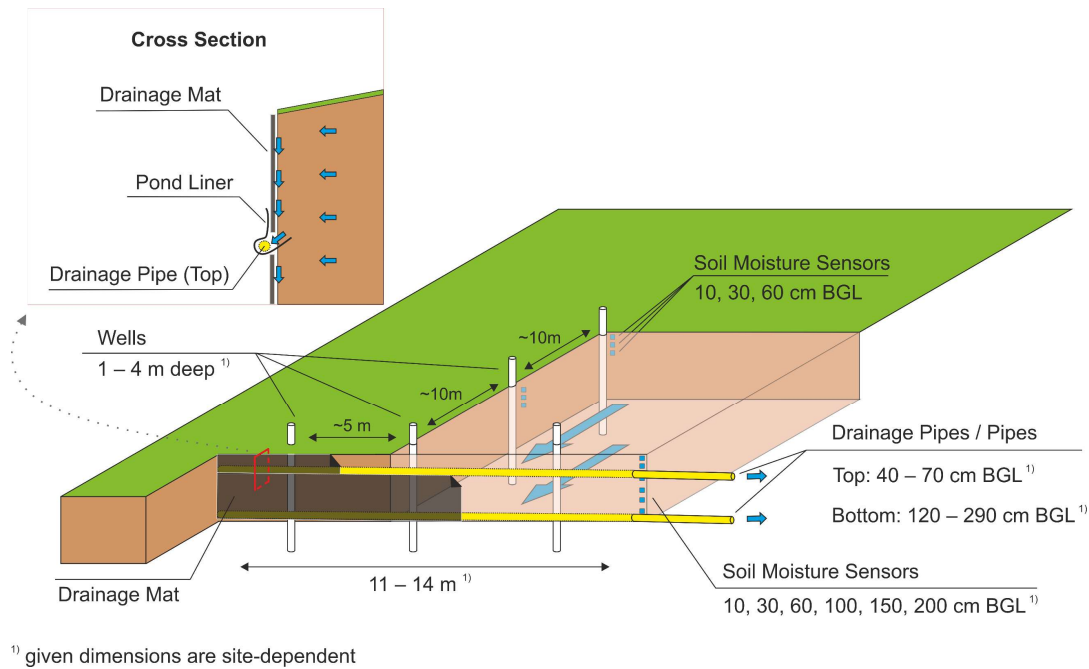


Figure 2. Experimental design of the trenches. Blue arrows show water flow directions.

Table 2. Trench properties

Properties	T1	T3
Trench total depth (m)	2.7 – 2.9	1.2 – 2.1
Upper section depth (m)	0.5 – 0.7	0.4 – 0.6
Trench width (m)	14.4	11.0
Soil moisture sensors depth (m)	0.1 / 0.3 / 0.6 / 1.5 / 2.0 / 2.5	0.1 / 0.3 / 0.6 / 1.0 / 1.4 / 2.0

Meteorological data was collected from a weather station set up in an open field near T1 and ca. 500 m from T3 at an elevation of ca. 350 m asl (47°57'25" N; 7°50'17" E). Rainfall was measured with a tipping bucket rain gauge (Thies) using a 5 min interval. Two gaps in the timeseries (12% of the total rainfall data) were filled with data from a station located ca. 2 km N-NW, equipped with a tipping bucket rain gauge (Young 52203). An event-based comparison between the two stations showed good agreement in the precipitation data, despite the secondary station being operated with a 10 min logging interval.

2.3 Data analysis

2.3.1 Data pre-processing

The analysed data was collected over a period of ca. 2 years from 25 August 2022 to 1 October 2024. The trenchflow data used for the analysis required some pre-processing. Short data gaps in the timeseries of T1 and T3 (e.g. due to sensor maintenance) were filled using linear interpolation. Besides a few exceptions, most gaps were shorter than 2 h. Outliers, caused by sporadic malfunctions of the tipping-bucket logging system, were manually filtered out and the gaps filled using the same linear approach. Longer gaps (e.g. due to sensor failure) were not filled and excluded from the analysis. Nearly all the data was recorded using a 10 min interval with some exceptions of 15 and 20 minutes. Therefore, to homogenize the timeseries the time was resampled into 10 min intervals using a linear interpolation approach. Since this study focuses on liquid precipitation,

periods characterized by snowfall and snowmelt were excluded from the analysis (10 December 2022–19 February 2023 and 5 January 2024–22 January 2024). These periods were identified based on data from a nearby snow-measuring DWD weather station (Freiburg) as well as field observations and trenchflow data. Only periods with data available for both trench sections (top and bottom) were considered. During a handful of events trenchflow exceeded the capacity of the tipping buckets of ca. 450–500 l/h leading to erroneous plateaus in trenchflow rate. These events were not included in the analysis. Nonetheless, a few exceptions were made for events that were not obviously cut off despite the observed extremely high flow rates. The rainfall timeseries, derived from the two weather stations (see previous section), was resampled using a 5-min interval. Soil moisture dynamics were monitored using SMT100 sensors (Truebner GmbH), which measure temperature and dielectric permittivity. The volumetric water content (VWC) was derived using the Complex Refraction Index Model (CRIM) to account for the influence of temperature on permittivity (see Appendix B for details). Short data gaps in the resulting VWC timeseries (e.g. due to sensor maintenance) were filled using a linear interpolation approach, while longer gaps were not filled. Most gaps were shorter than 2 hours.

2.3.2 Rainfall event delineation

To carry out the event-based analysis, events had to be delineated from the continuous rainfall timeseries. For this purpose, the commonly used minimum inter-event time (MIT) method in conjunction with the minimum rainfall depth (MRD) criteria was applied (Dunkerley, 2008). According to the MIT method, each rainfall event has to be preceded and succeeded by a fixed minimum dry period (MIT) (Brasil et al., 2022). In other words, two rainfall occurrences are considered part of the same event if they are separated by a time period shorter than the MIT. In the literature MIT values range from 3 min to 24 h depending on the site and application, and although values between 6 and 8 h are the most commonly adopted it is evident that no recommended MIT value exists (Dunkerley, 2008). Despite this, the MIT value heavily influences the number of delineated rainfall events as well as their characteristics (e.g. duration, depth, intensity) (Brasil et al., 2022). Therefore, different MITs were applied to our dataset and the results were evaluated. In our case, an MIT of 6 h yielded the most satisfactory results, as each SSF initiation was mostly associated with a single well-defined rainfall event and a relatively low number of single-tip events (i.e. events consisting of only one bucket tip) was generated. In comparison, longer MITs generated larger rainfall events spanning multiple SSF initiations while shorter MITs led to a greater segmentation of sometimes clearly associated rainfall pulses. Once the events were delineated (MIT of 6 h), the MRD criteria was applied to eliminate single-tip events, resulting in an MRD of 0.105 mm in our case.

2.3.3 Subsurface stormflow event delineation

Several methods from manual to fully automatic have been developed to delineate storm events from continuous streamflow records (Millar et al., 2022). However, most automatic and semi-automatic methods were developed for larger rivers, not for small, flashy, intermittent headwater creeks, whose complex flow dynamics more closely resemble those observed at the trenches (trenchflow). As a result, these methods often require somewhat smoother hydrographs, the presence of a continuous baseflow and in some cases well-defined recession limbs (Blume et al., 2007; Koskelo et al., 2012; Mei and Anagnostou, 2015). Additionally, only a handful of these methods integrate precipitation data linking rainfall events to stormflow events (Koskelo et al., 2012; Mei and Anagnostou, 2015). Automated methods for delineating SSF events based on trenchflow signal (e.g. not using a nearby stream to define the start and end of stormflow events) were not found in literature; authors dealing with trenchflow hydrographs generally use manual delineation methods or at least do not mention the use of automated methods (e.g. Fu et al., 2013; Han et al., 2020; Tani, 1997).

In this study, due to the long time series and to ensure the most objective delineation possible, a semi-automatic separation method was developed. The new method was tailored to the observed trenchflow, which was flashy, quite noisy and characterized by periods with and without “baseflow”. Furthermore, the method allowed to link rainfall events to SSF events

and further classify them. It scans the trenchflow discharge time series from the start of each rainfall event, and if the onset of an SSF event is detected, it links the two. If baseflow is absent, the onset of flow marks the event start. If baseflow is present, the start is detected by assessing the increase in discharge over two time windows (short and long) relative to a threshold. The end of SSF events is detected by analysing the change in discharge and its standard deviation over a fixed time window as well as considering the flowrate preceding the events. The delineation method is described more in detail in Appendix C. The automatic delineation results were satisfactory. For T1 and T3 on average only ca. 4% of the starts had to be added, 15% had to be removed and 12% had to be re-positioned, while 21% of the ends had to be re-positioned.

2.3.4 Classification of subsurface stormflow events

After the detection of starting and ending points in the trenchflow time series, the events were classified into simple events and subevents (Figure 3). Simple events are “stand alone” events that do not overlap with the preceding or subsequent events and are triggered by one rainfall event. Subevents are also associated with a triggering rainfall event but, unlike simple events, overlap with events detected before and/or after (i.e. they might start before the end of the preceding event and/or end after the start of the subsequent event) and thus are part of a larger, more complex event. For this reason, two or more overlapping events (i.e. subevents) are grouped into so-called complex events.

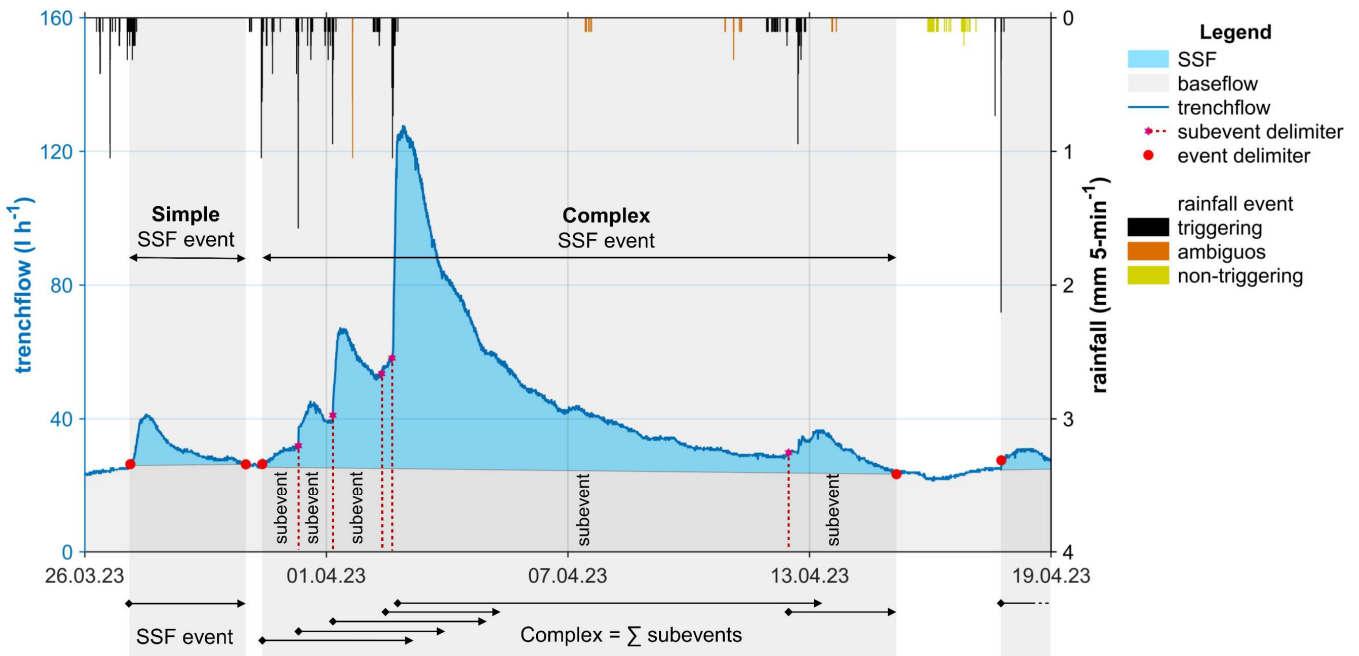


Figure 3. Classification of SSF and rainfall events

2.3.5 Rainfall event classification

Rainfall events are classified into three categories: triggering, ambiguous and non-triggering (Figure 3). Triggering events are all rainfall events that trigger SSF events (simple events and subevents), and are therefore linked to the start of SSF events. Ambiguous events are events that occur within SSF events but for which no SSF starting point could be detected (i.e. they do not lead to a noticeable increase in trenchflow); nevertheless, they may contribute to the event-trenchflow. Finally, non-triggering events are events that neither induce nor contribute to SSF events and therefore occur outside of delineated SSF events.

2.3.6 Event characteristics

For each rainfall event the total depth (P_{tot}), mean intensity (I_m), I_5 , I_{30} and I_{60} (i.e. max. 5-, 30- and 60-min rates) were calculated. For rainfall events that occurred during the period covered by the secondary weather station, I_5 was not considered due to the 10-minute logging interval of this station. Furthermore, we calculated the cumulative depth (P_{tot}) of all rainfall

events (triggering and, where present, ambiguous) associated with the respective simple or complex SSF event. This cumulative depth was used when analysing the relationship with metrics describing simple and complex SSF events: mean trenchflow rate ($\bar{Q}$), total SSF volume (V_{tot}) and minimum contributing area (MCA). Moreover, rainfall intensities (I_m , I_5 , I_{30} and I_{60}) of triggering events associated with the same complex SSF event were additionally averaged (e.g. for two triggering events A and B associated with the same complex SSF event, the I_{30} intensity was calculated as the arithmetic mean of the triggering events: $(I_{30A} + I_{30B})/2$). This averaging was performed to examine how rainfall intensity influences properties related to the entire complex event ($\bar{Q}$, V_{tot} , MCA)._{tot} The total SSF volume was computed after subtracting the baseflow, which was separated from the event runoff using the straight line method (Figure 4) (Maidment, 1995). Following this method, the timestep preceding the start of the event and the endpoint were connected through a straight line. The minimum contributing area (MCA), also called stormflow ratio, was calculated, as following (Dickinson and Whiteley, 1970):

$$\text{MCA} = V_{\text{tot}} / P_{\text{tot}} \quad (1)$$

The MCA represents the minimum area required to produce the observed SSF volume under the assumption that 100% of rainfall on that area is converted to SSF. For example, 10 mm rainfall producing 20 l of SSF corresponds to an MCA of 2 m². The contributing area is defined here as the area that contributes to the SSF collected at the trench during the SSF event. This area is expected to be larger than the MCA because only a fraction of rainfall over the contributing area typically contributes to the generation of SSF, due to losses to interception, soil moisture storage, vertical leakage, etc. The subsurface runoff coefficient (C_{sub}) is directly related to MCA ($C_{\text{sub}} = \text{MCA}/\text{SSF catchment area}$); however, because the extent of the SSF catchment area is not known, it can only be estimated by dividing MCA by the topographically derived catchment area. The remaining metrics ($Q_{\Delta\text{max}}$, time to peak, rising rate, response time and lag time) were calculated for simple events and subevents, and analysed against the rainfall characteristics of the triggering rainfall event associated with the respective simple or sub- SSF event (see Figure 3 and Figure 4). The maximum increase in trenchflow rate ($Q_{\Delta\text{max}}$) is the difference between the max. observed trenchflow rate and the initial trenchflow rate (Q_i) (i.e. flow preceding the event; see also Section 2.3.7 Antecedent conditions). The time to peak is defined as the period between the timestep preceding the start of the event and the max. trenchflow rate (peak flow), while the rising rate is calculated as $Q_{\Delta\text{max}}$ divided by the time to peak. Very small events having $Q_{\Delta\text{max}} < 3$ l/h were not considered for the rising rate calculations. The response time refers to the interval between the centroid of the rainfall event and the start of the SSF event, whereas the lag time is the time between the centroid and the maximum observed trenchflow rate.

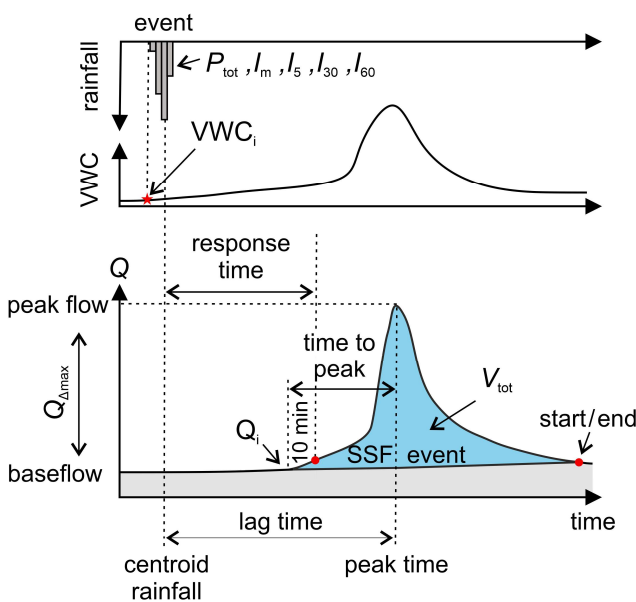


Figure 4. Definition sketch for the event-based analysis

310 In five cases, the rising phase of a subevent was interrupted by a successive subevent, preventing the potential peakflow of the preceding subevent from being reached. As a result, for these subevents the peak-dependent metrics were not considered. Moreover, occasionally simple events and subevents were characterized by a well-defined double peak with a higher second peak. In these and other exceptional cases (10 events) the $Q_{\Delta\max}$ was used in the analysis but the time-dependent metrics (e.g. rising rate) were not, as the double peak nature of the event would lead to inconsistent results.

315 2.3.7 Antecedent conditions

The antecedent wetness of the subsurface preceding the SSF events was characterized using soil moisture and trenchflow data. More specifically, the initial average volumetric water content (VWC_i), antecedent soil moisture index (ASI) and initial trenchflow rate were considered.

The VWC_i was calculated using a weighted average approach as described in Pangle et al. (2014):

$$\text{VWC}_i = \frac{\sum_{j=1}^6 \theta_j H_j}{H_{\text{tot}}} \quad (2)$$

320 where θ_1 to θ_6 represent the volumetric water content measured at different depths (see Table 2), H_1 to H_6 are the soil layer thicknesses that each sensor is assumed to represent, and H_{tot} is the total considered depth. For each sensor, H is calculated by dividing the distance between the sensor above and below by two. For the uppermost and lowermost sensors, the distance from the adjacent sensor was used. For example, for sensors depths of 10, 30, 60, 150, 200 and 250 cm the associated H would be 20, 25, 60, 70, 50 and 50 cm, respectively, and the total depth 275 cm. Since the soil moisture at 10, 30 and 60 cm BGL was
 325 measured at three different locations (i.e. at three soil moisture profiles: S1, S2 and S3), a mean value for each depth was computed and used in the depth-averaging procedure described above. Periods with missing or unreliable data were handled differently depending on the depth of the affected sensor. If sensors installed at depth 4, 5 or 6 had a data gap the VWC_i was not computed for that time period. On the other hand, for the shallower depths 1, 2 and 3 (i.e. 10, 30 and 60 cm BGL) if data from one or two sensors was missing, data from the remaining working sensor/s was used. The total equivalent water storage
 330 of the soil profile in mm can be obtained by multiplying VWC_i by H_{tot} .

The Antecedent Soil-moisture Index (ASI) was calculated using the following equation (Haga et al., 2005; Penna et al., 2015):

$$\text{ASI} = (\theta_1 \cdot 200) + (\theta_2 \cdot 250) + (\theta_3 \cdot 300) \quad (3)$$

where θ_1 , θ_2 and θ_3 represent the volumetric water content measured by the sensors at depths of 10, 30 and 60 cm BGL, respectively; they are multiplied by the layer thickness (in mm) that the sensors are assumed to represent (analogous to H in Eq. 2). As for Eq. 2, since soil moisture at 10, 30 and 60 cm BGL was measured at three different locations, the mean value
 335 for each depth was used in the calculations. Data gaps were handled as described for Eq. 2. The ASI, which represents the soil water storage (mm) in the upper soil layer, can be added to the total precipitation depth ($P_{\text{tot}} + \text{ASI}$), allowing us to describe the total rainfall and initial soil moisture conditions using one value. However, ASI is strongly dependent on the thickness of the considered soil profile (750 mm in our study); under identical moisture conditions, a deeper profile yields a larger ASI. Hence, the absolute value and event-to-event variability of ASI can be on the order of hundreds of mm, whereas P_{tot} is typically
 340 on the order of tens of mm. Consequently, ASI can be large relative to P_{tot} , so that antecedent conditions may receive disproportionate weight in the composite variable $P_{\text{tot}} + \text{ASI}$ when correlations with SSF metrics are analysed. We therefore introduced a second composite variable where ASI is weighted: $P_{\text{tot}} + \text{ASI}_{\text{adj}}$ (see section 2.3.8). Both the VWC_i and ASI were computed for the timestep preceding the triggering rainfall event. The trenchflow before the start of SSF events was also considered. The initial trenchflow rate (Q_i) was computed by averaging the flow over a period of 30 minutes preceding the
 345 start of the SSF event. For complex SSF events the initial trenchflow rate of the first subevent was considered.

2.3.8 Correlation analysis

The Spearman rank correlation coefficient (r_s) was used to quantify the degree of correlation between the metrics describing rainfall event characteristics, antecedent conditions, and SSF event characteristics. Correlation coefficients were not calculated for metric pairs that were not mathematically independent by construction and were therefore prone to spurious self-correlation. The correlation with the composite variable ($P_{\text{tot}} + \text{ASI}$) was also computed. However, since the results suggested that ASI was overrepresented in the composite variable (e.g. at T1 V_{tot} showed a smaller correlation with $P_{\text{tot}} + \text{ASI}$ than with P_{tot} alone), ASI was adjusted (ASI_{adj}) using a metric-specific weighting factor (w), which was optimized to maximize the Spearman correlation of the composite variable ($P_{\text{tot}} + w \cdot \text{ASI}$) with the other SSF metrics. This approach allowed us to assess the relative importance of antecedent wetness conditions within the composite variable when analysing its correlation with SSF metrics. The Spearman rank correlation was chosen to capture any monotonic relationship, as the observed relationships were sometimes nonlinear (e.g. exponential) and thus would have not been identified by the more restrictive Pearson correlation analysis. Given the large number of tested correlations, to keep the overall α -level at 0.05, following Bonferroni's correction the per-test significance level was obtained by dividing 0.05 by the total number of tested correlations (97 for each trench) (Curtin and Schulz, 1998). Correlations were considered statistically significant when the p-value was smaller than 0.0005.

Uncertainty in Spearman rank correlations was quantified using a bootstrap approach (Haukoos and Lewis, 2005). For each pair of metrics, the dataset was resampled with replacement (i.e. data points were randomly selected from the original dataset and could be selected multiple times) to generate 1000 bootstrap datasets (recommended ≥ 250 datasets; Efron and Tibshirani, 1986). Spearman's r_s was then computed for each resample, and a 95% confidence interval (CI) was obtained using the percentile method as the 2.5 and 97.5 percentiles of the bootstrap distribution of r_s . The confidence intervals of the Spearman rank correlations are presented in Appendix D.

To evaluate whether the correlation between Y and two alternative metrics (X_1 and X_2) improved, we computed the difference between the corresponding Spearman rank correlations, $\Delta r_s = r_{s2} - r_{s1}$, where $r_{s1} = r_s(Y, X_1)$ and $r_{s2} = r_s(Y, X_2)$. Uncertainty in Δr_s was quantified using a bootstrap approach (Wilcox, 2016). The dataset (Y, X_1, X_2) was resampled with replacement to generate 1000 bootstrap datasets; for each resample, Δr_s was computed and stored. A 95% confidence interval for Δr_s was then obtained using the percentile method. An improvement was considered supported when the 95% confidence interval for Δr_s ($\text{CI}_{\Delta r_s}$) lay entirely above zero. This analysis was performed only for selected metric pairs.

Since $\text{MCA} = V_{\text{tot}} / P_{\text{tot}}$, a direct regression of MCA against P_{tot} is susceptible to spurious self-correlation because a ratio is correlated with its own denominator (Kenney, 1982). To avoid ratio–denominator regression, we fit V_{tot} directly as:

$$V_{\text{tot}} = a \cdot P_{\text{tot}}^2 + b \cdot P_{\text{tot}} \quad (4)$$

This formulation implies a precipitation-dependent $\text{MCA}(P_{\text{tot}}) = a \cdot P_{\text{tot}} + b$, which we use for interpretation. Thus, $a > 0$ indicates that MCA increases with P_{tot} , and the value of a quantifies the rate of this increase. The lower bound for P_{tot} in Eq. 4 follows from $V_{\text{tot}} = 0$, which occurs at $P_{\text{tot}} = 0$ and at $P_{\text{tot}} = -b/a$. The upper bound for P_{tot} derives from the limiting case where the equivalent depth $V_{\text{tot}} / \text{CA}$ (mm; with V_{tot} in litre and CA the catchment area in m^2) equals P_{tot} , which occurs at $P_{\text{tot}} = (\text{CA} - b)/a$, provided that $a > 0$ and $\text{CA} > b$. Equation 4 was fitted separately for events with dry and wet antecedent soil moisture conditions. To classify the events, the observed range of VWC_i was divided into four equal intervals. Events with VWC_i values within the upper quarter of this range were classified as wet, while those below the upper quarter were classified as dry.

In general, uncertainty in the flow-rate measurements (see Section 2.2) is expected to increase scatter in the relationships, leading to lower correlation coefficients and wider confidence intervals, particularly for analyses involving the metrics V_{tot} and $Q_{\Delta\text{max}}$. Moreover, systematic errors associated with tipping-bucket measurements likely led to an underestimation of the highest flow rates and, consequently, of the $Q_{\Delta\text{max}}$ and V_{tot} values of events associated with such high flows. The statistical analyses were carried out using MATLAB (The MathWorks Inc, 2021).

3 Results

3.1 Rainfall events characteristics

During the observation period a total of 236 and 262 rainfall events were analysed for T1 and T3, respectively. The difference in number arises from the asynchronous data gaps in the time series of the trenches, since rainfall events occurring during data gaps were not analysed. Each simple SSF event and subevent (see Section 2.3.4) was associated with a specific triggering rainfall event (see Section 2.3.5). Figure 5a gives an overview of the precipitation rates of these individual triggering rainfall events. Some SSF metrics (e.g. V_{tot}) were not calculated for subevents but only defined for simple and complex events. Figure 5b shows the intensities of the triggering rainfall events associated with simple SSF events and the averaged intensities of the triggering rainfall events belonging to the same complex SSF event. Thus, one intensity value (e.g. I_5) was assigned to each SSF event, whether simple or complex. As expected, the rates decreased from I_5 to I_m and were quite similar for the two trenches.

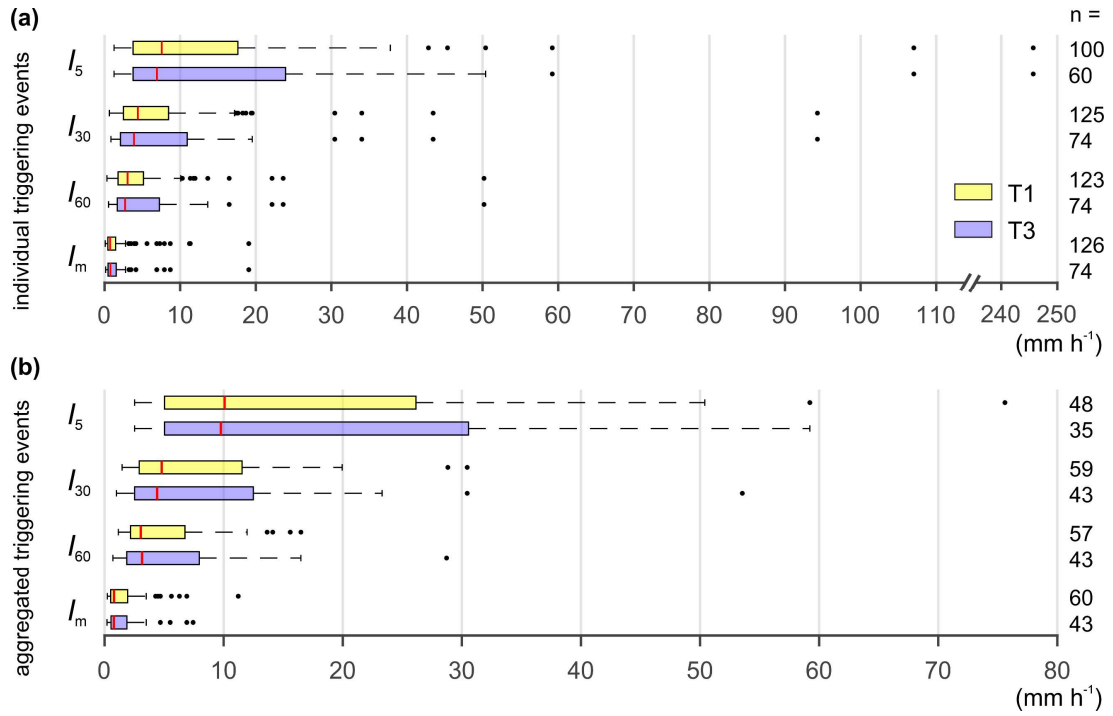


Figure 5. Overview of the rainfall rates of triggering rainfall events; (a) individual triggering events, (b) triggering events aggregated by their association with simple or complex SSF events (for each complex SSF event, a single representative rainfall intensity was computed as the mean of the intensities of its associated triggering rainfall events). The sample size (n) is shown on the right of each boxplot. I_5 , I_{30} and I_{60} are the max. 5-, 30- and 60-min rates, respectively, and I_m is the mean rate. Note that panel (a) has a gap in the axis and that the axis limits differ between panels (a) and (b).

The rainfall depths of triggering, ambiguous, and non-triggering events are shown in Figure 6a, while Figure 6b shows the total depth calculated by considering all rainfall events (triggering and, where present, ambiguous) associated with the same simple or complex SSF event. Most triggering events were larger than 5 mm; however, smaller events were also identified. T3 was characterized by larger triggering and non-triggering events, suggesting a higher precipitation threshold for the activation of SSF at the site. Moreover, at T3 the aggregated triggering and ambiguous events (Figure 6b) were usually characterized by a higher total rainfall amount compared to T1.

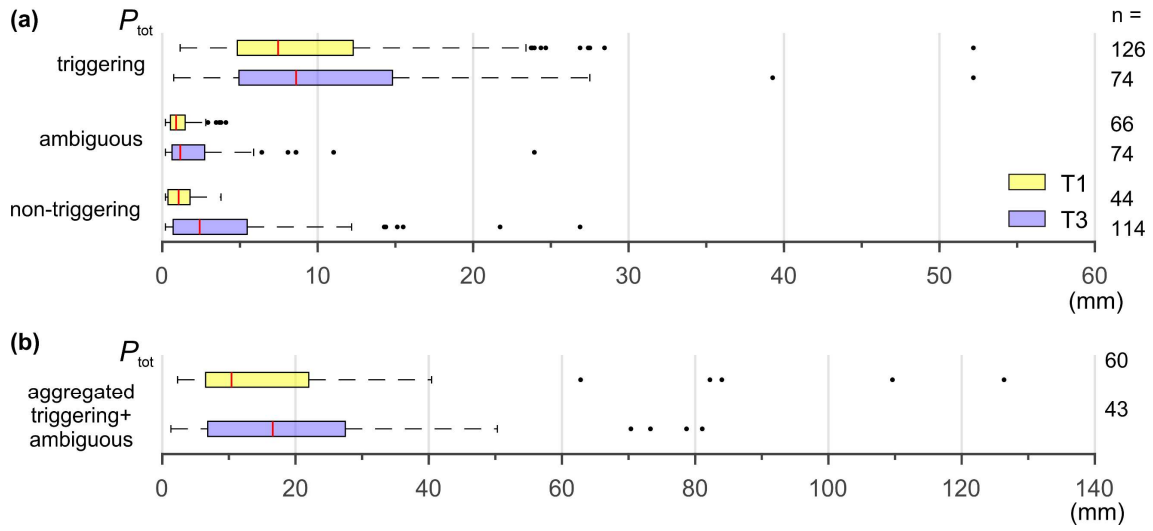


Figure 6. Overview of the (a) total rainfall depths of triggering, ambiguous and non-triggering rainfall events as well as of (b) total rainfall depth of the triggering and ambiguous rainfall events associated with each simple or complex SSF event (sum within each SSF event). The sample size (n) is shown on the right of each boxplot.

3.2 SSF events characteristics

In total, 38 simple events, 22 complex events and 88 subevents were identified at T1, whereas at T3, 29 simple events, 14 complex events and 45 subevents were identified. The characteristics of the analysed SSF events are presented in Figure 7. Overall, the SSF response was similar at the two sites. However, at T3 the 75th percentile of V_{tot} , $\bar{Q}$, MCA and $Q_{\Delta max}$ was higher, while it was longer for the response time and shorter for the lag time. Negative response times and lag times occur when the centroid of the rainfall event is located after the SSF start and peakflow time, respectively. The smaller response time at T1 indicates a faster SSF response and a smaller precipitation threshold, as the rainfall preceding the centroid was often sufficient to trigger SSF. The period between start of rainfall and start of SSF events (not plotted in Figure 7) was sometimes as small as 5 minutes at both sites (5 min time resolution).

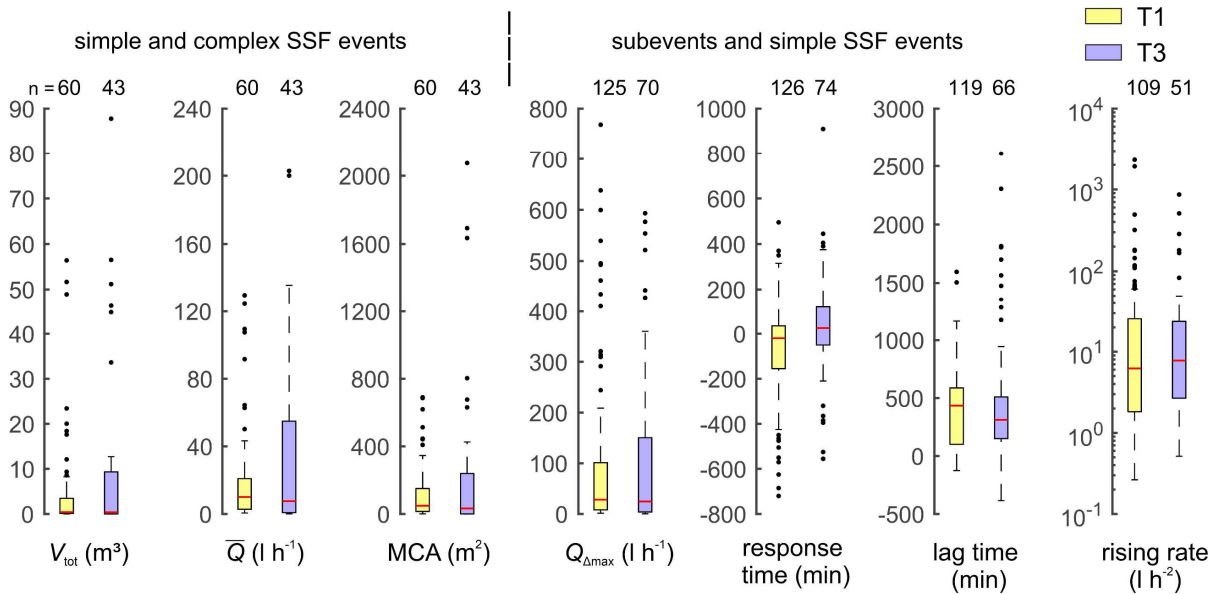


Figure 7. Overview of the characteristics of the analysed SSF events. The sample size (n) is shown above the boxplots. The difference in sample size of sub- and simple SSF event metrics for the same site results from the exclusion of some events from the calculation of specific metrics, according to the criteria outlined at the end of Section 2.3.6.

3.3 Correlation analysis

To study how different rainfall event characteristics and antecedent conditions influence the SSF response, the relationships between the descriptive metrics were analysed using the Spearman rank correlation (r_s). Figure 8 presents the resulting

430 correlation matrix, with the correlations for T1 and T3 displayed in the upper right and lower left portions of the table (i.e. above and below the diagonal), respectively. The associated confidence intervals are shown in Figure D1 of Appendix D.

The correlation patterns of T1 and T3 show a high level of congruence, suggesting that the observed relationships are not site specific. The total precipitation showed a moderate relationship with the intensities, with relationship strength increasing from I_m to I_{60} . The correlation between VWC_i and Q_i was strong at both trenches. However, while at T1, due to the continuous

435 baseflow, Q_i steadily increased with VWC_i starting from small VWC_i values, at T3 the increase started at higher VWC_i , since at this site baseflow was absent during dry conditions and thus lower VWC_i (data not shown). Comparing the correlation of the composite variable $P_{tot}+ASI$ and P_{tot} with the other metrics showed that for $P_{tot}+ASI$ the correlation was sometimes smaller than for P_{tot} (e.g. at T1, for $Q_{\Delta max}$ the r_s decreased from 0.81 to 0.41, with $CI_{\Delta rs} < 0$), suggesting that the influence of the initial soil moisture conditions (i.e. ASI) was overemphasized in the composite variable. We therefore also tested the composite

440 variable $P_{tot}+ASI_{adj}$, where ASI was weighted (adjusted) in order to achieve maximum correlation (see Section 2.3.8). The variables V_{tot} , $\bar{Q}$ and $Q_{\Delta max}$ showed a marked improvement in correlation (supported by $CI_{\Delta rs} > 0$) when the composite variable with the weighted ASI was considered, indicating a strong influence of the initial conditions on these variables. The improvement in correlation, the lower bound of $CI_{\Delta rs}$, and the weighting factor were higher at T3, suggesting that antecedent conditions play a stronger role at that site than at T1. For example, for $Q_{\Delta max}$ the correlation increased by 0.46 ($CI_{\Delta rs}$ 0.22–

445 0.70) at T3 but only by 0.07 ($CI_{\Delta rs}$ 0.02–0.12) at T1, and the weights for ASI were roughly two times higher for T3 than for T1. For the variables rising rate and response time, the differences in correlation with P_{tot} and $P_{tot}+ASI_{adj}$ were null or negligible, suggesting that initial conditions have little impact on these temporal metrics. Lag time was not correlated with P_{tot} , and the higher correlation obtained for the composite variable arise from its relatively higher correlation with VWC_i . The correlations with ASI are not shown in Figure 8, since they were comparable to the ones with VWC_i .

rainfall events						simple and complex events (aggregated rainfall events)			simple and subevents (triggering rainfall events)							
	P_{tot}	I_m	I_5	I_{30}	I_{60}	V_{tot}	$\bar{Q}$	MCA	$Q_{\Delta max}$	rising rate	lag time	response time	VWC_i	Q_i	$P_{tot}+ASI$	$P_{tot}+ASI_{adj}$
P_{tot}		0.39	0.50	0.63	0.68	0.83	0.79		0.81	0.65	<u>-0.23</u>	-0.63	-0.15	-0.14		
I_m	0.60		0.65	0.72	0.75	0.07	0.13	-0.08	0.36	0.62	-0.53	-0.01	<u>-0.30</u>	<u>-0.27</u>		
I_5	0.63	0.65		0.95	0.88	<u>-0.01</u>	0.01	-0.21	0.33	0.55	-0.48	-0.12	-0.60	-0.60		
I_{30}	0.71	0.74	0.96		0.97	0.09	0.16	-0.09	0.46	0.66	-0.54	<u>-0.22</u>	-0.39	-0.39		
I_{60}	0.74	0.80	0.90	0.97		0.14	0.19	-0.04	0.53	0.69	-0.55	<u>-0.26</u>	-0.37	-0.36		
V_{tot}	0.72	0.16	-0.27	0.06	0.09		0.95		0.82	0.01	<u>0.35</u>	-0.55	<u>0.31</u>	<u>0.35</u>	0.71	0.93
$\bar{Q}$	0.68	0.14	-0.23	0.06	0.08	0.96		0.91	0.86	0.02	0.27	-0.62	<u>0.35</u>	<u>0.40</u>	0.70	0.88
MCA		0.08	<u>-0.39</u>	-0.06	-0.02		0.94		0.63	-0.22	<u>0.50</u>	<u>-0.42</u>	0.49	0.53		
$Q_{\Delta max}$	0.41	<u>0.26</u>	-0.03	0.14	0.17	0.89	0.92	0.85			<u>-0.25</u>	-0.67	<u>0.21</u>		0.41	0.88
rising rate	0.62	0.52	<u>0.37</u>	<u>0.44</u>	0.49	0.33	0.36	0.26			-0.78	<u>-0.30</u>	-0.16		-0.04	0.65
lag time	-0.15	<u>-0.26</u>	-0.50	<u>-0.30</u>	<u>-0.32</u>	<u>0.44</u>	0.36	<u>0.49</u>	0.19	-0.50		0.06	<u>0.26</u>	<u>0.26</u>	<u>0.23</u>	<u>0.23</u>
response time	<u>-0.36</u>	-0.15	-0.07	-0.11	-0.13	-0.19	-0.16	-0.11	-0.45	-0.20	0.19		<u>-0.22</u>	<u>-0.20</u>	-0.35	-0.68
VWC_i	-0.44	<u>-0.39</u>	-0.72	-0.51	-0.51	0.60	0.63	0.67	<u>0.39</u>	-0.21	<u>0.36</u>	-0.17		0.94		
Q_i	-0.41	<u>-0.33</u>	-0.65	-0.47	-0.48	0.58	0.59	0.67			<u>0.25</u>	-0.19	0.87			
$P_{tot}+ASI$						0.85	0.83		0.52	-0.04	<u>0.35</u>	<u>-0.24</u>				
$P_{tot}+ASI_{adj}$						0.95	0.92		0.87	0.64	<u>0.35</u>	-0.42				
<i>italics</i> = not significant ($p > 0.0005$)						(.20)	(.22)		(.16)	(.07)	(1.0)	(.12)	T3			
<u>italics</u> = $p > 0.05$																
$ASI_{adj} = ASI \times (\text{weighting factor optimized for max. } r_s)$																

Figure 8. Spearman rank correlation coefficients between antecedent conditions, rainfall and SSF event characteristics at T1 (above diagonal) and T3 (below diagonal). The associated confidence intervals are provided in Appendix D.

3.3.1 Relationship between rainfall characteristics, total SSF volume and minimum contributing area (MCA)

For both trenches V_{tot} strongly correlated with P_{tot} , but the antecedent conditions also had a major influence on the SSF response. For similar P_{tot} the V_{tot} of events characterized by wet initial conditions were generally around one to three orders of magnitudes larger than their drier counterparts (see Figure 9 and Figure 10). Nevertheless, for very large events ($P_{tot} > 50$ mm) the influence of the antecedent conditions was often less pronounced and the $P_{tot}-V_{tot}$ relationship was poor, as larger P_{tot} did not always imply higher V_{tot} . Visual inspection of the scatterplots revealed that initial trenchflow rate (Q_i) was valuable to explain some of the results of T3, where events characterized by similar VWC_i and P_{tot} generated different trenchflow amounts (e.g. events with P_{tot} 15–20 mm in Figure 10). The rainfall amount to trigger a SSF response was minimal (2–3 mm) for both trenches. Nevertheless, the data suggests that the generation of somewhat larger events ($V_{tot} > 100$ l) required ca. 5 mm of rainfall for wet antecedent conditions and ca. 10 mm for dry ones at T1, whereas at T3 the amount was ca. 5 mm and ca. 25 mm, respectively. The influence of the antecedent conditions was effectively accounted for using $P_{tot}+ASI_{adj}$, as shown in Figure 11 and Figure 12, where for both trenches wet and dry events tend to form one common trend.

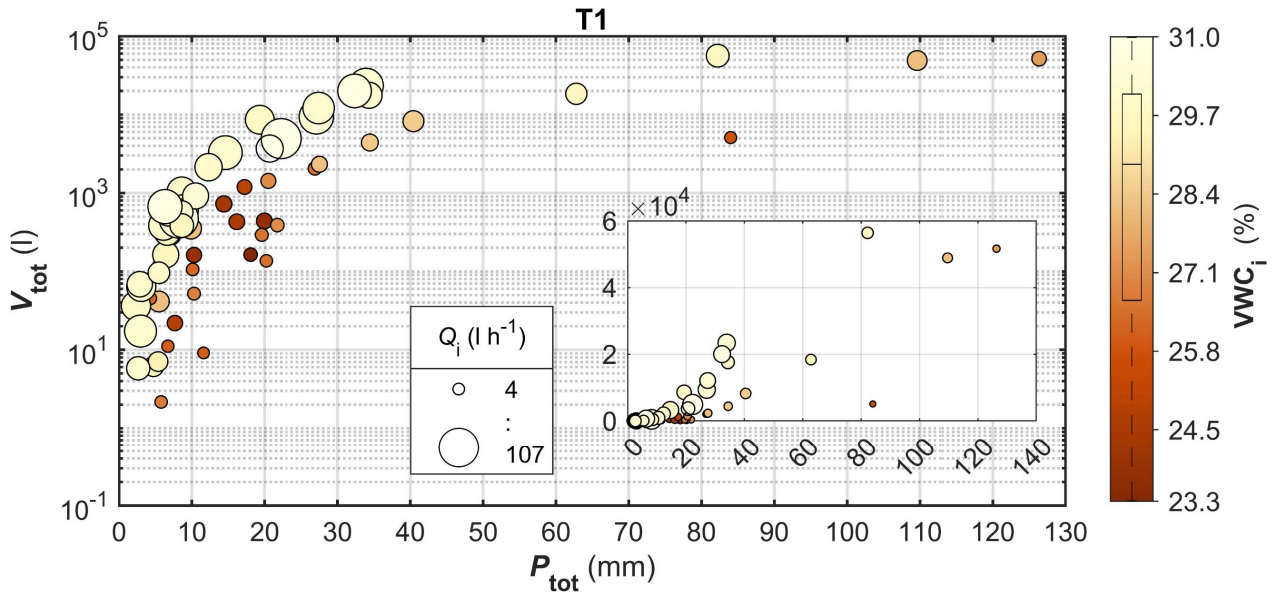


Figure 9. Total rainfall versus total SSF volume and antecedent conditions at T1 (event-based). The insert shows the same data on a linear-linear scale. In the insert the marker size was reduced for better visualization. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i.

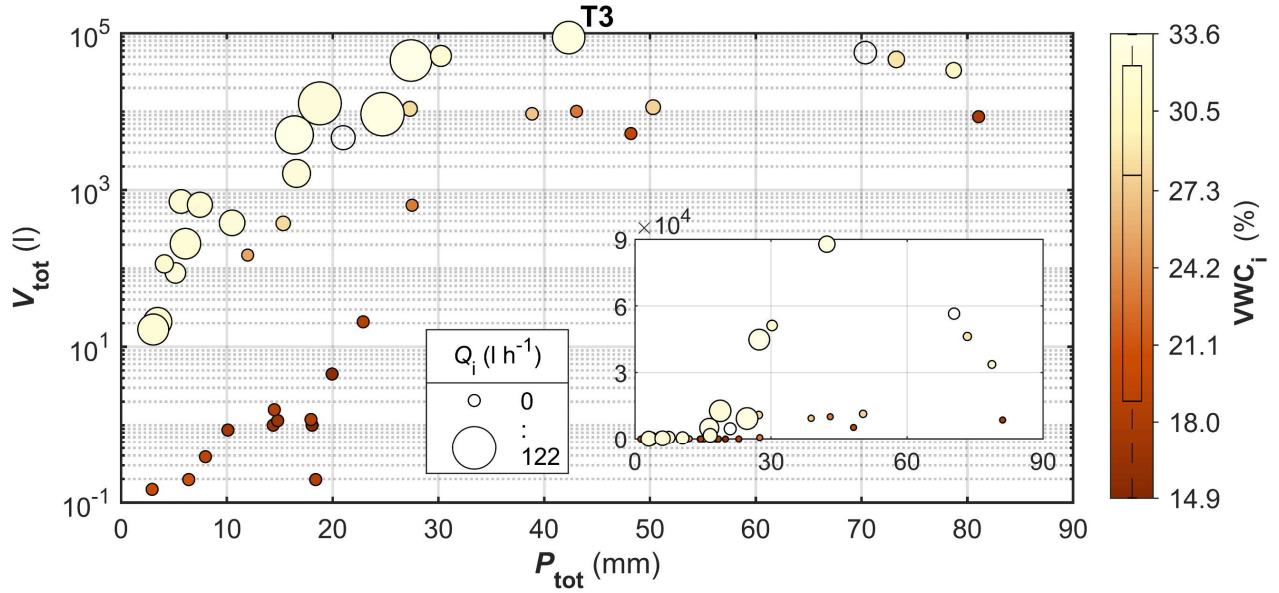


Figure 10. Total rainfall versus total SSF volume and antecedent conditions at T3 (event-based). The insert shows the same data on a linear-linear scale. In the insert the marker size was reduced for better visualization. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i.

Overall, at T1 the V_{tot} steadily increased with P_{tot} , whereas at T3, despite the general increase of V_{tot} with P_{tot} , this relationship appeared to undergo different stages, resulting in something like a step function on the semi-logarithmic plot (Figure 9 and Figure 10). More specifically, at T3, V_{tot} seems to increase markedly once P_{tot} exceeds ~15 mm under wet antecedent conditions and ~20 mm under dry antecedent conditions. This steeper increase appears to be preceded by a ~10 mm window in which V_{tot} shows only a weak increase with P_{tot} (5–15 mm under wet conditions and 10–20 mm under dry conditions). However, we acknowledge that the number of events with wet antecedent conditions is limited in this range.

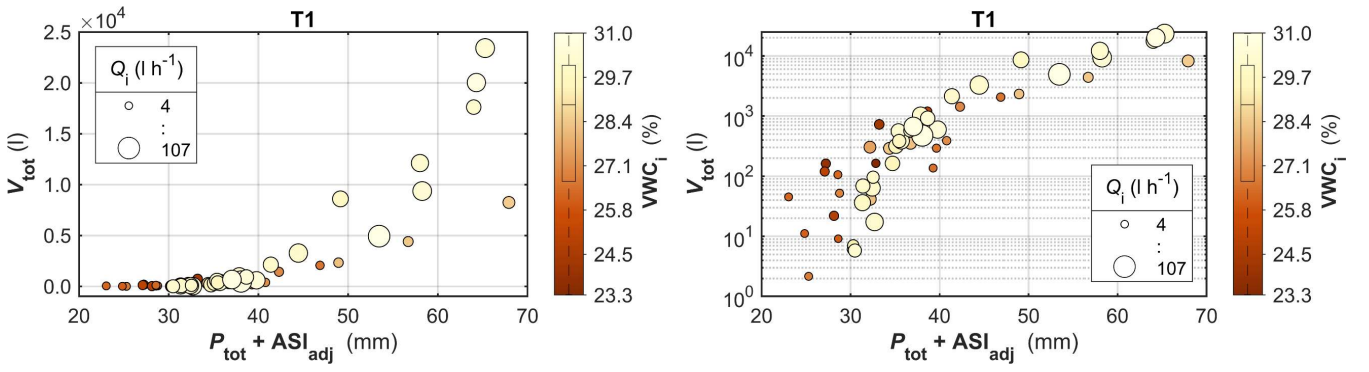


Figure 11. Relationship between total SSF volume and total rainfall + ASI_{adj} at T1 (left plot is lin-lin and right plot is lin-log). The largest events are not included in the plots. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i .

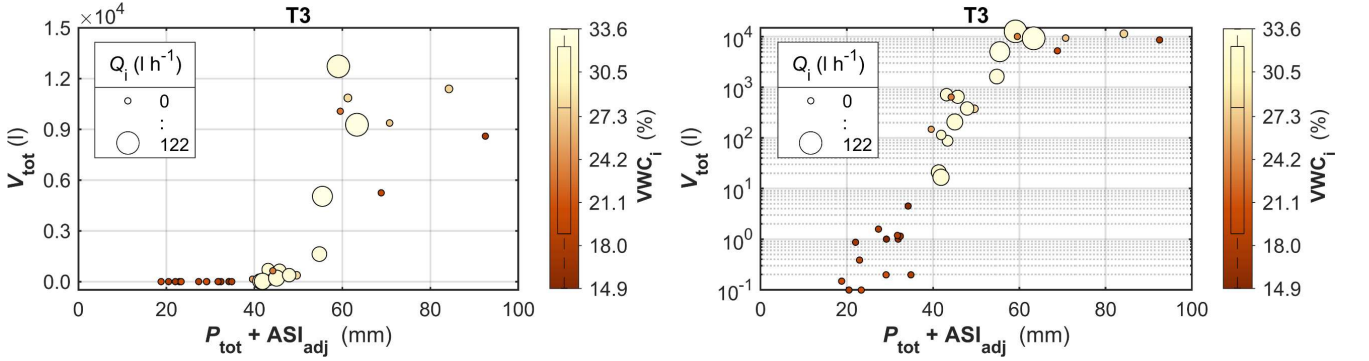


Figure 12. Relationship between total SSF volume and total rainfall + ASI_{adj} at T3 (left plot is lin-lin and right plot is lin-log). The largest events are not included in the plots. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i .

The initial conditions strongly influenced MCA, as under dry conditions MCA stayed relatively small, whereas under wet conditions MCA generally increased with P_{tot} reaching a maximum of ca. 700 m² and 2000 m² at T1 and T3, respectively (Figure 13). The increase in MCA suggests an increase of the SSF contributing area. At T3 the MCA increased steeply for events with P_{tot} greater than ca. 15 mm and wet initial conditions. The maximum MCA corresponds to an estimated maximum C_{sub} (calculated using the topographically derived catchment area) of approximately 0.23 and 0.08 for T1 and T3, respectively.

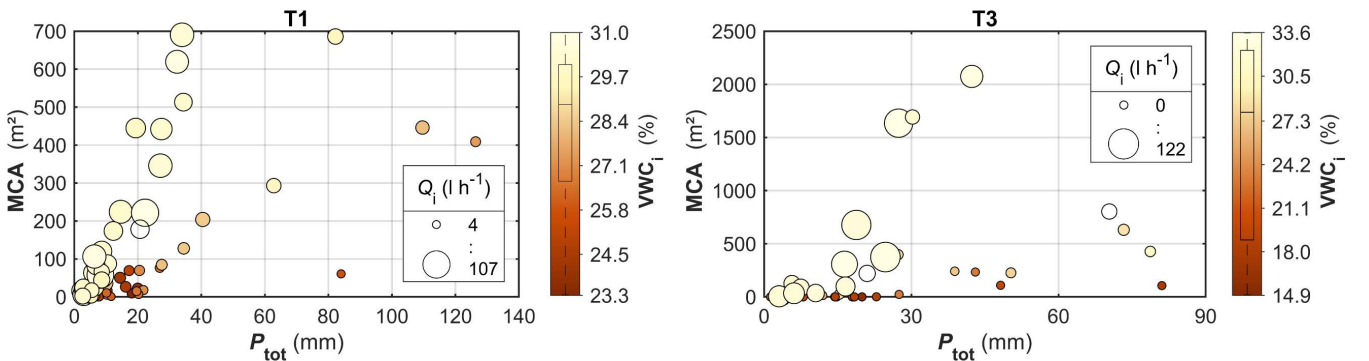


Figure 13. Relationship between MCA and total rainfall as well as antecedent conditions. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i .

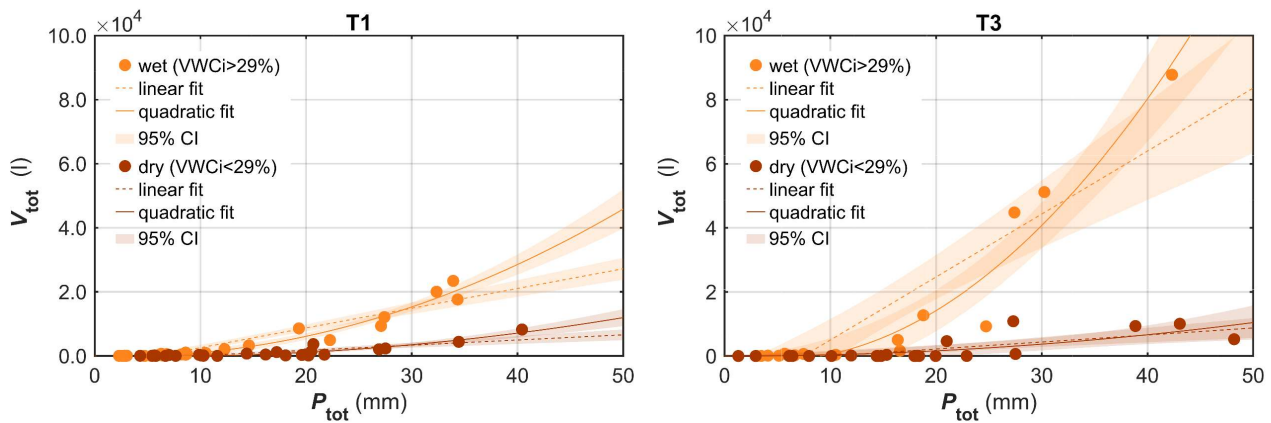
To further explore the relationship between V_{tot} , P_{tot} , VWC_i , and indirectly MCA the events were separated into two groups (wet and dry) based on their associated VWC_i . Furthermore, for T1 and T3 only events associated with $P_{tot} < 50$ mm were considered in the following analysis, as for those a more distinct V_{tot} vs P_{tot} relationship was identified. In total, 92% (T1) and 86% (T3) of all events were included in the analysis. As shown in Figure 14 and Table 3, within the considered range of P_{tot} , the linear fit between V_{tot} and P_{tot} was good at T1 (r^2 was 0.89 for wet events and 0.67 for dry events), and mediocre at T3 (r^2 was 0.46 and 0.56 for wet and dry events, respectively). Nevertheless, visually, the rate at which V_{tot} increases seemed to rise with P_{tot} , suggesting that a quadratic function would be better suited to represent the observed trend. This was evaluated by

fitting the quadratic Eq. 4 to the data. Since Eq. 4 is the volume-based reformulation of a rainfall-dependent MCA (see Section 2.3.8), the good fit of Eq. 4 (Table 3) supports the interpretation that the contributing area increases with P_{tot} , which is consistent with the trend suggested by the MCA– P_{tot} plot (Figure 13).

505 **Table 3 Coefficients with CI and r^2 of the fitted Eq. 4 and r^2 of the linear fit**

Trench		a	b	r^2 Eq. 4	r^2 lin. fit
T1	wet	20.4 (16.5–24.2)	-100.7 (-209.3–8.0)	0.96	0.89
	dry	6.1 (4.5–7.7)	-66.8 (-110– -23.0)	0.86	0.67
T3	wet	64.9 (47.4–82.3)	-586.7 (-1138–35.3)	0.94	0.46
	dry	4.3 (0.3–8.2)	-4.2 (-143.7–135.3)	0.55	0.56

The very different quadratic coefficients (a) for wet and dry conditions suggest that initial soil moisture conditions strongly influence the rate at which the contributing area increases with P_{tot} .



510 **Figure 14. Total rainfall versus total SSF volume of events with dry and wet antecedent conditions, with the best linear fit (dashed line) and the fit of Eq. 5 (solid line).**

The quadratic coefficient (a), computed for wet events, is higher for T3 than for T1, suggesting that at that site the contributing area increases faster with P_{tot} . Results from the quadratic $V_{\text{tot}}-P_{\text{tot}}$ relationship (Eq. 4) are considered to be valid only up to $P_{\text{tot}} \sim 50$ mm (the upper limit of the fitted range).

Rainfall intensities (I_5 , I_{30} , I_{60} and I_m) showed a very poor and often not significant relationship with V_{tot} and MCA (Figure 8). For both SSF metrics the variation decreased with the intensity, such that small intensities were associated with extremely large as well as extremely small V_{tot} and MCA, and very large intensities were linked to relatively small V_{tot} and MCA. At T1 and T3, for example, the largest SSF volumes were associated to I_5 of merely 10 and 6 mm/h, respectively. The large V_{tot} and MCA can be explained by the large P_{tot} and high antecedent soil moisture content associated with the events. Both have to be high, as emphasized by the results at T1 where under similarly high P_{tot} (82–84 mm) the event with dry antecedent conditions generated only 9% of the volume of the event with wet antecedent conditions, despite having some of the highest recorded intensities (I_5 and I_{30} of 76 and 29 mm/h, respectively).

3.3.2 Peakflow

The Spearman's correlation analysis showed that the maximum increase in discharge ($Q_{\Delta\text{max}}$) was strongly correlated to V_{tot} , and consequently to P_{tot} , especially when the antecedent conditions were also taken into account using $P_{\text{tot}}+\text{ASI}_{\text{adj}}$ (Figure 8).

525 Comparing the correlation with P_{tot} and $P_{\text{tot}} + \text{ASI}_{\text{adj}}$, the improvement in correlation and the lower bound of $\text{CI}_{\Delta\text{rs}}$ (0.02 at T1 and 0.23 at T3) was notably greater for T3, suggesting a stronger influence of the initial conditions on $Q_{\Delta\text{max}}$ at that site. The scatterplot analysis further showed how $Q_{\Delta\text{max}}$ was controlled by P_{tot} as well as antecedent soil moisture (VWC_i) mainly for events associated to rainfall events smaller than ca. 20 mm (Figure 15 and Figure 16). At T3, for example, 13–14 mm of precipitation with similar intensity resulted in a $Q_{\Delta\text{max}}$ of 1 l/h or ca. 230 l/h, depending on whether the initial conditions were

530 dry or wet, respectively, and at T1, 15–16 mm of rainfall triggered a $Q_{\Delta\text{max}}$ of 6 l/h or more than 320 l/h based on the different initial conditions.

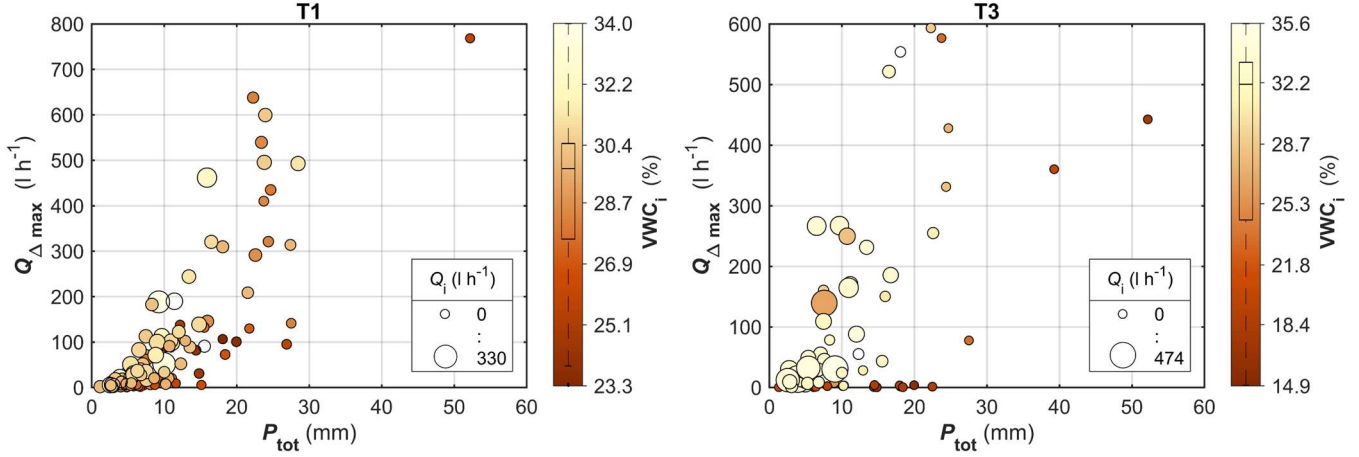


Figure 15. Relationship between $Q_{\Delta\text{max}}$ and P_{tot} as well as antecedent conditions. The antecedent conditions are represented by VWC_i (colour of markers) and Q_i (size of markers). The boxplot in the colorbar describes the distribution of VWC_i .

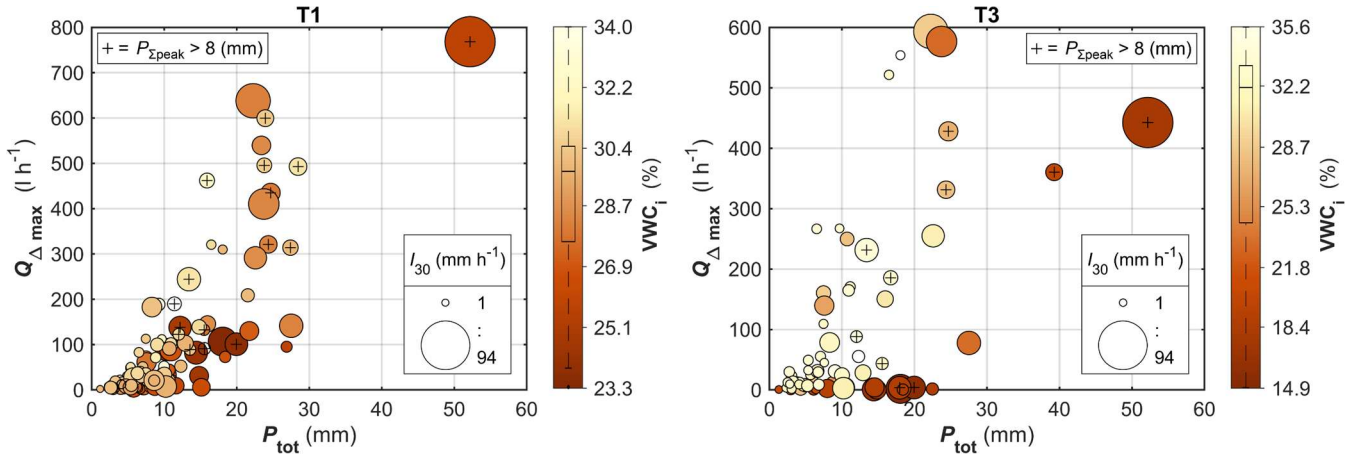


Figure 16. Relationship between $Q_{\Delta\text{max}}$ and P_{tot} as well as antecedent soil moisture conditions and I_{30} . Events where the cumulative rainfall calculated until peak rainfall intensity (I_5) is greater than 8 mm are marked with +. The size of the markers represents I_{30} and the colour of the markers represents the VWC_i . The boxplot in the colorbar describes the distribution of VWC_i .

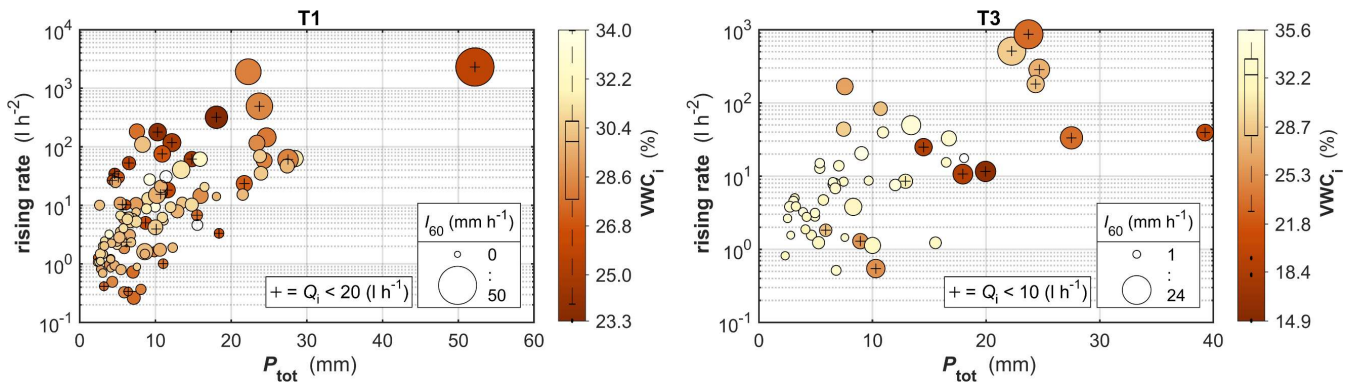
For SSF events associated with larger rainfall events ($P_{\text{tot}} > 20$ mm), on the other hand, the initial conditions appeared to play

540 a secondary role, as demonstrated by the extremely high $Q_{\Delta\text{max}}$ of events characterized by very dry initial conditions. For these larger events ($P_{\text{tot}} > 20$ mm) rainfall intensity as well as the amount of rainfall preceding the peak 5-min rainfall intensity ($P_{\Sigma\text{peak}}$ in Figure 16) appeared to play a more important role. At both trenches the highest $Q_{\Delta\text{max}}$ values were associated with fairly high rainfall intensities. For example, at T1 the largest $Q_{\Delta\text{max}}$ was associated with a rainfall intensity I_{30} of 94 mm/h (I_5 of 247 mm/h) and very dry initial conditions, whereas at T3 the highest $Q_{\Delta\text{max}}$ was associated with an I_{30} of 43 mm/h (I_5 of 107 mm/h). For $P_{\text{tot}} > 20$ mm, events with $Q_{\Delta\text{max}} > 200$ l/h were generally associated with high I_{30} (>18 mm/h) and/or with a rainfall amount preceding peak rainfall intensity exceeding 8 mm. Hence, the rainfall structure (i.e. temporal distribution of rainfall intensities) also showed to influence the SSF response. The results suggest that a combination of large P_{tot} , high intensities and/or large rainfall amounts preceding peak rainfall intensity was required to achieve very high $Q_{\Delta\text{max}}$, while initial conditions played a subordinate role.

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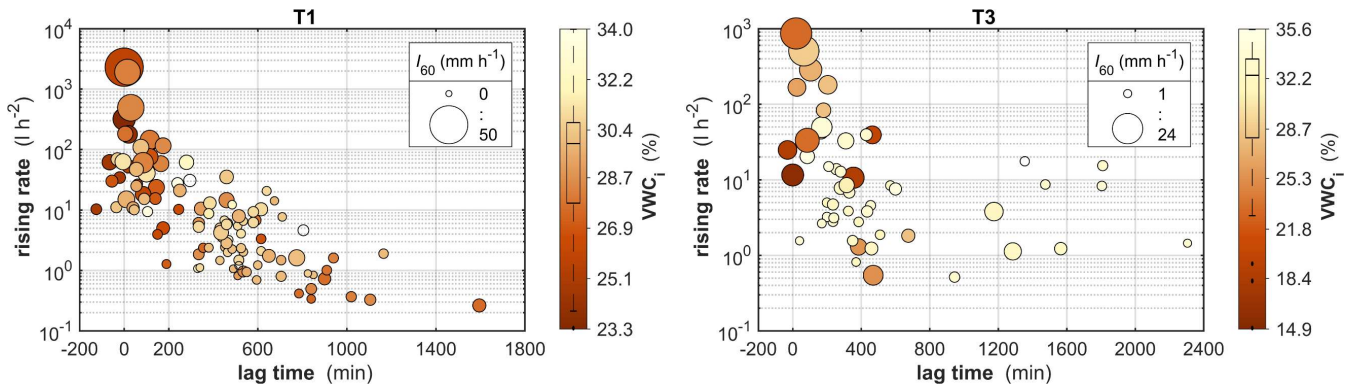
550 3.3.3 Rising rate, lag time and response time

The rising rate was positively correlated to P_{tot} as well as rainfall rate (Figure 17 and Figure 8). Especially events with extremely high rising rates were always associated with large P_{tot} and/or high rainfall intensities. Moreover, at T1, for example, events with a similar P_{tot} of 22–24 mm showed rising rates ranging from ca. 15 to 1900 l/h², which can be explained by the associated I_{60} ranging from 8 to 22 mm/h. The antecedent conditions, on the other hand, appeared to be less relevant. Nevertheless, some events with the highest rising rates were characterized by a low or a lack of initial trenchflow (see + in Figure 17) and dry antecedent soil moisture conditions, suggesting that dry antecedent conditions do not hinder high rising rates but sometimes favour them. This is evident in the results of T1, where for events with similar P_{tot} , those characterized by dry initial conditions often exhibited higher rising rates than those with wetter initial conditions. At T3, the influence of the intensity and initial conditions was less evident, partly due to the smaller number of events with dry initial conditions (see boxplots in Figure 17), as some events were excluded from the rising rate analysis (based on the criteria outlined in Section 2.3.6).



565 **Figure 17. Relationship between rising rate and P_{tot} as well as antecedent conditions and I_{60} . The size of the markers represents I_{60} and the colour of the markers represents the VWC_i . Events with $Q_i < 20$ l/h are marked with +. The boxplot in the colorbar describes the distribution of VWC_i . The y-axis is logarithmic.**

The time between centroid rainfall and peak flow (i.e. lag time) greatly varied for events with small rising rates, whereas it was mostly short for events with a high rising rate (Figure 18). For example, the median lag time of events with a rising rate larger than 50 l/h² was 20 min (T1) and 85 min (T3), whereas it was 465 min (T1) and 328 min (T3) for smaller rising rates.



570 **Figure 18. Relationship between rising rate and lag time as well as antecedent conditions and I_{60} . The size of the markers represents I_{60} and the colour of the markers represents the VWC_i . The boxplot in the colorbar describes the distribution of VWC_i . The y-axis is logarithmic.**

The lag time was not sensitive to the size of the rainfall event (P_{tot}) but was sensitive to its intensity (I_5 to I_m). Greater intensities were associated with smaller lag times. For example, at T1 the median I_{60} of events with a lag time greater than 100 min was 2.4 mm/h, whereas for shorter lag times it was 6.6 mm/h. Similarly, at T3 the median I_{60} was 2.5 and 6.5 mm/h, respectively. Dry initial conditions also seem to favour short lag times, as events with dry antecedent conditions generally exhibited short lag times. For example, at T1 the median lag time of events characterized by VWC_i and Q_i smaller than 26 % and 20 l/h was

20 min and 7 min, respectively, whereas for larger VWC_i and Q_i it was 460 min and 490 min, respectively. T3 showed a similar pattern: the difference in the median lag time of events with VWC_i and Q_i smaller and larger than 26 % and 10 l/h, respectively, was 213 min and 200 min, respectively.

The time between rainfall and SSF start can be calculated considering the rainfall centroid (i.e. response time, which is used in this analysis) as well as the start of the rainfall event. For both cases, their relationship to the rainfall event characteristics was found to be comparable. The P_{tot} as well as the cumulative rainfall calculated up to the SSF start showed to have a poor influence on the response time. Nevertheless, considering the cumulative rainfall up to SSF start, higher rainfall amounts were associated to a smaller variance in response time. Similar results were observed for the rainfall intensities preceding the SSF start, with variance decreasing as the intensities increase. These results indicate that a quick SSF response is more likely triggered by short, intense, and temporally focused rainfall events. The antecedent conditions did not show a clear relationship with the response time. Overall, the relationship between response time, rainfall event characteristics and antecedent conditions were not well defined.

3.3.4 Seasonality

To study the seasonality, the SSF and rainfall events were divided by season. For selected metrics the trimmed mean (i.e mean computed after excluding the lowest and highest 10% of values) was calculated on a seasonal basis (see Figure 19). The 95% confidence intervals (CI) were computed for each trimmed mean using a bootstrap approach (1000 resamples, percentile method); they are presented in Appendix E. The VWC_i and I_{30} associated to simple and complex events as well as non-triggering rainfall events showed a similar seasonal distribution as for the simple events and subevents and are thus not shown in Figure 19. Overall, both trenches showed similar seasonal patterns. The seasonal means of V_{tot} and MCA were higher in winter and spring; however, the CI of these metrics are wide and partially overlap, hence seasonal differences require cautious interpretation. These high winter–spring means coincide with higher VWC_i (with comparatively narrow CI), and are therefore consistent with the expectation that wetter initial conditions reduce the rainfall input required to generate SSF. The P_{tot} was higher in spring than winter (at T1 the CI overlap). Nevertheless, the higher winter VWC_i seems to compensate for the smaller rainfall amounts. High $Q_{\Delta max}$ were observed in spring at both trenches and at T1 it was also relatively high during summer. At T3, $Q_{\Delta max}$ was moderate during summer despite the occurrence of relatively large and intense rainfall events, which is consistent with the dry initial conditions and their stronger influence on the SSF response at this site. The rising rate was highest in spring at T3 and in summer at T1, but with wide CI. The longest mean lag time was observed in spring at both trenches, and arises due to a handful of events with extremely long lag time, as reflected by the broad CI.

The total rainfall of non-triggering rainfall events (** P_{tot} in Figure 19) also shows a seasonal trend. The highest and lowest ** P_{tot} , observed during summer and winter months, respectively, are consistent with lower VWC_i in summer and higher VWC_i in winter as well as the change in interception storage (i.e. leafed and leafless periods). The same picture is given when the events are divided by growing (May–October) and dormant (November–April) season. Overall, the interception storage appears to be small (ca. < 2–3 mm).

T1					T3				
** P_{tot}	(mm)	1.0 ¹³	0.9 ⁵	1.1 ⁹	1.7 ⁹	3.4 ⁴⁰	1.1 ⁸	1.9 ¹³	4.2 ²⁹
P_{tot}	(mm)	9.8 ³⁴	5.4 ²⁰	8.9 ²⁹	11.4 ¹⁷	12.1 ¹³	5.2 ¹⁷	11.4 ¹⁴	12.6 ¹⁴
* P_{tot}	(mm)	13 ¹⁷	16 ⁸	19 ¹³	18 ¹²	18 ¹²	11 ⁶	27 ⁸	24 ⁹
I_{30}	(mm h ⁻¹)	6 ³³	2 ²⁰	5 ²⁹	12 ¹⁷	6 ¹³	2 ¹⁷	6 ¹⁴	12 ¹⁴
VWC _i	(%)	27.7 ³³	30.5 ²⁰	29.4 ²⁹	28.6 ¹⁷	22.4 ¹²	33.2 ¹⁷	30.5 ¹³	26.6 ¹⁴
V_{tot}	(m ³)	0.7 ¹⁷	5.5 ⁸	6.9 ¹³	0.9 ¹²	2.4 ¹²	8.5 ⁶	10.6 ⁸	2.1 ⁹
MCA	(m ²)	39 ¹⁷	220 ⁸	162 ¹³	43 ¹²	97 ¹²	351 ⁶	234 ⁸	60 ⁹
$Q_{\Delta max}$	(l h ⁻¹)	59 ³⁴	33 ²⁰	68 ²⁸	72 ¹⁷	64 ¹³	39 ¹⁵	128 ¹²	47 ¹⁴
rising rate	(l h ⁻²)	20 ²⁸	4 ²¹	9 ²³	49 ¹⁵	25 ⁶	5 ¹⁵	63 ⁹	17 ¹¹
time lag	(min)	259 ³¹	496 ²⁰	550 ²⁸	186 ¹⁶	215 ¹²	440 ¹⁵	659 ¹²	276 ¹³
		fall	winter	spring	summer	fall	winter	spring	summer

** = non-triggering rainfall events
* = aggregated rainfall events

Figure 19. Seasonality of SSF and rainfall events characteristics. Values with a coloured background show the seasonal trimmed means, whereas the corresponding sample size (after trimming) is shown on a white background. For each metric, shading ranges from light to dark orange as values increase.

615 4 Discussion

4.1 Influence of total rainfall and antecedent conditions on contributing area and subsurface stormflow generation

The analysis has shown that at both trenches the event SSF volume (V_{tot}) is controlled by rainfall amount (P_{tot}) and antecedent wetness conditions (VWC_i and Q_i), with wet conditions producing one to three orders of magnitude larger events (V_{tot}) under similar P_{tot} (Figure 9 and Figure 10). These results are consistent with earlier studies reporting strong controls of rainfall amount and antecedent wetness on SSF volumes (e.g. Fu et al., 2013; Noguchi et al., 2001; Tromp-van Meerveld and McDonnell, 2006a). To account for the influence of the initial conditions on the generated stormflow volume, different authors have used the composite metric $P_{tot}+ASI$, which allowed to improve correlation strength and threshold detection (e.g. Detty and McGuire, 2010; Fu et al., 2013; Han et al., 2020; Zwartendijk et al., 2023). Nevertheless, ASI can over- or under-represent the initial soil moisture conditions in the composite metric $P_{tot}+ASI$, as it is highly dependent on the depth of the considered soil moisture profile; for the same soil moisture content, deeper profiles will yield higher ASI values, while shallower ones yield lower values. Therefore, as shown by Fu et al. (2013), the optimum depth to obtain the best fit can be site dependent. Hence, in this study we weighted the initially computed ASI in order to obtain the best fit to the datasets and explore the influence of the initial conditions. The observed higher correlation of V_{tot} vs $P_{tot}+ASI_{adj}$ than vs P_{tot} (supported by $CI_{\Delta rs} > 0$) highlights the considerable influence of the initial conditions on the total flow, and shows that the storage deficit, linked to drier antecedent conditions, was effectively accounted for using ASI_{adj} (see Figure 8). The correlation increased by 0.1 ($CI_{\Delta rs}$ 0.03–0.19) at T1 and by 0.2 ($CI_{\Delta rs}$ 0.10–0.43) at T3 when including ASI_{adj} . The larger increase in correlation and the slightly higher weight at T3 indicates that the initial conditions have a higher influence at that site. This is also suggested by the on average larger rainfall amounts of the non-triggering rainfall events observed at T3 (Figure 6).

Despite the importance of the antecedent conditions for SSF events associated with small to medium P_{tot} , the data shows that for SSF events associated with very large total rainfall depths ($P_{tot} > 50$ mm) the influence of the antecedent conditions is often less pronounced and the $P_{tot}-V_{tot}$ relationship is poor. These SSF events are characterized by a longer duration ($> 75^{th}$ percentile) and multiple peaks (subevents). Thus, the subsurface has more time to drain between peaks and more water is lost by the consecutive rainfall events to replenish the storage deficit and during the transmission to the trench. The higher losses and inadequacy of considering only the wetness conditions at the start of these events are also reflected by the comparatively smaller MCA (Figure 13) and poorer $P_{tot}+ASI_{adj}$ vs V_{tot} relationship. These results show the limitation of the applied approach for extremely large, long multi-peak SSF events. This finding matches the observations of Tromp-van Meerveld and McDonnell (2006a) where a poorer $P_{tot}-V_{tot}$ relationship and a weaker influence of the initial conditions is seen for SSF events

associated to $P_{\text{tot}} > 55$ mm. Hrnčič et al. (2010) also reported that for extremely large rainfall amounts (> 70 mm) the initial conditions only exerted a smaller control on the total subsurface stormflow.

645 Regarding smaller events, at both trenches, the minimum P_{tot} to trigger a SSF response (activation threshold) under wet antecedent conditions was minimal, ranging between 2 and 3 mm, and was only slightly higher for dry antecedent conditions (Figure 9 and Figure 10). The results show that, although the initial conditions greatly influence V_{tot} , they are less relevant concerning the activation of SSF. Other hillslope-based studies have also shown that small P_{tot} (< 5 –12 mm) are sufficient to activate SSF (e.g. Fu et al., 2013; Tromp-van Meerveld and McDonnell, 2006a) and that, although wet antecedent conditions
650 favour SSF activation and generate larger volumes, small P_{tot} are in some cases also sufficient to produce SSF under dry conditions (Noguchi et al., 2001).

At T1, beyond the SSF activation threshold of $P_{\text{tot}} \sim 2$ mm, especially events associated with wet antecedent conditions, show a nearly linear increase in V_{tot} with P_{tot} (Figure 9 and Figure 14). A similar linear increase, without any apparent further thresholds, was also observed at the catchment scale by Penna et al. (2011). In contrast, at T3, despite the general increase of
655 V_{tot} with P_{tot} , V_{tot} appears to increase only weakly over the ranges of ca. 5–15 mm (wet antecedent conditions) and ca. 10–20 mm (dry antecedent conditions), and to rise steeply once P_{tot} exceeds ca. 15 mm (wet) and ca. 20 mm (dry; Figure 10 and Figure 14). However, we note that the scatter and the relatively small number of events in the aforementioned P_{tot} ranges imply substantial uncertainty. The 15 mm and 20 mm rainfall amounts can be interpreted as a second threshold beyond which significantly more SSF is generated. The observations at T3 match the results of most SSF studies (e.g. Du et al., 2016; Fu et al., 2013; Tani, 1997; Tromp-van Meerveld and McDonnell, 2006a), where such a sharp increase in V_{tot} was also observed
660 once a precipitation threshold was exceeded. In the literature, the reported size of this second threshold, for the generation of significant SSF, under wet antecedent conditions ranges between 18–60 mm (Du et al., 2016; Mosley, 1979; Uchida et al., 2005; Weiler et al., 2006). Hence, our threshold of ca. 15 mm is located at the lower end of the spectrum of the reported values. Considering the composite variable $P_{\text{tot}} + \text{ASI}_{\text{adj}}$, the second threshold was at ca. 40 mm at T3 (Figure 11). This suggests that
665 significant SSF volumes can be generated by a relatively small rainfall event if the antecedent conditions are very wet (i.e. ASI_{adj} is close to 40 mm), whereas if the antecedent conditions are dry ($\text{ASI}_{\text{adj}} \ll 40$ mm) higher rainfall amounts are required to generate comparable SSF volumes. For example, a similar V_{tot} of 0.63–0.71 m³ resulted from a relatively small P_{tot} of 6 mm under wet initial conditions (ASI_{adj} ca. 37 mm), as well as from a large P_{tot} of 28 mm under dry initial conditions (ASI_{adj} ca. 17 mm). The initial conditions also influenced the MCA, as for similar P_{tot} the MCA was generally higher under wet antecedent
670 conditions (Figure 13). The strong fit of Eq. 4 suggests that the contributing area expands with P_{tot} , and the clear difference between events with wet and dry antecedent conditions indicates that the contributing area increases more rapidly with P_{tot} under wet than under dry conditions. For events with wet antecedent conditions, the quadratic coefficient a (Eq. 4) was markedly larger at T3 than at T1, implying a stronger rainfall dependence and a faster expansion of the contributing area at T3. The increase of the contributing area with P_{tot} is backed by field observations, which suggest that the contributing area can
675 be much smaller than the maximum MCA. It was observed how for some smaller events at T1 and T3, the uppermost instrumented well located roughly 20 m above the trench did not show any reaction (i.e. no increase in water level), suggesting that for these cases the contributing area probably ended downslope of the well and was therefore smaller than the calculated maximum MCA. However, we cannot fully exclude that water could have laterally bypassed the uppermost well without producing a measurable response in the water level; hence, the contributing area may have extended upslope of the well despite
680 the lack of a water level response. The increase in contributing area with storm size and wet antecedent conditions is in agreement with other studies at the hillslope scale (e.g. Šanda and Císlarová, 2009; Tromp-van Meerveld and McDonnell, 2006b, a) as well as at the catchment scale (e.g. Detty and McGuire, 2010; Zwartendijk et al., 2023). The dynamic nature of the subsurface contributing area and the difficulty to assess it, implies that converting the volumetric (hillslope) discharge into specific discharge (i.e. mm), which requires the use of a fixed catchment area, can lead to unrepresentative and misleading

685 specific discharge values. Hence, in this study, the volumes were not converted into mm. Nevertheless, we provide the topographically derived catchment area as an indicative reference area in Section 2.1.

Our findings support that, for small events, especially if characterized by dry antecedent conditions, the contributing area tends to be small, as an expansion further upslope is likely restricted by transmission losses (i.e. leakage through the impeding layer) and/or storage deficits (i.e. “storage barriers”), which are overcome during larger precipitation events. Studies have shown that these storage barriers can result, among other factors, from the spatial variation of soil thickness, the topography of the impeding layer (i.e. depression storage) and the vertical variation in hydraulic conductivity, which controls the leakage rate through the impeding layer as well as the possible onset of faster lateral flow within upper more permeable layers (McGuire et al., 2024). Moreover, low soil moisture levels represent an additional barrier that needs to be overcome (Penna et al., 2011). These barriers can increase gradually or abruptly, as suggested by the gradual and sharper increase of V_{tot} as well as MCA with P_{tot} observed at T1 and T3, respectively (Figure 9, Figure 10 and Figure 13). At T3, the phase of markedly reduced increase of V_{tot} with P_{tot} between ca. 5–15 mm (wet antecedent conditions) and 10–20 mm (dry antecedent conditions) suggests that, at this site, a storage barrier of ca. 10 mm prevents further expansion of the contributing area (see Figure 10). This barrier, which appears to be present during wet and dry initial conditions, may be the result of depressions on the irregular bedrock topography, which have to be filled before any significant later flow originating from areas further upslope can occur (fill and spill mechanism). This is supported by the steep topography and spatial distribution of the outcrops observed in the field. Nevertheless, other mechanisms cannot be ruled out, and we acknowledge that the small number of events within the reported P_{tot} ranges limits the robustness of this interpretation.

At T1 the continuous baseflow and shallow water table suggest that at the site SSF is generated by the water table rising into a more permeable zone near the surface (transmissivity feedback mechanism). Thus, the gradual increase of V_{tot} with P_{tot} can be explained by a gradual increase in storage losses and/or vertical losses further upslope.

4.2 Controls on peakflow

Our results show that $Q_{\Delta\text{max}}$ is mainly controlled by P_{tot} and antecedent soil moisture conditions (VWC_i) for events associated with rainfall events smaller than approximately 20 mm (Figure 15). Whereas for larger amounts of rainfall, while P_{tot} is still important, the initial conditions play a subordinate role and rainfall intensity as well as the amount of rainfall preceding peak rainfall intensity ($P_{\Sigma\text{peak}}$) appear to be important factors. The switch from initial conditions to rainfall intensities and/or $P_{\Sigma\text{peak}}$ as controlling factors is not abrupt but gradual, especially at T1 where the influence of the initial conditions is less pronounced than at T3. The strong influence of the initial conditions on $Q_{\Delta\text{max}}$ is reflected by the increase in correlation (corroborated by $\text{CI}_{\Delta\text{rs}}$) when ASI_{adj} is added to P_{tot} (see Figure 8). The increase and the adjustment factor of ASI (i.e. weight of the ASI) are particularly high at T3, supporting the larger influence of the initial conditions at the site. The relatively weak (T1) and extremely weak (T3) correlation coefficients between $Q_{\Delta\text{max}}$ and the rainfall intensities confirm that the intensities alone are not sufficient to explain $Q_{\Delta\text{max}}$. The somewhat higher values observed for T1 can be explained by the combined effect of the higher $V_{\text{tot}}-P_{\text{tot}}$ correlation and the correlation present between P_{tot} and the rainfall intensities. Moreover, at T1 the smaller influence of the antecedent soil moisture conditions suggests a smaller soil moisture storage, which being more easily filled up allows the rainfall intensity to exert a larger influence. The stronger influence of the initial conditions observed for smaller rainfall events can be explained by the larger fraction of water lost to fill the storage deficit of the soil. Only when the storage is filled and the contributing area is large are higher rainfall rates able to generate and sustain higher subsurface flowrates, and thus exert a larger control on $Q_{\Delta\text{max}}$. This is also backed by the higher $Q_{\Delta\text{max}}$ generated by events with moderate I_{30} but high precipitation amounts preceding peak rainfall.

Other authors observed similar relationships between peakflow, storm size, rainfall intensities and antecedent conditions. Noguchi et al. (2001) documented an increase in subsurface peakflow with I_{30} for events with wet conditions, while during dry conditions the relationship was less strong and only present for part of the observed trenchface. Hrnčíř et al. (2010) also found

that the initial soil moisture had a major control on the subsurface peakflow. Moreover, Fu et al. (2013) observed that beyond a threshold of I_{30} of ca. 6 and 12 mm/h (two trenches) the I_{30} correlated to the subsurface peakflow. However, two events with high antecedent soil moisture showed high peakflows even with low I_{30} (below threshold). At the catchment scale, Zwartendijk et al. (2023) observed how peakflow was highly correlated to P_{tot} +ASI as well as with V_{tot} and that while peakflow was also influenced by I_{60} the P_{tot} and antecedent conditions were the major controlling factors. These studies suggest that, like at our trench sites, rainfall intensity has a lower influence under dry initial conditions but once the storage deficit is filled and the contributing area is large high rainfall intensities can generate high trenchflow rates especially when associated with large P_{tot} .

4.3 Timing and rising rate

Our findings show how the rising rate is positively correlated to P_{tot} as well as rainfall rate (I_m , I_5 , I_{30} and I_{60}) (Figure 17 and Figure 8). The relationship with P_{tot} is roughly exponential, although the scatter is relatively large in part due to the different rainfall intensities. Dry initial conditions seem to not hinder the generation of steep rising limbs but, in some instances, to favour it (observed at T1 but not at T3).

The lag time (centroid rainfall–peakflow) is short for events with high rising rates but greatly varies for events with lower rising rates, resulting in a negative correlation between rising rate and lag time (Figure 8 and Figure 18). Similar results were found when the time between peak rainfall and peakflow was considered. The link between short lag times and steep rising limbs suggest that short travel times reduce the dispersion of the rainfall pulse traveling through the subsurface allowing for a sharper increase in discharge.

The lag time was found to be not correlated to the size of the rainfall event but to its intensity. The correlation between lag time and intensities is negative (see Figure 8), with the lag time variability decreasing as rainfall intensities increase. For example, at T1 and T3 events with I_{30} exceeding 15 mm/h had a standard deviation of the lag time seven and four times smaller than events with lower intensity, respectively. At the catchment scale, similar observations were made by Zhang et al. (2021). Knapp et al. (2025), also found that the lag time was shortened by higher precipitation intensities and was largely unaffected by antecedent wetness conditions. Our observations also align with the findings of Han et al. (2020), as they report that the time required to generate connectivity in the hillslope-riparian-stream system, and hence produce significant runoff, was related to the peak rainfall intensity (I_{30}), but not the accumulated rainfall amount. Moreover, they observed how dry initial conditions can favour connectivity within the catchment. In agreement, we found that short lag times were often associated with dry initial conditions. Shorter lag times were also associated with smaller MCA (Figure 8), especially under dry antecedent conditions. Similarly, Haga et al. (2005) observed how, at the catchment scale, the lag time increased with the increasing source area.

Our results suggest that dry conditions can favour preferential flow, which allows the pulse to travel more quickly through the subsurface bypassing the dry matrix. Evidence of preferential flow in our study catchment was found by Pyschik and Weiler (2025). Furthermore, dry conditions were linked to smaller MCA and thus shorter flow paths, which in turn promote a shorter travel time and hence smaller lag times. The reduced lag time linked to higher intensities can also be explained by the associated faster wetting (assuming rainfall intensity < infiltration capacity) and thus a quicker arrival of the main rainfall pulse to the trench.

The response time (centroid rainfall–start SSF) showed to be poorly related to P_{tot} as well as to the cumulative rainfall calculated up to the start of the SSF event. Nevertheless, the variability in response time shows a decrease with increasing rainfall amounts preceding the SSF start. Similarly, higher rainfall intensities were associated with a lower variance in response time. Overall, the relationship between response time, rainfall event characteristics and antecedent conditions is not well defined. Comparable (short) response times occurred under different combinations of rainfall characteristics and antecedent wetness. For example, events with low intensities and small rainfall amounts produced responses that were as fast as those of large and intense rainfall events under dry conditions, indicating that response time is not uniquely determined by rainfall

amount or intensity. Moreover, short response times were occasionally observed even for events with small rainfall amounts under dry antecedent conditions. The timing of SSF can be controlled by a multitude of factors (e.g. soil depth, bedrock topography, macropore and preferential flow network, slope, permeability contrast etc.) whose relevance can change depending on antecedent conditions, rainfall characteristics and SSF-producing mechanism (e.g. saturated flow over impeding layer, macropore flow, flow in higher permeability zone) (McGuire et al., 2024). Besides these complexities, the way the response time is defined can also lead to a large variation in response times. For example, the time can be calculated from the centroid or the peak rainfall, which in turn are influenced by the method and thresholds (e.g. MIT 6 h) used to define the rainfall events, but also by the particular structure of the rainfall events. Using the start of the rainfall event is also problematic, as the time will be highly influenced by the rainfall structure. The result is that only a handful of authors have studied the response time and its relationship with rainfall as well as hillslope/catchment characteristics. The large range in the response time and its variability within the hillslope was documented by Freer et al. (2002), who, for example, described how a certain trench section responded ca. 4 times slower than the fastest one. An even higher variability was found at the catchment scale in the study of Zwartendijk et al. (2023), who also observed that the response time (start rainfall to start stormflow) was poorly to not significantly correlated to the antecedent conditions.

4.4 Seasonal variation of SSF

The results showed a marked seasonal variation of the SSF response (Figure 19). The observed pattern can be linked to the seasonal variation of the rainfall characteristics and antecedent conditions, which ultimately control the SSF response. For instance, the great V_{tot} and MCA in winter and spring can be explained by the combined high precipitation amounts and wet initial conditions. Similarly, at the catchment scale, Penna et al. (2015) reported a strong seasonal variability in runoff responses related to antecedent moisture conditions and rainfall event size, with runoff coefficients being markedly higher in spring than in summer. Sidle et al. (1995) documented greater runoff coefficients during the wet season, which in combination with larger precipitation events, produced greater SSF volumes at the hillslope scale. Likewise, Tromp-van Meerveld and McDonnell (2006a) reported greater trenchflow runoff coefficients in winter than in summer. Blume et al. (2009) observed shorter response times for streamflow in summer compared to winter, consistent with our findings of much smaller lag times in summer compared to the wetter winter and spring.

Given the relevance of the antecedent wetness conditions, many SSF studies simply divide the dataset into wet and dry periods or into growing (typically dry) and dormant (typically wet) seasons (e.g. Detty and McGuire, 2010; Sidle et al., 1995). However, the seasonal variability in SSF is rarely examined explicitly, as most SSF studies are based on short observation periods and thus only cover a limited number of events. Our study highlights the importance of investigating SSF over extended time periods in order to capture the responses across seasons and, ultimately, a wide range of rainfall characteristics and antecedent conditions.

5 Conclusions

In our study, subsurface stormflow (SSF) volume was primarily controlled by total rainfall and antecedent wetness conditions, with wet initial conditions yielding up to three orders of magnitude more volume. Under wet antecedent conditions, the rate at which the total event SSF volume increased with total rainfall clearly differed between the two trenched hillslopes. Our data suggest that the observed increase in event SSF volume with total rainfall is associated with an expansion of the contributing area, as indicated by the increase in the minimum contributing area. Peakflow was mainly controlled by total rainfall and initial wetness conditions for small rainfall events, whereas for larger events the influence of the initial conditions diminished, and rainfall intensity as well as rainstorm structure became more important controlling factors. Steep rising limbs were related with short response times and occurred under higher rainfall amounts and intensities. Using a weighted antecedent soil moisture

index (ASI) in the combined variable total rainfall+ASI helped to quantify the influence of initial soil moisture conditions relative to the total rainfall on the different SSF response metrics.

Our results highlight the complex interactions between rainfall characteristics, antecedent wetness conditions and SSF responses, which could only be captured by analysing many events across seasons, underscoring the importance of long observation periods. The rapid SSF response and the large volumes and flow rates observed at our 11–14 m wide trenches, coupled with their proximity to the stream, suggest that SSF can substantially contribute to streamflow. Thus, our findings can serve both as a benchmark to verify hillslope-scale hydrological models and as a proxy for predicting the occurrence and magnitude of SSF in catchment-scale models. Further studies at the trench sites focusing on the inter-event dynamics of soil moisture, water level, trenchflow and its partitioning as well as artificial rainfall experiments will help to shed more light on the complex processes controlling SSF at the hillslope scale.

6 Appendix A



Figure A1. Trenches during construction

7 Appendix B

The volumetric water content (VWC) was derived from the dielectric permittivity and temperature data using the Complex Refraction Index Model (CRIM) (Eq. B1) which accounts for the individual permittivities of air (K_{air}), solid (K_s) and water (K_{water}) (Roth et al., 1990). In the equation, K_a is the measured (apparent) dielectric permittivity while n and β are the porosity and shape factor, respectively.

$$\text{VWC} = 100 \times \frac{K_a^\beta - (1 - n) \times K_s^\beta - nK_{\text{air}}^\beta}{K_{\text{water}}(T)^\beta - K_{\text{air}}^\beta} \quad (\text{B1})$$

While the effect of temperature on K_{air} and K_s is negligible, on K_{water} it is not (Wraith and Or, 1999). Therefore, if temperature is not accounted for when calculating the VWC (e.g. using the Topp equation; Topp et al., 1980), diurnal temperature cycles and seasonal temperature variations can lead to erroneous VWC values. This was avoided by applying the CRIM with a

830 temperature-corrected K_{water} . The temperature dependence of K_{water} was taken into account using Eq. B2 (Weast, 1986); where T° is the temperature in degree Celsius.

$$K_{\text{water}} = 78.54 \times [1 - 4.579 \times 10^{-3}(T^\circ - 25) + 1.19 \times 10^{-5}(T^\circ - 25)^2 - 2.8 \times 10^{-8}(T^\circ - 25)^3] \tag{B2}$$

Nevertheless, an accurate temperature-corrected VWC can only be obtained if the CRIM-variables porosity (n), K_s and β are reasonably well known, as those variables have a considerable influence on the resulting VWC. Hence, by applying the CRIM the temperature effect can be accounted for but the uncertainty on the absolute VWC values is related to the uncertainty of K_s and, more importantly, porosity (n).

In this study, K_s was assumed to be 4, since for mineral soils it typically ranges between 3 and 5 (Behari, 2005; Chan and Knight, 1999; Kirsch, 2009). A β of 0.5 was used, as this is the most commonly used value (Zadhoush et al., 2021), and K_{air} was set to 1. Where available, the porosity measured in the lab (from fitted water retention curve obtained from HYPROP 2 and WP4C (METER-Group) measurements) for the undisturbed soil samples taken near the sensor location was considered.

840 However, soil samples were only collected along the deepest soil moisture profile (S3), thus for the shallower profiles (S1 and S2) the porosities were estimated based on the one measured at S3. The lab-measured porosities were adjusted when the max. VWC resulting from the Topp and from the CRIM equation (computed using the original lab-measured porosities) were substantially higher than the measured porosities (e.g. 41% vs 25% for the 2 m BGL sensor at T3 S3; see Table A1). Since VWC values greater than the porosity do not make physically sense, the porosity was adjusted using the max. observed VWC

845 (Topp) as a guideline and ensuring that the max. CRIM-calculated VWC approximated the adjusted porosity, under the assumption that during saturated or near-saturated conditions the VWC approximates porosity. The porosities used for the final calculation are shown in Table A1. Differences between lab-measured porosities and those used to calculate soil moisture with the CRIM approach may result from spatial offsets between sampling and sensor locations, small-scale heterogeneity, and installation-related disturbance in gravel-rich deposits, which may locally modify the porosity within the sensor's

850 measurement volume.

Table A1. Porosity at the sensor’s location used to calculate the soil moisture (CRIM)

T1				T3			
	S1	S2	S3		S1	S2	S3
depth (m BGL)	porosity (-)	porosity (-)	porosity (-)	depth (m BGL)	porosity (-)	porosity (-)	porosity (-)
0.1	0.57	0.57	0.57*	0.1	0.54	0.54	0.54*
0.3	0.52	0.52	0.52*	0.3	0.51	0.51	0.51*
0.6	0.42	0.45	0.34*	0.6	0.55	0.52	0.52*
1.5			0.32	1.0			0.42
2.0			0.42	1.5			0.36 (0.29*)
2.5			0.42	2.0			0.41 (0.25*)

* = lab-measured porosities
(not used porosity)

8 Appendix C

The SSF events were delineated using a semi-automatic method developed within this study. In the following, the method is described in detail, including inputs, outputs, implemented steps and variables involved.

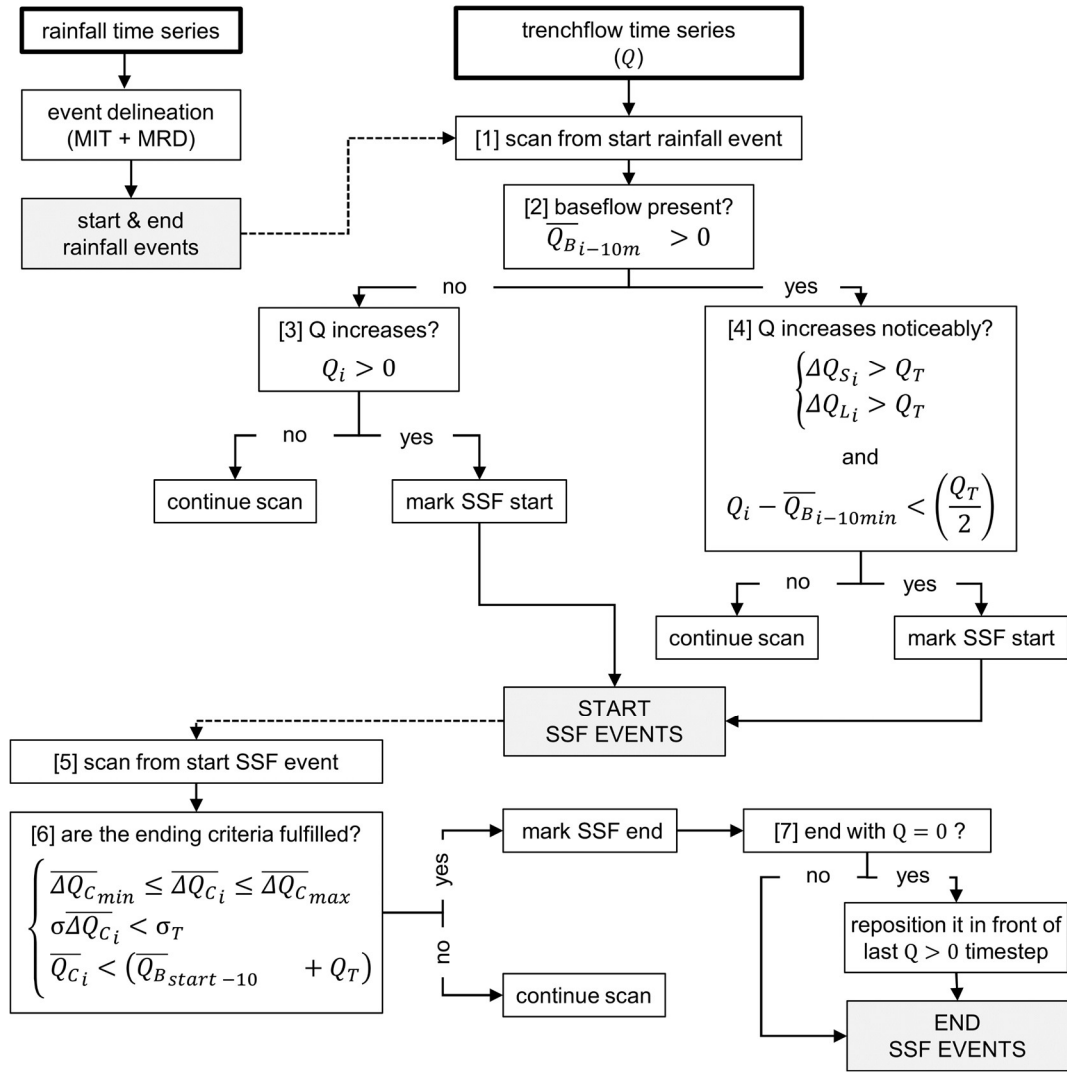


Figure C1. Flow chart of the subsurface stormflow event delineation method

The rainfall timeseries must have a temporal resolution of 5 or 10 min and be previously separated into events (see Section 2.3.2 Rainfall event delineation), as the start and end of the events serve as input. The trenchflow record must have, or be resampled to, a time resolution of 10 min. The method was written using MATLAB (The MathWorks Inc, 2021) and the commented sourcecode is available upon request.

The flowchart in Fig. C1, outlines the steps taken to detect the start and end points of the events, whereas Fig. C2 and Fig. C3 further illustrate the moving windows and criteria used in the process. The start is detected as follows:

[1] The time series is scanned from the start to the end + 6 h of *every* rainfall event to evaluate if an “unusual” increase in discharge (Q), which indicates the start of an SSF event, occurs. Six hours are chosen due to the defined MIT of 6 h.

[2] During the scan, it is tested if baseflow, computed using a 5 h backward moving average ($\overline{Q}_B$) of the trenchflow rate (Q) (i.e. arithmetic average between Q_i and $Q_{i-300min}$), is present. This is:

$$\overline{Q}_{Bi-10min} > 0 \quad (C1)$$

[3] If no baseflow is detected, the algorithm tests the time series for flow onset; this is:

$$Q_i > 0 \quad (C2)$$

- If an increase is found, the position is marked as the start of a new event and the scanning stops;
- else, the scanning continues up to the end of the rainfall event + 6 h.

[4] If baseflow is present, the code evaluates if there is any significant (i.e. “unusual”) increase in discharge by: calculating short (10 min) and long (60 min) forward moving averages ($\overline{Q_{FS}}$ and $\overline{Q_{FL}}$) (i.e. arithmetic average from Q_i to $Q_{i+10min}$ and Q_i to $Q_{i+60min}$, respectively), computing the differences between $\overline{Q_B}$ of the previous timestep and $\overline{Q_{FS}}$ as well as $\overline{Q_{FL}}$

$$\Delta Q_{S_i} = \overline{Q_{FS_i}} - \overline{Q_{B_{i-10min}}} \quad (C3)$$

$$\Delta Q_{L_i} = \overline{Q_{FL_i}} - \overline{Q_{B_{i-10min}}} \quad (C4)$$

and evaluating if ΔQ_S and ΔQ_L are both above a certain threshold Q_T

$$\begin{cases} \Delta Q_{S_i} > Q_T \\ \Delta Q_{L_i} > Q_T \end{cases} \quad (C5)$$

as well as testing if

$$Q_i - \overline{Q_{B_{i-1}}} > \left(\frac{Q_T}{2}\right). \quad (C6)$$

The latter avoids that the start is defined one timestep too early (i.e. in cases where Q_i is similar to $\overline{Q_{B_{i-10}}}$ but due to the high value of $Q_{i+10min}$ the criterion in Eq. C5 is fulfilled), as the start has to coincide with the “unusual” increase in Q .

- If the above criteria (Eq. C5 and C6) are not fulfilled the scanning continues;
- else, the time step is marked as the start of a new event and the scanning stops.

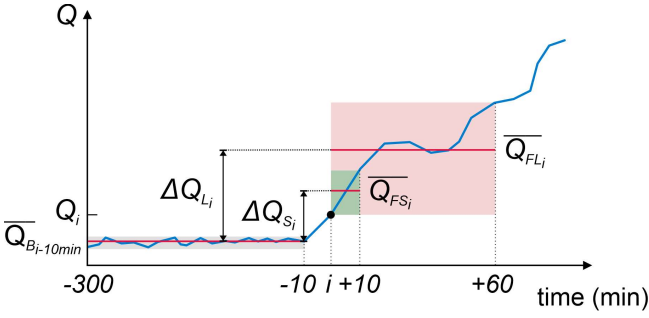


Figure C2. SSF start detection method when baseflow is present

Once the starting points of the events are defined, the ending points, which have to be followed by an “uneventful” period (i.e. a period associated with negligible changes), are detected as follows:

[5] The time series is scanned from each previously detected starting point.

[6] The change of the smoothed discharge ($\overline{Q_C}$), calculated using an 80 min centered moving average (i.e. arithmetic average between $Q_{i-40min}$ and $Q_{i+40min}$), is computed over a 4 h period ($\Delta \overline{Q_C} = \overline{Q_{C_{i+240min}}} - \overline{Q_{C_i}}$). The time series of differences $\Delta \overline{Q_C}$ is then further smoothed using a 20 min centered average approach (resulting in $\overline{\Delta \overline{Q_C}}$). Next, the standard deviation of $\overline{\Delta \overline{Q_C}}$ is calculated over a 4 h forward window (i.e. $\sigma \overline{\Delta \overline{Q_C}}$ is equal to the sd. between $\overline{\Delta \overline{Q_C}_i}$ and $\overline{\Delta \overline{Q_C}_{i+240min}}$). Finally, the end of an event is marked when the following three criteria are fulfilled:

$$\begin{cases} \overline{\Delta \overline{Q_C}_{min}} \leq \overline{\Delta \overline{Q_C}_i} \leq \overline{\Delta \overline{Q_C}_{max}} \\ \sigma \overline{\Delta \overline{Q_C}_i} < \sigma_T \\ \overline{Q_{C_i}} < (\overline{Q_{B_{start-10min}}} + Q_T) \end{cases} \quad (C7)$$

where $\overline{\Delta \overline{Q_C}_{min}}$ and $\overline{\Delta \overline{Q_C}_{max}}$ are threshold-values that limit the acceptable change in the discharge over the next 4 h, while the threshold σ_T quantifies the acceptable variation of the discharge withing the same 4 h period. Setting low threshold values ensures that in the period following potential ending points Q does not increase nor decrease substantially. Low σ_T values in particular help to discard periods with fast-changing Q and to detect periods with well-defined trends. Due to the

methodology a period of ca. 4 h from every timestep is analysed. To avoid that high “plateaus” within an event are erroneously detected as ending points the discharge value ($\overline{Q_C}$) of the ending point has to be lower than the baseflow at the start of the event ($\overline{Q_{B_{start-10min}}} + Q_T$) (threshold also used for the start detection) (i.e. it has to be less than Q_T above the baseflow). The addition of Q_T avoids that extremely shallow recession limbs hinder the detection of event ends within a reasonable time and that new slightly higher baseflows are not readily discarded for the placement of an ending point.

[7] Lastly, because the average-based method makes it difficult to pinpoint the end of an event when Q alternates between flow and no-flow conditions, the code scans backwards from the detected end point until Q is greater than zero. Once the condition is met, the endpoint is moved in front of the identified non-zero Q time step.

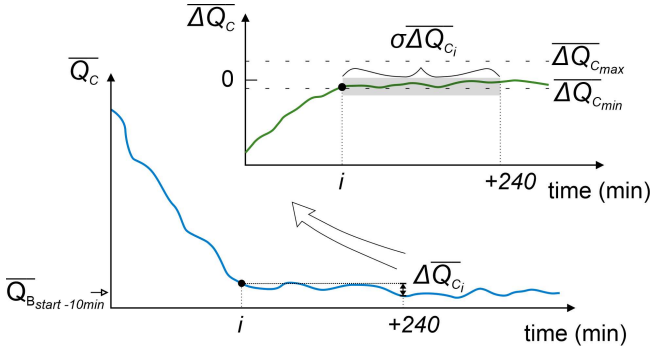


Figure C3. SSF end detection method

The above-described method was applied to the measured trenchflows using following parameters: $Q_T = 1.0$ l/h, $\sigma_T = 0.5$ l/h, $\overline{\Delta Q_{C_{max}}} = 0.3$ l/h and $\overline{\Delta Q_{C_{min}}} = -0.05$ l/h. The chosen values obviously affect the number as well as the placement of the detected starting and ending points. Therefore, they have to be carefully selected according to the observed trenchflow signal. The optimal value of Q_T and σ_T will depend, among other things, on the resolution of the flow gauge and on the amplitude of the signal noise. In this study the flow was measured using tipping bucket systems. Therefore, Q_T and σ_T were related to the flow equivalent to 1 tip/10 min which equalled to ca. 0.5 l/h.

The delineation results showed that most of the detected starts and ends of the events appear, after a visual inspection, at acceptable locations. Nevertheless, a small portion of them was placed at inappropriate locations. In fact, the method struggles to correctly locate the start when the increase in Q is very small and of similar magnitude as the signal noise, or when the start of a new event occurs during the steep recession phase of the preceding event. The latter arises due to the 5 h backward time window used to compute the “baseflow”, as higher discharge values of the falling limb would then also be included in the time window. Misplacements of ending points, on the other hand, can arise when the recession is more noisy than usual or when the new baseflow is considerably higher than the one preceding the event.

Hence, after the automatic delineation, the results had to be manually checked and were modified if necessary. This meant adding or removing a starting point as well as adjusting the position of starting and ending points. For this reason, the method is considered to be semi-automatic. To get an idea on the performance, for T1 and T3 on average ca. 4% of the starts had to be added, 15% had to be removed and 12 % of the starts had to be re-positioned, while 21% of the ends had to be re-positioned. The mentioned lengths of the periods used to calculate the averages, differences, etc., were chosen to best fit the analysed trenchflow data. Nevertheless, for different scales and other hydrological and hydrogeological settings the lengths of these periods could be adjusted.

	P_{tot}		I_m		I_5		I_{30}		I_{60}		V_{tot}		$\bar{Q}$		MCA	
P_{tot}			0.23	0.54	0.23	0.54	0.5	0.73	0.57	0.77	0.71	0.9	0.64	0.86		
I_m	0.42	0.74			0.52	0.75	0.63	0.8	0.65	0.83	-0.2	0.33	-0.04	0.4	-0.33	0.19
I_5	0.42	0.74	0.48	0.77			0.92	0.96	0.82	0.91	-0.28	0.27	-0.27	0.28	-0.47	0.07
I_{30}	0.56	0.81	0.60	0.84	0.92	0.97			0.95	0.98	-0.17	0.36	-0.12	0.39	-0.33	0.17
I_{60}	0.60	0.85	0.68	0.88	0.83	0.94	0.95	0.98			-0.14	0.39	-0.1	0.44	-0.3	0.23
V_{tot}	0.52	0.83	-0.18	0.44	-0.59	0.10	-0.26	0.37	-0.22	0.39			0.9	0.97		
$\bar{Q}$	0.47	0.80	-0.20	0.45	-0.56	0.11	-0.27	0.39	-0.24	0.37	0.92	0.98			0.84	0.95
MCA			-0.25	0.36	-0.66	-0.06	-0.36	0.25	-0.32	0.27			0.87	0.97		
$Q_{\Delta max}$	0.16	0.61	-0.01	0.48	-0.35	0.29	-0.13	0.40	-0.09	0.43	0.74	0.95	0.80	0.98	0.68	0.92
rising rate	0.39	0.77	0.26	0.73	0.02	0.63	0.15	0.66	0.22	0.70	-0.36	0.77	-0.35	0.76	-0.37	0.70
lag time	-0.38	0.10	-0.48	-0.01	-0.72	-0.23	-0.53	-0.03	-0.54	-0.08	0.10	0.66	0.04	0.61	0.15	0.70
response time	-0.56	-0.16	-0.35	0.07	-0.32	0.19	-0.36	0.13	-0.35	0.13	-0.54	0.18	-0.51	0.26	-0.46	0.26
VWC_i	-0.62	-0.22	-0.58	-0.19	-0.82	-0.57	-0.68	-0.29	-0.68	-0.31	0.37	0.74	0.37	0.81	0.45	0.79
Q_i	-0.60	-0.20	-0.53	-0.10	-0.80	-0.45	-0.65	-0.25	-0.65	-0.27	0.33	0.75	0.32	0.77	0.44	0.80
$P_{tot}+ASI$											0.70	0.92	0.69	0.91		
$P_{tot}+ASI_{adj}$											0.90	0.97	0.84	0.95		

	$Q_{\Delta max}$		rising rate		lag time		response time		VWC_i		Q_i		$P_{tot}+ASI$		$P_{tot}+ASI_{adj}$	
P_{tot}	0.72	0.87	0.53	0.75	-0.41	-0.04	-0.72	-0.52	-0.32	0.02	-0.3	0.04				
I_m	0.18	0.52	0.47	0.74	-0.66	-0.36	-0.2	0.18	-0.45	-0.12	-0.43	-0.1				
I_5	0.12	0.5	0.35	0.73	-0.65	-0.29	-0.29	0.06	-0.71	-0.44	-0.72	-0.45				
I_{30}	0.31	0.61	0.49	0.78	-0.66	-0.39	-0.38	-0.03	-0.54	-0.24	-0.54	-0.23				
I_{60}	0.37	0.66	0.55	0.79	-0.66	-0.41	-0.45	-0.06	-0.52	-0.19	-0.51	-0.2				
V_{tot}	0.68	0.9	-0.33	0.35	0.04	0.6	-0.76	-0.28	0.09	0.5	0.09	0.57	0.5	0.84	0.86	0.96
$\bar{Q}$	0.72	0.93	-0.33	0.36	-0.05	0.55	-0.8	-0.38	0.12	0.54	0.14	0.63	0.48	0.84	0.78	0.94
MCA	0.37	0.81	-0.52	0.17	0.23	0.69	-0.68	-0.11	0.28	0.65	0.33	0.71				
$Q_{\Delta max}$					-0.42	-0.06	-0.75	-0.56	0.05	0.38			0.23	0.56	0.82	0.92
rising rate					-0.84	-0.67	-0.46	-0.12	-0.37	0.04			-0.23	0.16	0.52	0.74
lag time	-0.08	0.45	-0.70	-0.25			-0.14	0.26	0.08	0.45	0.06	0.43	0.04	0.42	0.03	0.4
response time	-0.64	-0.23	-0.46	0.07	-0.12	0.49			-0.39	-0.04	-0.36	-0.04	-0.52	-0.19	-0.75	-0.56
VWC_i	0.14	0.60	-0.47	0.09	0.09	0.61	-0.38	0.07			0.9	0.95				
Q_i					-0.03	0.48	-0.39	0.02	0.75	0.94						
$P_{tot}+ASI$	0.28	0.70	-0.31	0.25	0.10	0.58	-0.47	0.02								
$P_{tot}+ASI_{adj}$	0.79	0.92	0.43	0.79	0.06	0.57	-0.60	-0.22								

T1T3

Figure D1. Lower and upper 95% confidence intervals of the Spearman rank correlations presented in Figure 8. The confidence intervals were computed using a bootstrap approach (1000 resamples, percentile method; see Section 2.3.8).

		T1			
** P_{tot}	(mm)	1.0 (0.6–1.5)	0.9 (0.6–1.1)	1.1 (0.6–1.7)	1.7 (1.0–2.5)
P_{tot}	(mm)	9.8 (7.7–12.2)	5.4 (4.2–7.3)	8.9 (6.7–11.4)	11.4 (8.9–16)
* P_{tot}	(mm)	13 (9–22)	16 (8–26)	19 (9–39)	18 (10–32)
I_{30}	(mm h ⁻¹)	6 (4–7)	2 (2–3)	5 (4–7)	12 (9–19)
VWC _i	(%)	27.7 (27.0–28.5)	30.5 (30.2–31.0)	29.4 (28.9–29.9)	28.6 (27.4–29.6)
V_{tot}	(m ³)	0.7 (0.2–4.8)	5.5 (1.0–12.3)	6.9 (0.9–18.4)	0.9 (0.3–4.2)
MCA	(m ²)	39 (18–105)	220 (87–388)	162 (65–312)	43 (23–93)
$Q_{\Delta max}$	(l h ⁻¹)	59 (30–115)	33 (18–53)	68 (29–132)	72 (44–138)
rising rate	(l h ⁻²)	20 (11–37)	4 (2–6)	9 (4–30)	49 (17–231)
time lag	(min)	259 (159–365)	496 (440–558)	550 (435–688)	186 (66–334)
		fall	winter	spring	summer

		T3			
** P_{tot}	(mm)	3.4 (2.4–4.7)	1.1 (0.6–2.8)	1.9 (1.2–2.9)	4.2 (2.8–6.0)
P_{tot}	(mm)	12.1 (8.5–16.4)	5.2 (4.1–6.8)	11.4 (7.5–15.6)	12.6 (9.6–17.6)
* P_{tot}	(mm)	18 (10–30)	11 (5–21)	27 (15–43)	24 (13–45)
I_{30}	(mm h ⁻¹)	6 (3–10)	2 (2–3)	6 (4–10)	12 (9–21)
VWC _i	(%)	22.4 (19.6–26.0)	33.2 (32.8–33.6)	30.5 (29.2–32.0)	26.6 (22.4–30.3)
V_{tot}	(m ³)	2.4 (0.1–12.6)	8.5 (0.2–25.6)	10.6 (2.9–32.6)	2.1 (0.1–9.6)
MCA	(m ²)	97 (11–266)	351 (25–1113)	234 (98–748)	60 (3–175)
$Q_{\Delta max}$	(l h ⁻¹)	64 (3–182)	39 (18–92)	128 (43–260)	47 (16–107)
rising rate	(l h ⁻²)	25 (4–316)	5 (3–8)	63 (10–203)	17 (7–50)
time lag	(min)	215 (94–533)	440 (308–784)	659 (286–1210)	276 (120–543)
		fall	winter	spring	summer

** = non-triggering rainfall events
* = aggregated rainfall events

Figure E1. Lower and upper 95% confidence intervals of the seasonal trimmed means presented in Figure 19. The confidence intervals were computed using a bootstrap approach (1000 resamples, percentile method).

11 Code and data availability

940 The data is available at <https://doi.org/10.48323/89ts5-vg424> (Thoenes, 2025).
The code will be made available upon request.

12 Author contribution

The study was planned and designed by TB, MW, LH, BK, SA; ET contributed to the methodological development, processed, analysed and visualised the data, and wrote the first draft of the manuscript. All authors contributed to the interpretation of the data and the editing of the paper.

13 Competing interests

MW is a member of the editorial board and TB is Chief Executive Editor of the journal Hydrology and Earth System Sciences.

14 Acknowledgments

We thank all the members of the research unit “Fast and Invisible: Conquering Subsurface Stormflow through an Interdisciplinary Multi-Site Approach” (RU 5288) who helped in the construction of the trenches and instrumentation of the hillslopes as well as the technicians of Uni Freiburg (Department of Hydrology), in particular Britta Kattenstroth, for ensuring

the smooth running of the operation on site. This research has been funded by the German Research Foundation (DFG) and the Austrian Science Fund (FWF) (10.55776/I5940).

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