

1           **Modeling surface water and groundwater mixing and mixing-dependent**  
2                                   **denitrification with bedform dynamics**

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**Notation**

$S$	<u>Stream slope [-]</u>
$H$	<u>Water depth [L]</u>
$U$	<u>Stream velocity [L T<sup>-1</sup>]</u>
$\lambda$	<u>Ripple wavelength [L]</u>
$u_c$	<u>Bedform migration celerity [L T<sup>-1</sup>]</u>
$l$	<u>Streambed height [L]</u>
$H_d$	<u>Ripple height [L]</u>
$B$	<u>Riverbed per unit width [L]</u>
$D_{50}$	<u>Median sediment size [L]</u>
$x$	<u>Horizontal coordinate, rightward positive [-]</u>
$y$	<u>Vertical coordinate, upward positive [-]</u>
$k$	<u>Sediment permeability [L<sup>2</sup>]</u>
$K$	<u>Sediment hydraulic conductivity [L T<sup>-1</sup>]</u>
$h$	<u>Hydraulic head [L]</u>
$h_m$	<u>Amplitude of the sinusoidal head variation [L]</u>
$m$	<u>Wavenumber of the variation [-]</u>
$g$	<u>Gravity acceleration [L T<sup>-2</sup>]</u>
$c_i$	<u>Concentration of reactive components [M L<sup>-3</sup>]</u>
$c_{gw}$	<u>Groundwater tracer [M L<sup>-3</sup>]</u>
$v$	<u>Seepage velocity [L T<sup>-1</sup>]</u>
$v_i$	<u>Seepage velocity in the <math>i</math>-direction [L T<sup>-1</sup>]</u>

$v_j$	<u>Seepage velocity in the <math>j</math>-direction [L T<sup>-1</sup>]</u>
$\theta$	<u>Sediment porosity [-]</u>
$\alpha_L$	<u>Longitudinal dispersivity [L]</u>
$\alpha_T$	<u>Transverse dispersivity [L]</u>
$D_{ij}$	<u>Hydrodynamic dispersion [L<sup>2</sup> T<sup>-1</sup>]</u>
$D_m$	<u>Molecular diffusion coefficient [L<sup>2</sup> T<sup>-1</sup>]</u>
$\tau$	<u>Tortuosity factor [-]</u>
$V_{AR}$	<u>Maximum reaction rate of aerobic respiration [T<sup>-1</sup>]</u>
$V_{DN}$	<u>Maximum reaction rate of denitrification [T<sup>-1</sup>]</u>
$K_{inh}$	<u>Non-competitive inhibition factor [M L<sup>-3</sup>]</u>
$K_{DOC}$	<u>Half-saturation for dissolved organic carbon [M L<sup>-3</sup>]</u>
$K_{NO_3^-}$	<u>Half-saturation for nitrate [M L<sup>-3</sup>]</u>
$K_{O_2}$	<u>Half-saturation for oxygen [M L<sup>-3</sup>]</u>
$X_{AR}$	<u>Microbial concentration facilitating aerobic respiration [M L<sup>-3</sup>]</u>
$X_{DN}$	<u>Microbial concentration facilitating denitrification [M L<sup>-3</sup>]</u>
$\rho_s$	<u>Sediment density [M L<sup>-3</sup>]</u>
$\rho$	<u>Water density [M L<sup>-3</sup>]</u>
$P$	<u>Hydraulic pressure [M L<sup>-1</sup>T<sup>-2</sup>]</u>
$D^*$	<u>Dimensionless particle parameter [-]</u>
$u^*$	<u>Bed shear velocity [L T<sup>-1</sup>]</u>
$u_{cr}^*$	<u>Critical bed shear velocity [L T<sup>-1</sup>]</u>
$n$	<u>Manning coefficient [-]</u>

$\tau^*$	<u>Shield parameter [-]</u>
$\tau_{cr}$	<u>Critical shear stress [M L<sup>-1</sup>T<sup>-2</sup>]</u>
$\tau_{cr}^*$	<u>Critical Shield parameter [-]</u>
$r$	<u>Submerged specific gravity of sediment [-]</u>
$R_{O_2}$	<u>Aerobic respiration rate [M L<sup>-3</sup>T<sup>-1</sup>]</u>
$R_{s-NO_3}$	<u>Non-mixing-dependent denitrification rate [M L<sup>-3</sup>T<sup>-1</sup>]</u>
$R_{g-NO_3}$	<u>Mixing-dependent denitrification rate [M L<sup>-3</sup>T<sup>-1</sup>]</u>
$R_{DOC}$	<u>Dissolved oxygen carbon consumption rate [M L<sup>-3</sup>T<sup>-1</sup>]</u>
$\nu$	<u>Kinematic viscosity of water [L<sup>2</sup> T<sup>-1</sup>]</u>
$\mu$	<u>Dynamic viscosity of water [M L<sup>-1</sup> T<sup>-1</sup>]</u>
$u_s$	<u>Underflow seepage velocity induced by stream gradient [L T<sup>-1</sup>]</u>
$Re$	<u>Reynolds number [-]</u>
$U_r$	<u>ratio of bedform celerity to pore water velocity [-]</u>
$u_p$	<u>Darcy velocity induced by pumping process [L T<sup>-1</sup>]</u>
$u_q$	<u>Vertical groundwater flux [L T<sup>-1</sup>]</u>
$U_b$	<u>Ratio of vertical groundwater flux to hyporheic exchange flux</u>
$\tau_R$	<u>Biogeochemical reaction timescale [T]</u>
$\tau_T$	<u>Water transport timescale [T]</u>
$Da$	<u>Damköhler number [-]</u>
$\Delta x$	<u>Bedform migrating displacement per timestep (L)</u>
$dt$	<u>The length of per timestep (T)</u>
$Da'$	<u>Conversion factor for unit Darcy to m<sup>2</sup> [-]</u>

<u><math>F_{mix}</math></u>	<u>Proportion of mixing flux to hyporheic exchange flux [-]</u>
<u><math>A_{mix}</math></u>	<u>Proportion of mixing zone to the whole domain [-]</u>
<u><math>A</math></u>	<u>Streambed area [L<sup>2</sup>]</u>
<u><math>M_{NDN}</math></u>	<u>Nitrate removed by non-mixing-dependent denitrification [M T<sup>-1</sup>]</u>
<u><math>M_{DN}</math></u>	<u>Nitrate removed by mixing-dependent denitrification [M T<sup>-1</sup>]</u>
<u><math>N_{RE}</math></u>	<u>Nitrate removal efficiency [-]</u>
<u><math>M_{in}</math></u>	<u>Nitrate being introduced into streambed [M T<sup>-1</sup>]</u>
<u><math>F_{in}</math></u>	<u>Oxygen influx [M L<sup>-2</sup> T<sup>-1</sup>]</u>
<u><math>F_{out}</math></u>	<u>Oxygen outflux [M L<sup>-2</sup> T<sup>-1</sup>]</u>
<u><math>A_o</math></u>	<u>Oxygenated area [L<sup>2</sup>]</u>

#### Abbreviations

<u>HZ</u>	<u>Hyporheic zone</u>
<u>SW</u>	<u>Surface water</u>
<u>GW</u>	<u>Groundwater</u>
<u>HEF</u>	<u>Hyporheic exchange flow</u>
<u>MF</u>	<u>Mixing flux</u>
<u>NMD</u>	<u>Non-mixing-dependent</u>
<u>MD</u>	<u>Mixing-dependent</u>
<u>AR</u>	<u>Aerobic respiration</u>
<u>DN</u>	<u>Denitrification</u>
<u>RMSE</u>	<u>Root means square error</u>

DNRA      Dissimilatory nitrate reduction to ammonium

ANAMMOX      Anaerobic ammonium oxidation

18 **Abstract** The hyporheic zone (HZ), where surface water (SW) and groundwater (GW) interact  
19 and mix, acts as a critical interface that attenuates contaminants through enhanced  
20 biogeochemical cycling. While bedform migration significantly influences hyporheic exchange  
21 and non-mixing-driven reactions of solutes from upstream SW, the effects of bedform migration  
22 on SW-GW mixing dynamics and mixing-triggered biogeochemical reactions—particularly  
23 under gaining stream conditions—remain poorly understood. Establishing~~Pioneering~~ a coupled  
24 hydrodynamic and reactive transport model that incorporates bedform migration, this paper  
25 systematically examines nitrogen processing for scenarios of variable sediment grain size,  
26 stream velocities, and upwelling GW fluxes. Results of this study reveal that SW-GW mixing  
27 and mixing-triggered denitrification zones progressively transition from crescent shapes into  
28 uniform band-like configurations as bedforms migrate. Both hyporheic exchange flux and  
29 mixing flux increase with increasing stream velocity and associated bedform celerity. The  
30 mixing proportion and mixing zone size increase at the start of migration, while they  
31 ~~reach~~remain approximately stable ~~constant~~ when turnover becomes the dominant hyporheic  
32 ~~water~~ exchange mechanism ~~for fine-medium sandy riverbed~~. Slow to moderate migrated  
33 bedforms with enhanced mixing dynamics facilitate mixing-triggered denitrification, whereas  
34 ~~f~~Fast stream flows and migrating bedforms shorten ~~reduce~~ solute residence timescales and limits  
35 denitrification ~~potential~~opportunities. Consequently, in fine to medium sandy sediments,  
36 groundwater-borne nitrate removal efficiency declines significantly with bedform migration.  
37 ~~Consequently, nitrate removal efficiency from both stream and groundwater-borne sources~~  
38 ~~decreases significantly with bedform migration in fine-medium sandy sediments.~~ The self-  
39 purification capacity of the HZ, and particularly its functioning as a natural barrier against GW

40 contamination, is hindered under such dynamic bedform conditions. These findings highlight the  
41 need to maintain stable bedform conditions in restoration projects to enhance the capacity of HZ  
42 contaminant attenuation.

## 43 **1. Introduction**

44 Anthropogenic activities such as the intensification of agricultural practices with its  
45 increased use of mineral and organic fertilizers, together with high livestock densities and  
46 emissions of inadequately treated domestic and industrial wastewater have significantly  
47 increased nitrogen loading to rivers and groundwater, which impacts water quality, causing  
48 eutrophication, hypoxic and related deterioration of ecosystem functions (Conley et al., 2009;  
49 Rouse et al., 1999). Long-term regulatory monitoring data (e.g., from the UK) indicate that  
50 nitrate levels have stabilized in many rivers, while nitrate concentrations in groundwater-fed  
51 rivers continue to increase (Burt et al., 2011; Howden and Burt, 2008). The persistence of nitrate  
52 contamination in groundwater and associated risks of a “nitrate time bomb” (Ascott et al., 2019)  
53 has highlighted the urgency of exploring the potential of natural microbial processes to mediate  
54 nitrate transformation and removal in riverbed sediments (Shelley et al., 2017; Lansdown et al.,  
55 2015; Rivett et al., 2008).

56 The hyporheic zone (HZ) has received significant attention for its potential to facilitate  
57 enhanced nitrate transformation and removal via denitrification that is a primary process  
58 permanently reducing nitrate, with hyporheic exchange flows (HEFs) acting as a critical  
59 mechanism for transporting nitrate-rich surface water to the riverbed sediments where microbial  
60 activities and biogeochemical reaction rates are enhanced (Boano et al., 2014; Boulton et al.,

61 1998; Cardenas, 2015; Xian et al., 2022; Krause et al., 2022). It has for long been assumed that  
62 predominantly stream waters provide inputs of bioavailable (mainly dissolved) organic carbon  
63 (DOC), oxygen (O<sub>2</sub>) and nitrate (~~NO<sub>3</sub><sup>-</sup>~~NO<sub>3</sub>) into the riverbed where the residence and re~~a~~ction  
64 times determine the occurrence of aerobic respiration and the potential for shifts into anaerobic  
65 conditions that may facilitate denitrification along the HEF paths in the presence of enough  
66 remaining DOC (Zarnetske et al., 2011a, b). These hydrological and biogeochemical  
67 mechanisms are in this form mainly representative of headwater streams, where the HEF is  
68 induced by stream flow turbulence (Boano et al., 2011; Roche et al., 2018, 2019), local  
69 geomorphological setting (Cardenas et al., 2008; Marzadri et al., 2012; Tonina and Buffington,  
70 2007), and flow obstacles such as woody debris, streambed engineering or restoration structures  
71 (Briggs et al., 2013; Wondzell et al., 2009), and hyporheic nitrate removal processes expected to  
72 mainly occur at the middle to end- hyporheic flow paths within HEF cell sediments.

73 In lowland systems, groundwater-fed streams and rivers in permeable catchments will be  
74 dominated by base flow of often nitrate enriched groundwaters. The subsurface hydrological  
75 conditions are driven by horizontal HEF as well as vertical fluxes resulting from regional  
76 groundwater flow toward (or from) the stream. Spatial variability in regional groundwater flow  
77 can significantly affect hyporheic exchange and biogeochemical cycling (Boano et al., 2013;  
78 Krause et al., 2013; Munz et al., 2011; Naranjo et al., 2015). It is important to note that the  
79 groundwaters of many piedmont plains and lowlands are often contaminated with high nitrate  
80 concentrations, but usually low in bio-available DOC (Krause et al., 2022). When nitrate-rich  
81 groundwater upwells through deeper sediments and reaches a region enriched in availability of  
82 DOC, nitrate reduction processes are significantly facilitated in the condition (Krause et al.,

83 2009; Naranjo et al., 2015; Ping et al., 2023; Trauth et al., 2017). Stelzer and Bartsch (2012)  
84 developed such a conceptual framework of nitrate-rich gaining fluvial setting from 8 sites in the  
85 Waupaca River Watershed with three order magnitude in groundwater nitrate concentration.  
86 Lansdown et al. (2014) also measured high denitrification rate at deep sediment in the coarse-  
87 grained sediments typical of groundwater-fed system, located within the River Leith (Cumbria,  
88 UK) where diverse nitrogen concentration changes were confirmed earlier (Krause et al., 2009).

89 Turnover and removal of the large amounts of  $\text{NO}_3^-$  from groundwater require DOC either  
90 from autochthonous streambed sources or from downwelling surface water to stimulate nitrate  
91 reduction (Krause et al., 2013, 2022; Ping et al., 2023; Sawyer, 2015; Trauth et al., 2017). For  
92 coarse grain or sandy riverbeds with low autochthonous organic matter content, stream-borne  
93 DOC dominates the supply of carbon sources for nitrogen transformation processes. Sandy  
94 sediments with less autochthonous organic carbon sources covering the majority of alluvial  
95 riverbeds are commonly characterized by topographical features such as ripples, dunes, and  
96 riffle-pool sequenced. The typical and multiple HEF cells induced by bedform topography are  
97 generally in crescent shapes (Fox et al., 2014; Wu et al., 2024). The downward advection of  
98 stream borne DOC provides electron donor and mixes with nitrate-rich and anoxic groundwater.  
99 It has been shown that the highest potential for mixing triggered denitrification is often found at  
100 the margin of the HEF cells, which represents the last natural protection before nitrate enter a  
101 stream (Hester et al., 2013, 2014; Gu et al., 2008; Nogueira et al., 2022).

102 The effects of mixing triggered denitrification on groundwater borne nitrate transformation  
103 in HZs have been studied almost exclusively for the case of stationary, that is immobile  
104 bedforms (Hester et al., 2017, 2019; Trauth and Fleckenstein et al., 2017; Ping et al., 2023).

105 Bedforms are mobile in dynamic equilibrium or undergo constant changes during periods of  
106 moderate to high stream flow, and they are typically found in medium and larger waterways  
107 under realistic field conditions (Bartholdy et al., 2015; Risse-Buhl et al., 2023; Schindler et al.,  
108 2015). For example, Harvey et al. (2012) observed the migrating bedforms of dunes (with a  
109 median grain size  $D_{50}$  of  $\sim 380 \mu\text{m}$  for the riverbed sediments) at a velocity of 57.6 cm/h  
110 during base flow in the “Clear Run” stream in eastern North Carolina, USA. Ahmerkamp et al.  
111 (2017) found that the ripple bedforms for sands ( $D_{50} = 63 \mu\text{m}$ ) ranged from 11 to 29 cm with a  
112 constant ratio of bedform height and length at 1/9, and migrated at velocities of 0.7–6.5 cm/h in  
113 the German Bight, Southeastern part of the North Sea. Bedforms migration complicates the  
114 development of hyporheic flow fields, ~~facilitate~~increases solute exchange, alters redox  
115 conditions in riverbeds, and affects contaminant transport and transformation (Ahmerkamp et  
116 al., 2015; Schulz et al., 2023; Peleg et al., 2024). Previous studies focused on ripple-driven  
117 hyporheic exchange in headwater to midstream reaches under neutral conditions, and simulated  
118 bedform migration via a moving frame of reference (Ahmerkamp et al., 2015; Jiang et al., 2022;  
119 Kessler et al., 2015; Ping et al., 2022; Zheng et al., 2019). Ping et al. (2022) used a fixed  
120 reference frame and noted that bedform migration negatively impacts non-mixing-dependent  
121 denitrification rates and stream-borne nitrate removal efficiency~~Specifically, bedform migration~~  
122 ~~has negative influences on non-mixing dependent denitrification (where nitrate and DOC are~~  
123 ~~both derived from surface water and travel together along the flow paths) rate and nitrate~~  
124 ~~removal efficiency (Jiang et al., 2022; Kessler et al., 2015; Ping et al., 2022; Zheng et al., 2019).~~  
125 However, no studies have yet investigated the effects of bedform migration on surface water-  
126 groundwater mixing in the lowland reaches of streams under gaining conditions, nor have they

127 explored its controls or implications for mixing-induced denitrification in groundwater-fed  
128 streams and rivers. However, no studies have yet investigated and explored the effects of  
129 bedforms migration on mixing of surface water and groundwater, as well as its controls and  
130 implications for mixing triggered denitrification reaction in groundwater fed streams and rivers.

131 In this study, numerical modeling of hyporheic flow and multi-component solute (DOC,  
132 O<sub>2</sub>, and NO<sub>3</sub><sup>-</sup>) transport is used to evaluate study the effect of bedform migration on mixing-  
133 dependent denitrification in the HZ of a gaining river, where the overlying stream water is  
134 induced into the sediment by ripple-type bedforms. The objectives of this study are to determine  
135 the effects of bedform migration on the overall extent and magnitude of mixing of surface water  
136 and upwelling groundwater, as well as its influences on groundwater-borne nitrate transport and  
137 transformation.

## 138 **2. Methods**

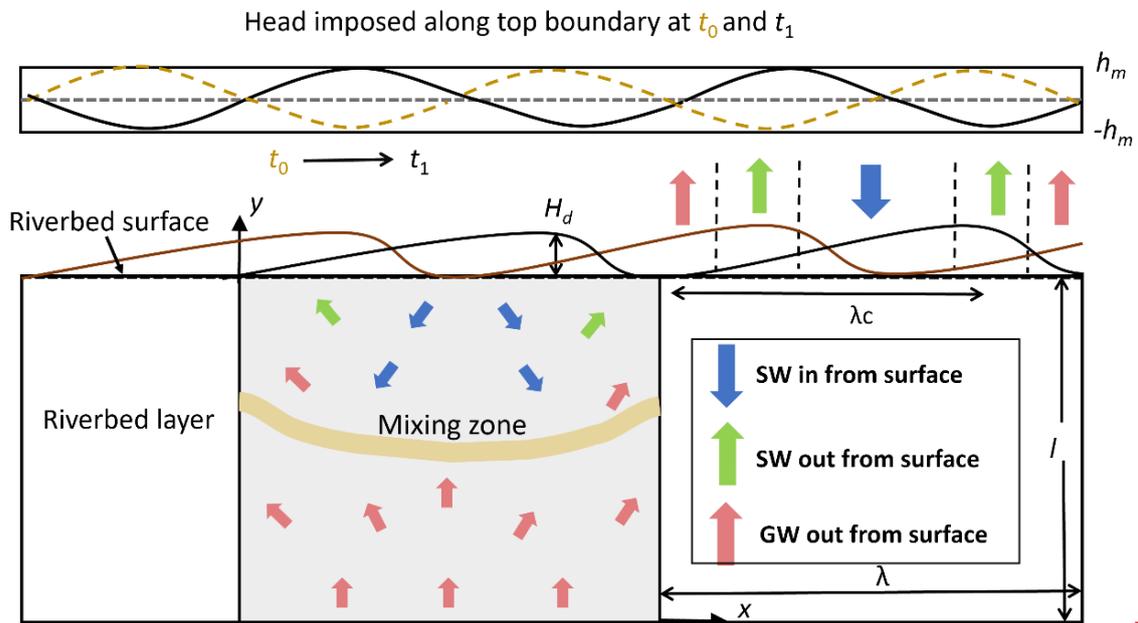
### 139 **2.1 Model description**

140 The stream flow over the bedform generates head gradients along the riverbed surface,  
141 drives hyporheic exchange, and induces the formation of periodic ripples. Hyporheic flow,  
142 solute transport, and biogeochemical reactions were modelled in saturated sediments beneath a  
143 riverbed. Ripples develop form in the riverbed and migrate downstream above the riverbed  
144 surface as a consequence of due to sediment bedload transport processes, with sediment grain  
145 eroded and deposited at the surface-, whereas deeper streambed sediments remain relatively  
146 stable (Harvey et al., 2012; Precht et al., 2004). From the perspective of an observer who stands  
147 at the target single riverbed and watches the ripples passing by, the bedform shapes and

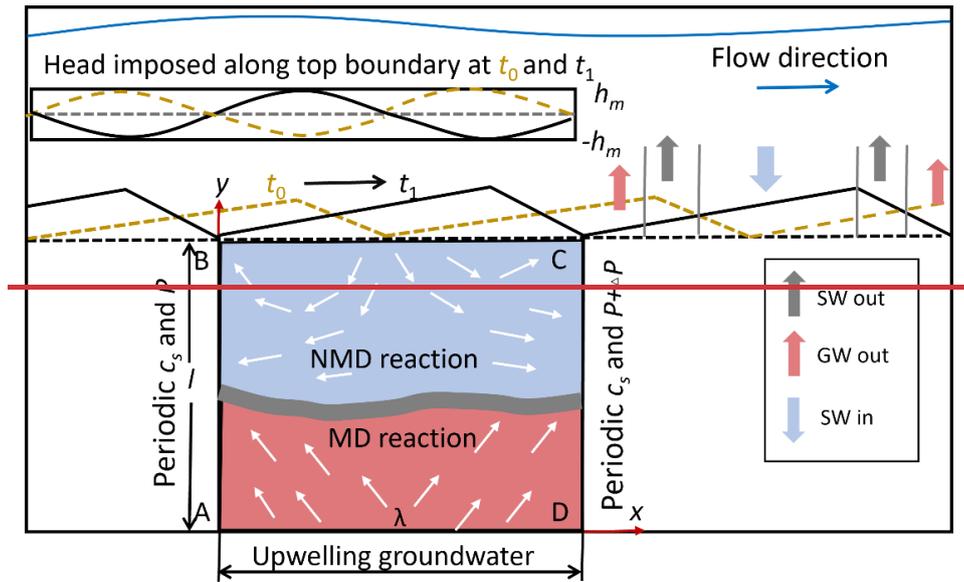
148 associated pressure profiles migrate downstream over the immobile riverbed layer. Because  
149 ripple heights ( $H_d = 2$  cm) accounts for 20% of the stream water depth ( $H = 10$  cm) and are far  
150 smaller than that of the underlying immobile domain ( $l = 16$  cm), the undulating riverbed can be  
151 reasonably approximated as a flat bed with time-varying pressure patterns during bedform  
152 migration. Besides, reactive solute transport also primarily takes places within the immobile  
153 riverbed sediment. Therefore, a two-dimensional (2D) rectangular domain is constructed with a  
154 length ( $\lambda$ ) of 0.2 m and depth ( $l$ ) of 0.16 m. A fixed reference frame is adopted, with its origin  
155 located at the bottom-left corner of the model. Here, the horizontal direction corresponds to the  
156  $x$ -axis and the vertical direction to the  $y$ -axis. The model domain has four corners: A (0, 0), B (0,  
157  $l$ ), C ( $\lambda$ ,  $l$ ), and D ( $\lambda$ , 0). The lateral boundaries are AB (left,  $x = 0$ ) and CD (right,  $x = \lambda$ ); the top  
158 boundary is BC ( $y = l$ ), and the bottom boundary is AD ( $y = 0$ ).

159 The model schematic is sketched in Figure 1. The model is relevant to the model used by  
160 Ping et al. (2022) but with significant differences. Specifically, Ping et al. (2022) focused on  
161 ripple-driven hyporheic exchange processes in the headwater to midstream sections of streams  
162 under neutral conditions. In contrast, the present study focuses on the downstream sections of  
163 rivers under gaining conditions, aiming to simulate surface water-groundwater interacting and  
164 mixing processes under dynamic bedforms. To our knowledge, previous migrating ripple models  
165 have extensively examined surface water-groundwater interaction processes (i.e., ripple-driven  
166 hyporheic exchange processes), yet the mixing process remains uninvestigated (Ahmerkamp et  
167 al., 2017; Jiang et al., 2022; Zheng et al., 2019; Kessler et al., 2015). In downstream sections,  
168 groundwater is typically nitrate-contaminated, and denitrification is often co-regulated by both  
169 mixing regimes. The model is also developed with advancements of these previous models by

170 including an important biogeochemical reaction in nitrate dynamics, that is mixing-triggered  
 171 denitrification (see next section for details).



172 The  
 173 stream geometry is parameterized through its slope  $S$ , average water depth  $H$ , and mean velocity  
 174  $U$ . Triangular shaped ripple bedforms of wavelength  $\lambda$  are considered to develop and migrate  
 175 downstream by a unidirectional average velocity  $u_e$ . Flow is driven by pumping (pressure  
 176 variation along the bedform surface) and bedform migration processes and influenced by  
 177 upwelling groundwater.



**Figure 1.** Schematic of the model domain with bed form geometry and boundary conditions.

Stream flow and bedform migration are from left to right. The dashed yellow lines represent the streambed surface and head profile at time  $t_0$ , and the solid black lines represent the streambed surface and head profile after migration at  $t_1$ . “SW in” is where surface water enters the riverbed layer, and “SW out” is where surface water discharges to the stream, and “GW out” is along the upstream and downstream sides of bedforms where groundwater discharges to the stream. The yellow-gray band represents the mixing zone of surface water and groundwater. NMD reaction = non-mixing-dependent reaction and MD reaction = mixing-dependent reaction. This figure is modified from Figure 2 in Ping et al. (2022), with permission granted by Wiley.

In this study, hyporheic flow, multi-component solutes (DOC,  $O_2$ , and  $NO_3^-$ ) transport, and biogeochemical reactions (both non-mixing-dependent NMD and mixing-dependent MD reactions) are modelled in saturated sediments beneath a riverbed layer. Stream flow over the riverbed is not simulated. The hyporheic exchange flow is driven by pumping (overlying flow over the bed form produces a hydraulic head  $h|_{v=1}$ ) and bedform migration (triangular shaped

ripples migrate downstream by an average velocity  $u_c$ ) and influenced by upwelling groundwater. Stream water transports DOC,  $O_2$ , and  $NO_3^-$  into the riverbed, whereas groundwater polluted with  $NO_3^-$  upwells toward the stream. In order to distinguish NMD and MD denitrification, we define stream-borne nitrate ( $s-NO_3^-$ ) and groundwater-borne nitrate ( $g-NO_3^-$ ) as two distinct reactants.

## 2.2 Model formulation

### 2.2.1 Governing equation

The pore water flow was calculated using Darcy's law and the groundwater flow equation:

$$\nabla \cdot (-K \nabla h) = 0 \quad (1)$$

where  $h$  [L] is the hydraulic head, and  $K$  [ $L T^{-1}$ ] =  $k\rho g/\mu$  is the hydraulic conductivity,  $k$  [ $L^2$ ] is the permeability of the riverbed,  $\rho$  [ $M L^{-3}$ ] are the density of water,  $g$  [ $L T^{-2}$ ] is the gravity acceleration, and  $\mu$  [ $M L^{-1} T^{-1}$ ] is the dynamic viscosity of water.

The head profile on the streambed surface (BC) was described as a sinusoidal function that moves downstream by the ripple's migration velocity  $u_c$  (Ping et al., 2022):

$$h|_{y=l} = h_m \cdot \sin m(x - u_c dt) \quad (2)$$

where  $h_m$  [L] is the amplitude of the head variation,  $m$  [-] is the wave number of the variation ( $m = 2\pi/\lambda$ ), the head difference is related to the properties of the overlying flow (Elliott and Brooks, 1997):

$$h_m = a \frac{U^2}{2g} \left( \frac{H_d / H}{0.34} \right)^m \quad (3)$$

where  $\alpha = 0.28 [-]$  is a dimensionless coefficient,  $U [L T^{-1}]$  is the average stream velocity,  $H_d [L]$  is the height of the ripple,  $H [L]$  is the water depth, and  $g [L T^{-2}]$  is the gravity acceleration. The exponent  $n$  equals to  $3/8$  if  $H_d < 0.34H$  and  $3/2$  otherwise.

The transport of reactive solutes within the streambed sediment was described by the advection-dispersion-reaction equation:

$$\frac{\partial c_i}{\partial t} - \nabla \cdot (D_{ij} \nabla c_i) + \nabla \cdot \mathbf{v} c_i = R_i \quad (24)$$

where  $c_i [M L^{-3}]$  represents the concentration of reactive components,  $\mathbf{v} [L T^{-1}]$  is the seepage or linear pore water velocity vector,  $D_{ij} [L^2 T^{-1}]$  is the hydrodynamic dispersion tensor, and  $R_i [M L^{-3} T^{-1}]$  denotes the biogeochemical rate of reactive components; the elements of the dispersion tensor  $D_{ij} [L^2 T^{-1}]$  are defined by Bear and Verruijt (1998):

$$D_{ij} = (\alpha_L - \alpha_T) \cdot \frac{v_i v_j}{|\mathbf{v}|} + \delta_{ij} \cdot (\alpha_T |\mathbf{v}| + \theta \cdot \iota D_m) \quad (53)$$

where  $\alpha_L [L]$  and  $\alpha_T [L]$  are longitudinal and transverse dispersivities, respectively,  $D_m [L^2 T^{-1}]$  is molecular diffusion coefficient,  $v_i$  and  $v_j$  denote the seepage velocities in the  $i$ -direction and  $j$ -direction, respectively; and  $\iota [-]$  is tortuosity.

The reactive transport model considered three chemical species: DOC,  $O_2$ , and  $NO_3^-$ . In order to distinguish non-mixing dependent and mixing dependent denitrification, we divided nitrate into two separate pools, denoted  $s-NO_3^-$  for nitrate transported from the surface water and  $g-NO_3^-$  for nitrate from upwelling groundwater. The biogeochemical reactions: aerobic respiration (AR), non-mixing-dependent, and mixing-dependent denitrification (DN) are

232 considered in the reactive transport model:



235 Equations R1 and R2, with DOC as the electron donor and O<sub>2</sub> and NO<sub>3</sub><sup>-</sup> as sequential  
236 electron acceptors, capture the primary mechanism of NO<sub>3</sub><sup>-</sup> cycling and are widely used in  
237 studies on hyporheic zone nitrogen dynamics (Bardini et al., 2012; Hester et al., 2019; Zheng et  
238 al., 2019). Denitrification is the well-recognized critical process for NO<sub>3</sub><sup>-</sup> transformation and  
239 reduction in riparian and hyporheic zones. The reaction stoichiometry and electron acceptor  
240 utilization order in R1 and R2 match well-established principles from lab incubations and field  
241 investigations (Hedin et al., 1998; Liu et al., 2017; Zarnetske et al., 2011a, 2011b). Dissimilatory  
242 nitrate reduction to ammonium (DNRA) and anaerobic ammonium oxidation (ANAMMOX) are  
243 not included in the model, as these processes play secondary role on nitrogen cycling and  
244 require highly specific conditions to occur (Zarnetske et al., 2012). Ammonia (NH<sub>4</sub><sup>+</sup>) was  
245 excluded, as it is unstable in the study environment: NH<sub>4</sub><sup>+</sup> in surface water or groundwater is  
246 prone to nitrification (converting NH<sub>4</sub><sup>+</sup> to NO<sub>3</sub><sup>-</sup>) either within hyporheic flow cells or in  
247 upwelling groundwater upon O<sub>2</sub> exposure, leading to relatively low NH<sub>4</sub><sup>+</sup> concentrations (Hester  
248 et al., 2014).

249 Reaction rates were defined using Monod kinetics (Monod, 1949; Zarnetske et al., 2012):

$$250 \quad R_{AR} = V_{AR} \times X_{AR} \times \frac{c_{O_2}}{c_{O_2} + K_{O_2}} \times \frac{c_{DOC}}{c_{DOC} + K_{DOC}} \quad (64)$$

$$251 \quad R_{DN} = V_{DN} \times X_{DN} \times \frac{c_{NO_3^-}}{c_{NO_3^-} + K_{NO_3^-}} \times \frac{c_{DOC}}{c_{DOC} + K_{DOC}} \times \frac{K_{inh}}{K_{inh} + c_{O_2}} \quad (57)$$

252 The reaction terms  $R_i$  was given by

$$253 \quad R_{s-NO_3^-} = -R_{DN}(c_{s-NO_3^-}) \quad (86)$$

$$254 \quad R_{g-NO_3^-} = -R_{DN}(c_{g-NO_3^-}) \quad (97)$$

$$255 \quad R_{O_2} = -R_{AR} \quad (108)$$

$$256 \quad R_{DOC} = -R_{AR} - 0.8 \times (R_{DN}(c_{s-NO_3^-}) + R_{DN}(c_{g-NO_3^-})) \quad (119)$$

257 where  $V_{AR}$  and  $V_{DN}$  [ $T^{-1}$ ] are the maximum reaction rate of AR aerobic respiration and DN-  
258 denitrification,  $X_{AR}$  and  $X_{DN}$  [ $M L^{-3}$ ] are the biomass of functional microbial groups facilitating  
259 the reaction components of AR and DN.  $K_{inh}$  [ $M L^{-3}$ ] is a non-competitive inhibition factor used  
260 for representing inhibition of DN given oxygen availability.

### 261 2.2.2 Boundary condition

262 The hydraulic head imposed along the top boundary was described as a sinusoidal function  
263 that moves downstream by the ripple migration velocity ( $u_c$  [ $L T^{-1}$ ]; Boano et al., 2013):

$$264 \quad h|_{y=l} = h_m \cdot \sin m(x - u_c dt) \quad (10)$$

265 where  $m$  [-] is the wave number of the variation ( $m = 2\pi/\lambda$ ),  $dt$  [T] denotes a timestep in the  
266 simulation, and  $h_m$  [L] is the amplitude of the head variation (Elliott and Brooks, 1997):

$$267 \quad h_m = a \frac{U^2}{2g} \left( \frac{H_d / H}{0.34} \right)^e \quad (11)$$

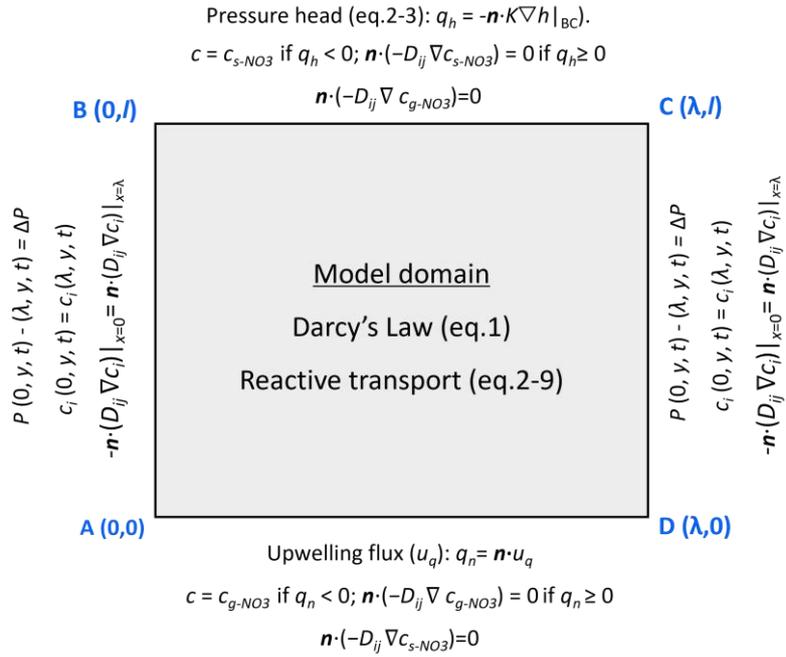
268 where  $a = 0.28$  [-] is a dimensionless coefficient,  $U$  [ $L T^{-1}$ ] is the average stream velocity,  $H_d$  [L]  
269 is the height of the ripple,  $H$  [L] is the water depth. The exponent  $e$  equals to  $3/8$  if  $H_d < 0.34H$   
270 and  $3/2$  otherwise.

271 Given the periodic characteristics typically observed in riverbeds under field conditions  
 272 (Ahmerkamp et al., 2017; Harvey et al., 2012), a single ripple was chosen in this study. Periodic  
 273 pressure and solute boundary conditions were applied to side boundaries AB and CD to replicate  
 274 the repetitive geometric constraints of the streambed. This is a well-established practice in  
 275 hyporheic exchange modeling, as it minimizes edge effects without the need for computationally  
 276 intensive full-scale simulations of multiple consecutive bedforms. The lateral boundaries AB and  
 277 DC were set to be periodic boundaries:—

$$278 \quad \underline{P(0, y, t) = P(\lambda, y, t) + \Delta P} \quad (12)$$

$$279 \quad \underline{c_i(0, y, t) = c_i(\lambda, y, t) \quad \mathbf{n} \cdot (-D_{ij} \nabla c_i) \Big|_{x=0} = \mathbf{n} \cdot (D_{ij} \nabla c_i) \Big|_{x=\lambda}} \quad (13)$$

280 where  $P$  [ $M L^{-1} T^{-2}$ ] is the pressure calculated using the hydrostatic head from equation (10),  $P =$   
 281  $\rho g h$ .  $P(0, y, t) = P(\lambda, y, t) + \Delta P$ ,  $c_i(0, y, t) = c_i(\lambda, y, t)$  and  $\partial c_i(0, y, t) / \partial y = \partial c_i(\lambda, y, t) / \partial y$ . The  
 282 additional pressure drop  $\Delta P$  [ $M L^{-1} T^{-2}$ ] ~~was~~ derived from the streambed gradient and  
 283 calculated using  $\Delta P = S \rho g \lambda$ . For the validation of model generalization, simulation of three  
 284 consecutive ripples was conducted. The middle ripple was focused on to compare its vertical  
 285 boundary-related pressure and solute concentration with outcomes from the single-ripple model.  
 286 Simultaneously, comparisons were made between the two models (single-ripple and three-  
 287 consecutive-ripple) regarding simulated riverbed flow fields, nitrate plumes, influxes, and  
 288 reaction rates. Although moderate left/right boundary pressure differences were detected  
 289 between their simulation results, such variations had negligible effects on reactive solute  
 290 transport. This comparison is provided in the Supporting Information (Text S2).



**Figure 2.** Schematic diagram of the simulated domain and boundary conditions.

At the bottom boundary, an upward groundwater flux was defined to mimic the process of groundwater discharge. For groundwater borne nitrate, the bottom boundary The top boundary BC was specified as an open Dirichlet boundary with constant  $g-NO_3^-$  solute concentrations in groundwater: the stream.

$$\begin{cases} c = c_i & \mathbf{n} \cdot \mathbf{v} < 0 \\ \mathbf{n} \cdot (-D_{ij} \nabla c_i) = 0 & \mathbf{n} \cdot \mathbf{v} \geq 0 \end{cases} \quad (14)$$

where the outward unit normal is  $n$ . Besides, the top boundary for the  $g-NO_3^-$  was assigned as outflow condition.

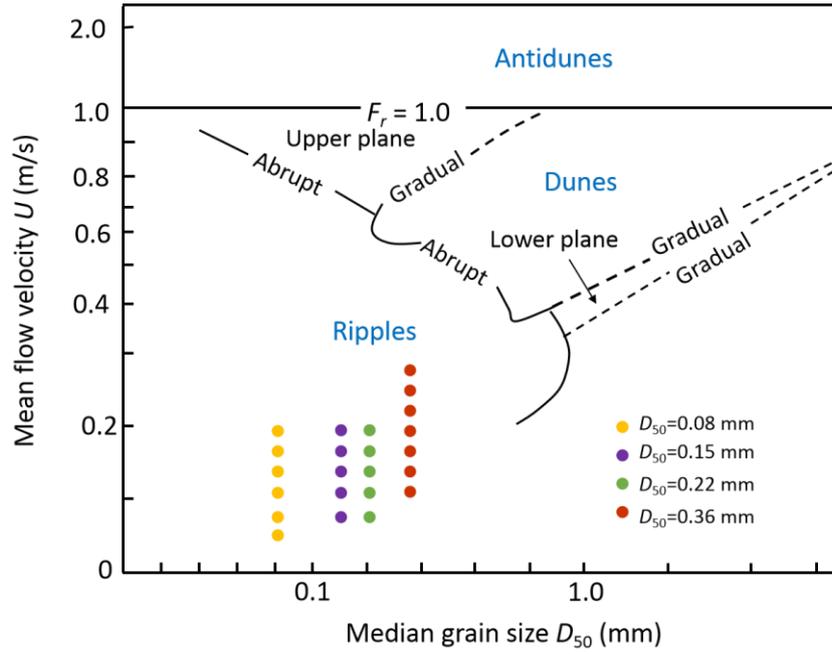
For stream-borne solutes includes DOC,  $O_2$ , and  $s-NO_3^-$ , an open boundary with a constant solute concentration was imposed on the top boundary, and outflow boundary was applied on the bottom of the domain. The concentrations of the all solutes were initially assumed to be zero. A quantitative mass-balance check for the model was included in Supporting Information

304 Text S2.

305 ~~An upward groundwater flux with constant nitrate concentration was specified at the~~  
306 ~~bottom boundary AD to mimic a nitrate polluted groundwater discharge.~~

### 307 **2.3 Bedform migration**

308 In this study, ripples formation was qualitatively determined using the bedform stability  
309 diagram (Figure 3). This diagram is a summarized reference for examining equilibrium bed  
310 configurations in unidirectional flow, derived from a series of flume experiments and field  
311 studies (Ashley, 1990). Besides, a series of quantitative criteria (Text S1 in the supporting  
312 information: Criteria for ripple migration) was examined to ensure ripple formation under the  
313 modeled scenarios and the achievement of dynamic equilibrium~~The mechanisms of bedform~~  
314 ~~initialization, formation and migration are initiated through a set of criteria to ensure that ripples~~  
315 ~~are expected to form under the modeled scenarios and reach a state of dynamic equilibrium,~~  
316 ~~where the ripples remain mobile while maintaining their shape. The development of ripples is~~  
317 ~~assessed under different conditions of median particle size  $D_{50}$  and flow velocity based on a set~~  
318 ~~of quantitative criteria (as reference in Ping et al., 2022 for criteria on ripple formation). For a~~  
319 ~~specific grain size of streambed sediment ( $D_{50}$ ), particular stream velocities ( $U$ ) satisfying that~~  
320 ~~fulfill all these criteria for the development of ripple bedforms that are mobile under dynamic~~  
321 ~~equilibrium are selected and displayed in (Figure 32). All simulation scenarios were listed in~~  
322 ~~the Table S13, in the supplementary material.~~



323  
 324 **Figure 32.** The bedform stability diagram (modified from Ashley, 1990) showing the bedform  
 325 properties and hydraulic conditions considered in this study.

326 Ripple migration velocities were implemented using an empirical relation after Coleman  
 327 and Melville (1994), which was derived from flume experiments:

$$328 \quad \frac{u_c}{(u^* - u_{cr}^*)(\tau^* - \tau_{cr}^*)} (H_d / D_{50} - 3.5)^{1.3} = 40 \quad (15)$$

329 where  $D_{50}$  [L] is the median grain size,  $u^* = (gHS)^{0.5}$  [L T<sup>-1</sup>] is the bed shear velocity,  $S$  [-] is the  
 330 stream gradient and calculated by Chezy equation ( $U = H^{2/3} S^{1/2} / n$ , where  $n$  [-] is the Manning  
 331 coefficient and assumed to be 0.02 for sand).  $u_{cr}^*$  [L T<sup>-1</sup>] is the critical bed shear velocity and it  
 332 can be calculated by the critical Shield parameter  $\tau_{cr}^*$  ( $\tau_{cr}^* = \tau_{cr} / g(\rho_s - \rho)D_{50}$ ,  $u_{cr}^* = (\tau_{cr} / \rho)^{0.5}$ ,  $\rho_s$   
 333 [M L<sup>-3</sup>] and  $\rho$  [M L<sup>-3</sup>] are the density of sediment and water), and  $\tau^*$  is the shield number related  
 334 to the bed shear velocity ( $\tau^* = u^{*2} / rgD_{50}$ ;  $r$  [-] is the submerged specific gravity of sediment =  $(\rho_s$   
 335  $- \rho) / \rho$ ). The critical shields parameter  $\tau_{cr}^*$  defines the threshold for the initialization of motion,  
 336 and it is determined by the dimensionless particle parameter  $D^*$  [-] (Soulsby, 1997; Zheng et al.,

337 2019):

$$338 \quad \tau_{cr}^* = \frac{0.3}{1+1.2D^*} + 0.055[1 - \exp(-0.02D^*)] \quad (16)$$

339 where  $D^* = D_{50} \cdot (rg/v^2)^{1/3}$ ,  $v$  is the kinematic viscosity of water [ $L^2 T^{-1}$ ]; The derived celerity was  
340 substituted into Equation (10) to determine the migration of sinusoidal head profile with ripples  
341 moving.

## 342 2.4 Governing non-dimensional numbers

343 The characteristics of the modeled system were depicted by a series of non-dimensional  
344 numbers, which represent the relative dominance of various forces, transport, and reaction  
345 processes in this system. Firstly, ~~we used~~ the Reynolds number  $Re$  was used to characterize the  
346 flow condition of surface water (Cardenas and Wilson, 2006):

$$347 \quad Re = \frac{UH_d}{\nu} \quad (17)$$

348 where  $\nu$  [ $L^2 T^{-1}$ ] represents kinematic viscosity of water.

349 ~~We introduced~~ the dimensionless parameter  $U_r$  [-] was introduced to represent the relative  
350 magnitude of bedform celerity and the pressure-induced pore water velocity driven by pressure  
351 variation over the ripple surface and upwelling groundwater:

$$352 \quad U_r = \frac{\theta \cdot u_c - u_s}{u_p} \quad (18)$$

353 where  $u_s$  [ $L T^{-1}$ ] is the seepage velocity of the underflow induced by stream gradient ( $u_s = KS$ ),  
354 and thus the characteristic horizontal velocity is  $u_c - u_s/\theta$ .  $u_p/\theta$  [ $L T^{-1}$ ] is the pore water velocity  
355 induced by pumping process and is calculated using the analytical solution after Boano et al.

(2009) and Fox et al. (2014) accounting for vertical groundwater flux ( $u_q$  [L T<sup>-1</sup>]):

$$u_p = u_{p,0} \sqrt{1 - \left( \frac{u_q}{\pi u_{p,0}} \right)^2} + \left( \frac{|u_q|}{\pi} \right) \sin^{-1} \left( \frac{|u_q|}{\pi u_{p,0}} \right) - \left( \frac{|u_q|}{2} \right) \quad (195)$$

$$u_{p,0} = a \frac{KU^2}{g\lambda} \left( \frac{H_d/H}{0.34} \right)^m \quad (2046)$$

where  $u_{p,0}$  [L T<sup>-1</sup>] represents the hyporheic exchange solely driven by pressure variation over the sediment-water interface; if  $U_r > 1$ , turnover process dominates and controls the hyporheic exchange, otherwise, the system is pumping process dominated (Jiang et al., 2022).

The relative magnitude of hyporheic exchange flux driven by pressure variation along the riverbed ripple surface and upwelling groundwater flux wasis determined as:

$$U_b = \frac{u_q}{u_{p,0}} \quad (2117)$$

The relative dominance of hyporheic exchange and biogeochemical reaction in nitrate removal can be defined by the Damköhler number (Ocampo et al., 2006; Zarnetske et al., 2012; Zheng et al., 2019):

$$Da = \frac{\tau_T}{\tau_R} \quad (2218)$$

the characteristic timescale for the transport of solutes through the ripple wasis estimated as (Azizian et al., 2015):

$$\tau_T = \frac{\lambda\theta}{\pi^2 u_p} \quad (2319)$$

and the reaction timescale ( $\tau_R$ ) represents the time needed to consume dissolved oxygen of hyporheic water to a prescribed anoxic environment threshold (2 mg/L). The reaction timescale

374 ~~was~~ described as:

$$375 \quad \tau_R = \frac{\ln(c_{O_2}/c_{O_2,\text{lim}})}{V_{AR}} \quad (240)$$

376 biogeochemical reactions are transport-limited when  $Da < 1$ . The biogeochemical reactions  
377 depended on reaction kinetics due to the brevity of the time that reactants spend within the HZ.  
378 Under these low  $Da$  conditions, the HZ remains oxic conditions, resulting in a minimal or no  
379 denitrification to occur. Conversely, when  $Da > 1$ , the residence time of reactants exceeds the  
380 reaction time, and thus oxygen is consumed and favors for the occurrence of denitrification in  
381 anoxic conditions (Jiang et al., 2022; Zarnetske et al., 2011a).

## 382 2.5 Model setup and parametrization

383 All parameter values in this study were shown in Table 1. The bedform geometry of Ping et  
384 al. (2022) was adopted for this study (~~streambed length and height:  $\lambda = 0.2$  m and  $l = 0.16$  m;~~ the  
385 ripples located at  $\lambda_c = 0.15$  m with a height of  $H_d = 0.02$  m). Here grain sizes  $D_{50}$  of 0.08, 0.15,  
386 0.22 and 0.36 mm were considered, typically falling within characteristic grain diameters on  
387 sandy riverbeds (Ahmerkamp et al., 2017; Harvey et al., 2012). The permeability of riverbed  
388 was calculated using ~~We used~~ the empirical relation  $k = Da'_\times 735 \times 10^6 \times D_{50}^2$  (where  $Da'_ =$   
389  $9.869 \times 10^{-13}$  is the conversion factor for unit Darcy to  $\text{m}^2$ ; Gangi, 1985).

390 The concentrations of DOC,  $\text{O}_2$ , and  $\text{s-NO}_3^-$  in stream were specified as 30 mg/L, 8 mg/L,  
391 and 5 mg/L. This configuration represents a pristine stream characterized by moderate nutrient  
392 levels (Ocampo et al., 2006). The  $\text{g-NO}_3^-$  in groundwater was set as 15 mg/L, representing the  
393 chemical signature of nitrate-contaminated groundwater that lacks both oxygen and organic

394 matter (Hester et al., 2014). The maximum reaction rate and corresponding functional microbial  
 395 concentration for AR and DN ~~were~~ listed in Table 1, the choose biogeochemical values are  
 396 consistent with the parameter setting of nutrient cycling in hyporheic zones and riparian zones  
 397 (Gu et al., 2008; Nogueira et al., 2021; Zarnetske et al., 2012).

398 **Table 1.** Model parameters used in numerical simulations

Parameter	Description	Value
$l$ [m]	Streambed depth	0.16 <sup>a, b</sup>
$l_c$ [m]	Ripple crest	0.15 <sup>a, b</sup>
$\lambda$ [m]	Wavelength of ripple	0.2 <sup>a, b</sup>
$H_d$ [m]	Height of ripple	0.02 <sup>a, b</sup>
$H$ [m]	Stream water depth	0.1 <sup>a, b</sup>
$\theta$ [1]	Porosity	0.38 <sup>e</sup>
$\alpha_L$ [m]	Longitudinal dispersivity	0.01 <sup>f</sup>
$\alpha_T$ [m]	Transverse dispersivity	0.001 <sup>f</sup>
$K_{inh}$ [mg L <sup>-1</sup> ]	Inhibition constant	0.25 <sup>c, d</sup>
$K_{DOC}$ [mg L <sup>-1</sup> ]	Half-saturation constant for dissolved organic carbon	6 <sup>c, d</sup>
$K_{NO3}$ [mg L <sup>-1</sup> ]	Half-saturation constant for nitrate	1 <sup>c, d</sup>
$K_{O2}$ [mg L <sup>-1</sup> ]	Half-saturation constant for dissolved oxygen	0.5
$V_{DN}$ [h <sup>-1</sup> ]	Maximum specific uptake rate for denitrification	1 <sup>c, d</sup>
$V_{AR}$ [h <sup>-1</sup> ]	Maximum specific uptake rate for aerobic respiration	2 <sup>c, d</sup>
$C_{DOC}$ [mg L <sup>-1</sup> ]	Concentration of dissolved organic carbon in stream	30
$C_{O2}$ [mg L <sup>-1</sup> ]	Concentration of dissolved oxygen in stream	8

$C_{s-NO_3^-}$ [mg L <sup>-1</sup> ]	Concentration of nitrate in stream	5
$C_{g-NO_3^-}$ [mg L <sup>-1</sup> ]	Concentration of nitrate in groundwater	15

<sup>a</sup> Janssen et al. (2012) <sup>b</sup> Ping et al. (2022) <sup>c</sup> Zarnetske et al. (2012) <sup>d</sup> Sawyer (2015) <sup>e</sup> [Ahmerkamp et al. \(2015\)](#) <sup>f</sup> [Bardini et al. \(2012\)](#)

The following distinct model experiments were carried out: The Reynolds number, i.e., mean stream velocity, was varied for  $Re = 2000-6000$  in intervals of 500, with corresponding stream water velocities of  $U = 0.1-0.3$  m/s. The range of  $U_b$  was set from 0.3 to 0.7 in intervals of 0.1; ~~1~~ correspondingly, the upwelling groundwater flux  $u_q$  was ranged between  $0.3 \times u_{p,0}$  and  $0.7 \times u_{p,0}$ . A larger upward flux than  $0.9 \times u_{p,0}$  would eliminate the entire hyporheic flow cell, thus ~~we set~~ the maximum boundary flux was set at a value slightly below this threshold. Meanwhile a minimum of  $0.2 \times u_{p,0}$  ensures that upwelling groundwater is still mixing with surface water with minor influences on hyporheic flow cell.

The finite element software, COMSOL Multiphysics (version 6.1) was used to solve the Darcy flow and multi-component solute transport model. Three computational grids were evaluated to ensure grid resolution independence of simulated results, confirming that the grid is sufficiently refined to capture mixing dynamics and minimize numerical dispersion. The fine, base and coarse grid sizes were 1.5 mm, 2 mm, and 2.5 mm, respectively. The grid independence analysis was provided in the Supporting Information (Text S2). For the simulations, the base grid size was selected, with the domain discretized into 19,940 cells. ~~The domain was discretized with a grid spacing from  $4 \times 10^{-4}$  to 0.2 cm, the resultant mesh consisting of 19,940 elements.~~ To maintain a constant bedform displacement ( $\Delta x$ ) per timestep, the simulation was conducted with  $\Delta x = 2$  mm, while  $dt$  was adjusted in inverse proportion to the

419 migration celerity  $u_c$ . The total duration of the simulation was set to be equal to the time needed  
420 for hundreds of ripples to travel across the modeled domain until the hyporheic exchange and  
421 biogeochemical processes reached quasi-steady states.

## 422 2.6 Model Metrics

### 423 2.6.1 Mixing of surface water and groundwater

424 Here, macroscopic mixing is defined as the colocation of surface water (SW) and  
425 groundwater (GW) within a specific aquifer volume (e.g., a numerical model element or cell), a  
426 process that causes solutes to be present simultaneously in an overlapping region (Nogueira et  
427 al., 2022). There is no established standard for the threshold governing SW and GW fractions  
428 during the mixing process; nonetheless, the general recognition is that mixing occurs when the  
429 GW proportion falls within the range of 10% to 90% (Hester et al., 2013; Woessner et al., 2000).  
430 This interval effectively distinguishes the occurrence of mixing, where SW and GW interact  
431 dynamically, from the two endmembers: pure GW (>90%) and pure SW (<10%). To evaluate  
432 the influence of threshold variations on mixing metrics, sensitivity analysis was conducted to  
433 examine three alternative GW fraction ranges: 10%–90% (wider interval), 16%–84%, and 20%–  
434 80% (narrower range). Thus, mixing flux and mixing zone were calculated based on these three  
435 thresholds.

436 ~~Mixing occurs at the interface between regions of advected surface water and upwelling~~  
437 ~~groundwater, where the flow paths from these two sources run parallel to each other. A We~~  
438 ~~specified a constant concentration of conservative tracer ( $c_{gw} = 1$  mg/L) in groundwater~~GW was  
439 specified to represent the tracer signature of groundwater, following the methods outlined by

440 Hester et al. (2013, 2014). The tracer plume is used to visualize the surface water and  
441 groundwater mixing size and calculate the mixing magnitude quantitatively. The mixing zone is  
442 defined as the area where the groundwater proportion varies between 16% and 84% (Santizo et  
443 al., 2020). The size of the surface water and groundwater mixing zone ( $A_{mix}$ ) was calculated by  
444 integrating the area where the concentration of the conservative tracer ranges from 0.16 to 0.84  
445 mg/L.

446 The groundwater-borne tracer ~~amount of tracer mass~~ that undergoes mixing as it transitions  
447 from flow paths originating at the bottom boundary to those emerging at the streambed surface  
448 was quantified to determine the net effect of mixing ~~occurring along the entire length of the~~  
449 mixing zone. The streambed surface was divided into three zones (Figure 1): “SW IN”, where  
450 surface water enters the riverbed layerbed; “SW OUT”, where surface water discharges back  
451 into the overlying water column; and “GW OUT”, where upwelling groundwater discharges into  
452 the stream. The conservation tracer ~~mass~~ flux for the SW OUT zone was used to describe  
453 mixing. If no mixing occurred, all the conservative tracer ~~mass~~ entering the model at the bottom  
454 boundary would exit through the GW OUT zone on the riverbed~~streambed~~ surface. ~~The mixing~~  
455 flux (MF) across the sediment-water interface was determined by integrating the Darcy flux  
456 flowing outward through the SW-OUT zone, representing the total cumulative effect of mixing  
457 along the entire length of the mixing zone (Hester et al., 2013). The arrival of conservative solute  
458 at the riverbed surface is defined as the start of mixing, and the complete leave of all the  
459 conservation solute from the riverbed is defined as the end of mixing. The mixing flux across  
460 the riverbed surface was determined by integrating the outward volumetric flux containing  
461 groundwater-borne conservation solute within the specified concentration range (i.e.,  $c_{gw} = 0.1$

462 0.9 mg/L,  $c_{gw} = 0.16\text{--}0.84$  mg/L,  $c_{gw} = 0.2\text{--}0.8$  mg/L). This flux corresponds to groundwater  
463 flowing through “SW OUT” zone. Concurrently, the mixing zone was calculated by integrating  
464 the riverbed area that contains groundwater-borne conservative solute within the specified  
465 concentration range. The average flux and zone over this mixing period were computed and  
466 termed the mixing flux ( $MF$ ) and mixing zone, respectively.

467 The mixing fraction ( $F_{mix}$ ) was computed as the ratio of the mixing flux to the total  
468 hyporheic flux, which was derived from integrating the volumetric flux along the sediment-  
469 water interface (including both SW OUT and GW OUT). ~~This total flux is triggered by both~~  
470 ~~pumping and bedform migration, and is simultaneously influenced by upwelling~~  
471 ~~groundwater~~  $F_{mix}$  serves as a metric to quantify the proportion accounted for by the SW-GW  
472 mixing flux within the overall flux of SW and GW interaction across the sediment-water  
473 interface.  $A_{mix}$  is defined as the ratio of the mixing zone to the entire modeled domain,  
474 representing the proportion of the riverbed occupied by the mixing zone.

475 The mixing zone is defined as the area where the groundwater proportion varies between  
476 16% and 84% (Santizo et al., 2020). The size of the surface water and groundwater mixing zone  
477 ( $A_{mix}$ ) was calculated by integrating the area where the concentration of the conservative tracer  
478 ranges from 0.16 to 0.84 mg/L.

## 479 2.6.2 Nitrate reaction rate and efficiency

480 When the hydro-physical and biogeochemical conditions reach a quasi-steady state, ~~we~~  
481 ~~select the last 10 periods of ripple migration and calculate the~~ average flux of total amount of  
482 stream- or groundwater- borne nitrate entering ~~being induced into~~ the riverbed layer over the

final ~~during several periods of ripple migration~~ the time interval was calculated:

$$M_{in,s-NO_3^-} = \frac{1}{w} \sum_T B \int v c_{s-NO_3^-} - \theta D_{ij} \cdot \nabla c_{s-NO_3^-} dL_{top} \quad (251)$$

$$M_{in,g-NO_3^-} = \frac{1}{w} \sum_T B \int v c_{g-NO_3^-} - \theta D_{ij} \cdot \nabla c_{g-NO_3^-} dL_{bottom} \quad (262)$$

where  $B$  [L] is the per unit width,  $\theta$  [-] is the sediment porosity, and  $w = T/dt$  [-];  ~~$F_{SWT}$  [ $L \cdot T^{-1}$ ] is~~  
~~the inward flux across the ripple surface~~  $L_{top}$  and  $L_{bottom}$  refer to the top boundary BC and bottom  
boundary AD, respectively.

The total amount of nitrate removed by ~~non-mixing-dependent (NMD)~~ denitrification and  
~~mixing-dependent (MD)~~ denitrification were are calculated as follows during the same time  
interval:

$$M_{NMD} = \frac{1}{w} \sum_T B \int R_{DN}(c_{s-NO_3^-}) dA \quad (273)$$

$$M_{MD} = \frac{1}{w} \sum_T B \int R_{DN}(c_{g-NO_3^-}) dA \quad (284)$$

where  $A$  [ $L^2$ ] is the area of the streambed.

Nitrate removal efficiency was quantified as the rate of nitrate removed from the riverbed  
layer divided by the nitrate flux entering the riverbed layer ~~The nitrate removal efficiency is~~  
~~defined as the ratio of the amount of nitrate being removed by DN to the amount of nitrate being~~  
~~induced into the riverbed:~~

$$N_{RE-NMD} = \frac{M_{NMD}}{M_{in,s-NO_3^-}}, \quad N_{RE-MD} = \frac{M_{MD}}{M_{in,g-NO_3^-}} \quad (295)$$

## 500 **3. Results**

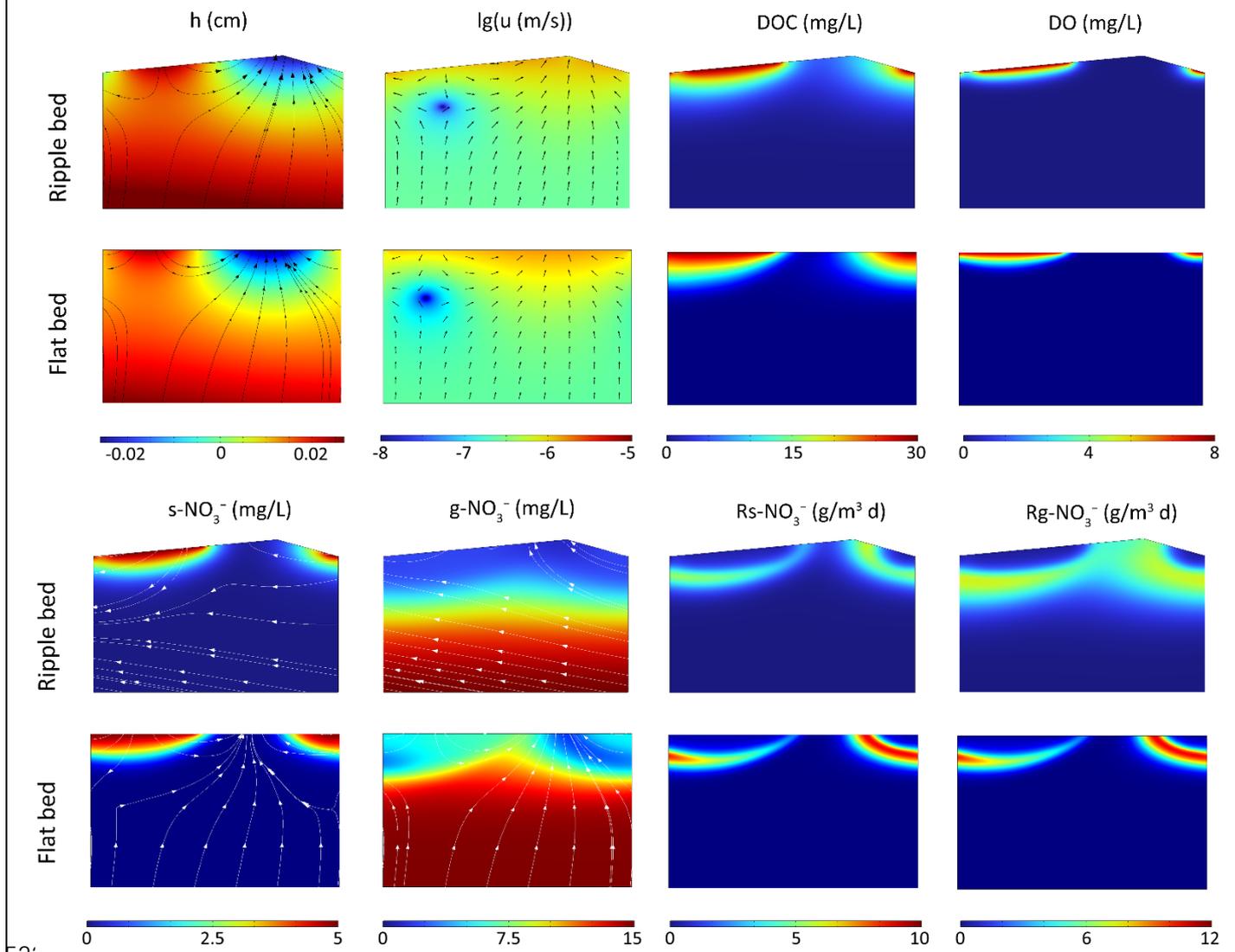
### 501 **3.1 Model validation**

#### 502 3.1.1 Validation of flat bed model with triangular ripple model

503 Model validation was conducted in two steps, with two numerical methods employed to  
504 represent moving bedforms. Beyond simulating porewater flow and reactive solute transport in a  
505 fixed frame of reference with a flat riverbed surface, a reference frame moving with triangular  
506 ripples was also adopted to capture bedform migration, following the methods of Bottacin-  
507 Busolin and Marion (2010), Ahmerkamp et al. (2015), and Kessler et al. (2015). The latter  
508 method captures bedform migration while preserving the ripple shape, and comparisons between  
509 the two approaches were conducted to examine the impact of neglecting ripple geometry on the  
510 simulated results.

511 A typical case ( $Re = 3000$ ,  $U_b = 0.6$  and  $D_{50} = 0.15$  mm) was chosen here for comparison.  
512 The overall patterns of pressure head, Darcy velocity, and stream-borne solute plume  
513 distributions were mostly similar (Figure 4). The difference in groundwater-borne nitrate plumes  
514 is attributed to the distinct advection velocities for reactive solute transport. Specifically, the  
515 moving reference frame involves a transformation of the horizontal coordinate system, based on  
516 the assumption that the frame of reference moves in the direction of bedform migration and at  
517 the same migration celerity; in this frame, the modeled advection velocity of reactive solutes is  
518 the seepage velocity minus the celerity. Thus, in the triangular ripple model,  $g\text{-NO}_3^-$  flows  
519 upstream with larger advection velocities, resulting in a more uniform solute plume; in contrast,  
520 it exits the riverbed across the entire riverbed surface in the flat bed model (see the white

521 streamlines). Correspondingly, discrepancies emerge in the crest area of the MD denitrification  
522 zone.



523 Figure 4. Comparison of hyporheic flow and reactive solute transport modeling results from the  
524 flat bed model and the triangular ripple model. Streamlines and arrows in the Darcy flow field  
525 indicate porewater flow, while streamlines in nitrate solute plumes represent the advection  
526 velocity of reactive solute transport.

527 Compared with the triangular ripple model, the flat bed model exhibit slightly larger  
528 hyporheic exchange flux, mixing flux, and stream-borne nitrate influx. The relative errors are  
529

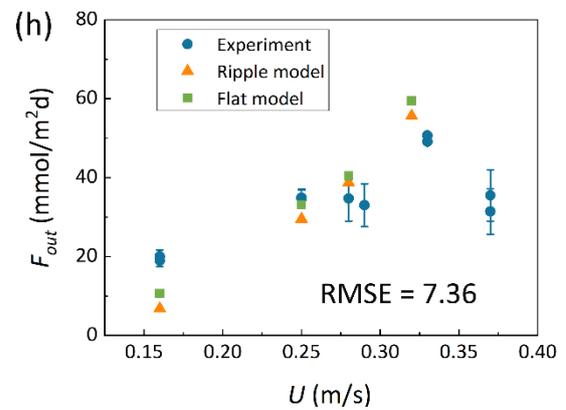
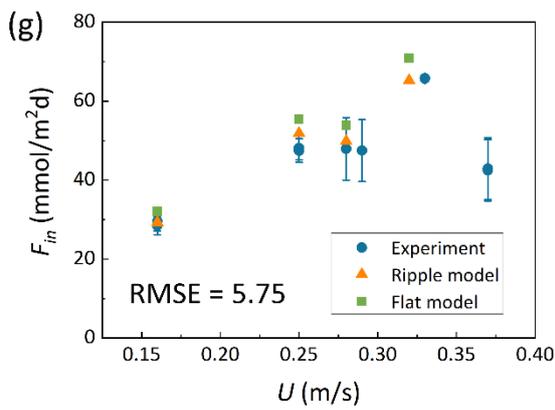
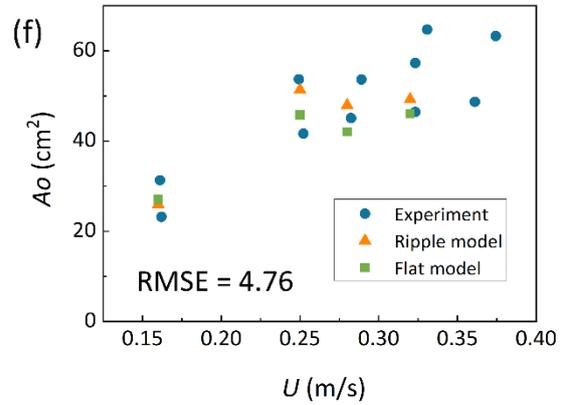
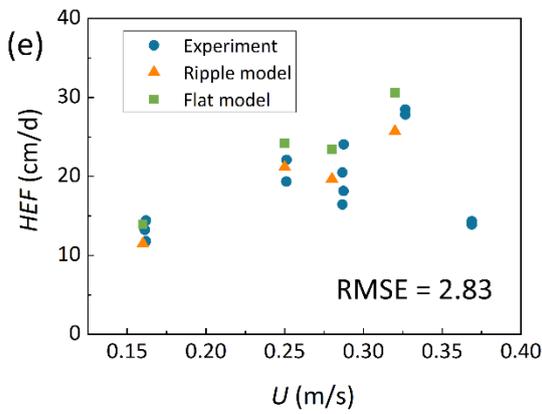
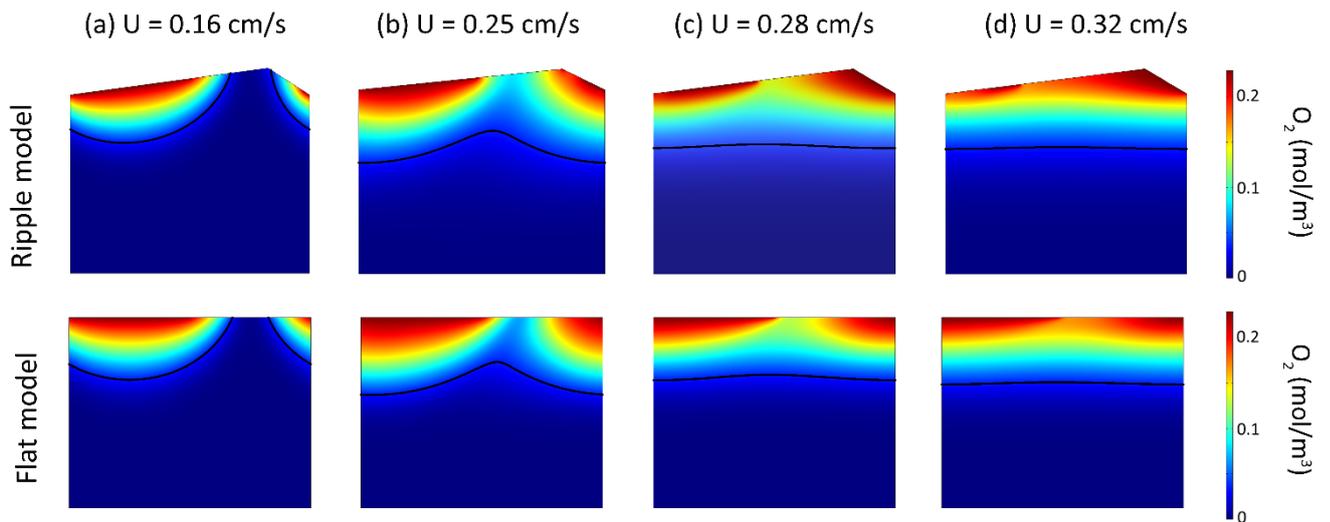
530 approximately 10%. Meanwhile, both the MD and NMD denitrification rates are higher in the  
531 triangular ripple model than in the flat bed model, with relative errors around 20%. Differences  
532 in the results of the simulated domain cause subtle changes in flow and transport processes,  
533 changes that are acceptable for our intended purposes. To further assess the differences between  
534 the two models and their reliability, two methods were utilized to perform validation against the  
535 experiments by Wolke et al. (2020).

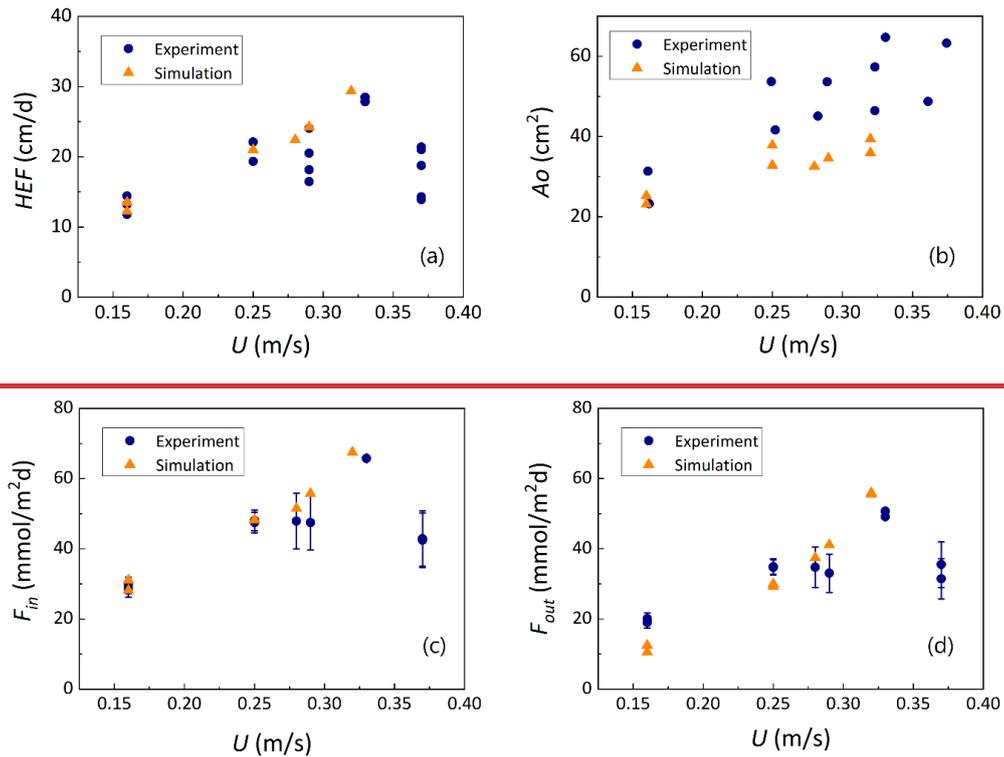
### 536 3.1.2 Validation of numerical models with laboratory experiments

537  
538 The developed models ~~model development~~ were validated through ~~by~~ comparison with ~~to~~  
539 ~~flume experiments of~~ Wolke et al.'s (2020) flume experiments, which investigated the  
540 evolution of oxygen in the riverbed under varying conditions ~~that were conducted to study the~~  
541 ~~evolution of oxygen in the riverbed under different conditions~~ of mean stream velocity  
542 (0.16–0.32 m/s) and bedform migration celerity (0–0.394 cm/h). ~~No upwelling flux of~~  
543 ~~groundwater was considered at the bottom of the riverbed.~~ The experiment was designed with a  
544 total of 5 operating conditions, each of which was repeated twice and labeled as Set 1 and Set 2.  
545 Oxygen distribution within sediments was measured via planar optodes, and the oxygenated  
546 zone was defined as regions where oxygen saturation exceeds 15%. Oxygen consumption was  
547 observed within riverbed layer and aerobic respiration is widely recognized as the dominant  
548 process driving oxygen consumption within riverbed sediments (Ahmerkamp et al., 2017;  
549 Reeder et al., 2018). In their experiments, a fixed amount of NaCl was added to surface water;  
550 this NaCl was then used to compute hyporheic exchange flux (HEF) from the decline in its  
551 concentration, following the method by Fox et al. (2018). The oxygen influx ( $F_{in}$ ) and outflux

552 ( $F_{out}$ ) were calculated by multiplying  $HEF$  by the surface water oxygen concentration and the  
553 mean oxygen concentration in the oxygenated zone, respectively.

554 ~~Based on the criteria for ripple migration, it was determined that under the hydraulic~~  
555 ~~conditions of Run 5, ripples could not migrate while maintaining their shapes due to increased~~  
556 ~~flow intensity. Therefore, the model validation simulations considered four hydraulic conditions~~  
557 ~~of Run 1 to 4.~~





559  
 560 **Figure 53.** Comparison of numerical modeled (a) hyporheic exchange flux, (b) summed oxic  
 561 zone and (c) oxygen influx and (d) oxygen outflux and experimental measurements by Wolke et  
 562 al. (202019) under various conditions.

563 Two numerical models: the flat bed model and the triangular ripple model, were developed  
 564 under the same hydraulic conditions, with their parameters adjusted to fit  $HEF$ ,  $F_{in}$ ,  $F_{out}$ , and  $A_o$   
 565 to experimental data. In the numerical model,  $HEF$  was calculated by integrating the volumetric  
 566 flux over the inflow zones along the sediment-water interface. Oxygen transport was governed  
 567 by the advection-dispersion-reaction equation, where oxygen consumption occurred through  
 568 aerobic respiration following the Monod kinetics described by Equation (8). The  $F_{in}$  and  $F_{out}$   
 569 were then computed by multiplying the  $HEF$  by the oxygen concentration in the surface water  
 570 and the oxygen concentration over the outflow zones along the riverbed surface, respectively.  
 571 The oxygenated area was calculated by integrating the riverbed portions where oxygen  
 572 saturation exceeds 15%. Based on the criteria for ripple migration, it was determined that under

573 the hydraulic conditions of Run 5, ripples could not migrate while maintaining their shapes due  
574 to increased stream flow velocity. Therefore, the model validation simulations considered four  
575 hydraulic conditions of Run 1 to 4. The model parameters used and adjusted were summarized  
576 in Supporting Information Text S3: Model Calibration.

577 The parameters used for model validation are shown in supporting information Table S1  
578 and S2. For stationary and slow-migrating beds, the spatial distribution of oxygen creates a  
579 conchoidally shaped plume beneath the bedform; in contrast, beneath fast-migrating beds, the  
580 oxygen plume shifts to a more uniform front. The  $HEF$ ,  $F_{in}$ ,  $F_{out}$ , and  $A_O$  simulated using both  
581 the ripple bedform model and the flat bed model were presented in Figure 5; these results are in  
582 good qualitative and quantitative agreement with the measurements of Wolke et al. (2020). The  
583 root means square error (RMSE) were further calculated between the experimentally measured  
584 values and those simulated by the flat bed model. All RMSE values fall within reasonable  
585 ranges, indicating the critical processes of bedform migration and oxygen dynamic are captured  
586 by the numerical models. In stationary and slow-migrated beds, the spatial distribution of  
587 oxygen creates a typical conchoidally shaped plume in the riverbed. In contrast, for fast-  
588 migrated beds, the oxygen plume becomes a more uniform front (Ping et al., 2022). The  
589 comparison of modeled oxygen distributions with experimental measurements reveals that  
590 simulated values of the oxygenated zone was slightly lower than observed. This discrepancy is  
591 mainly attributed to two factors: firstly, the dissolved oxygen concentrations measured by the  
592 planar optode system were relatively high, as noted in Wolke et al. (2019) themselves. Secondly,  
593 only oxygen fluxes within immobile riverbeds were simulated and did not include the areas of  
594 the mobile sections in the numerical modeling. Because the mobile riverbeds exist in oxygen-

rich environments, this exclusion led to the simulated values being lower than the measured values. Overall, the simulated hyporheic exchange flux, oxygen area and oxygen fluxes are displayed in Figure 3 and they are in good agreement with the measurements of Wolke et al. (2019) in trend, suggesting that the mobile bedform model is capable to reproduce realistic conditions well.

### 3.2 Effect of bedform migration on mixing regimes and solute dynamic transport

To simulate the range of natural environmental conditions, the reactive transport equations were solved for different stream velocities, grain sizes and groundwater upwelling fluxes, which include a corresponding range of ripple migration celerities and sediment permeabilities derived from the empirical relations. As an example, the patterns of pore water transport as well as SW and GW mixing are shown for a grain size of 0.15 mm, a constant ratio between pumping driven hyporheic exchange flux and upwelling GW flux  $U_b = 0.6$  and four different stream velocities, that is four different  $Re$  numbers.

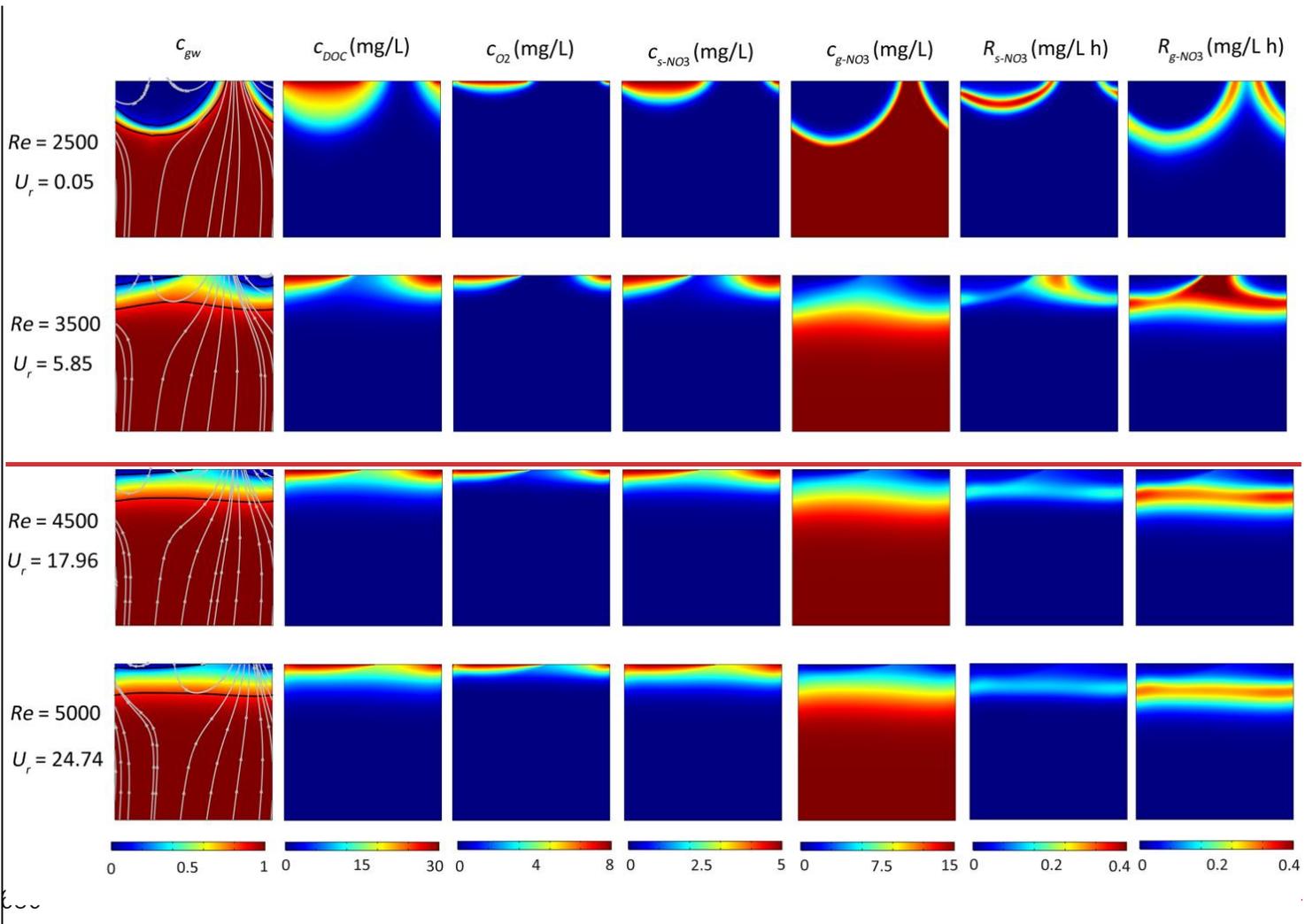
For low surface water flow velocity ( $Re = 2500$ ,  $U = 0.125$  m/s), no migration of bedform was predicted by the model. SW enters the riverbed layer sediment in the high-pressure region on the stoss side, flows through the porous medium, and exits the riverbed layer bedform in the low-pressure region on the lee side, forming a typical conchoidally shaped hyporheic flow cell. Upwelling GW is diverted around the hyporheic flow cell, mixes with SW, and then exits into the overlying water from both sides, in patterns similar to those shown previously by Fox et al. (2014) and Hester et al., (2019). SW and GW mixing zone (i.e. the mixing area where the fraction of GW ranges between 16% and 84%) emerges as a thin band along the hyporheic flow cell, and covers over 2.978% of the modeled domain. Meanwhile, NMD denitrification occurs

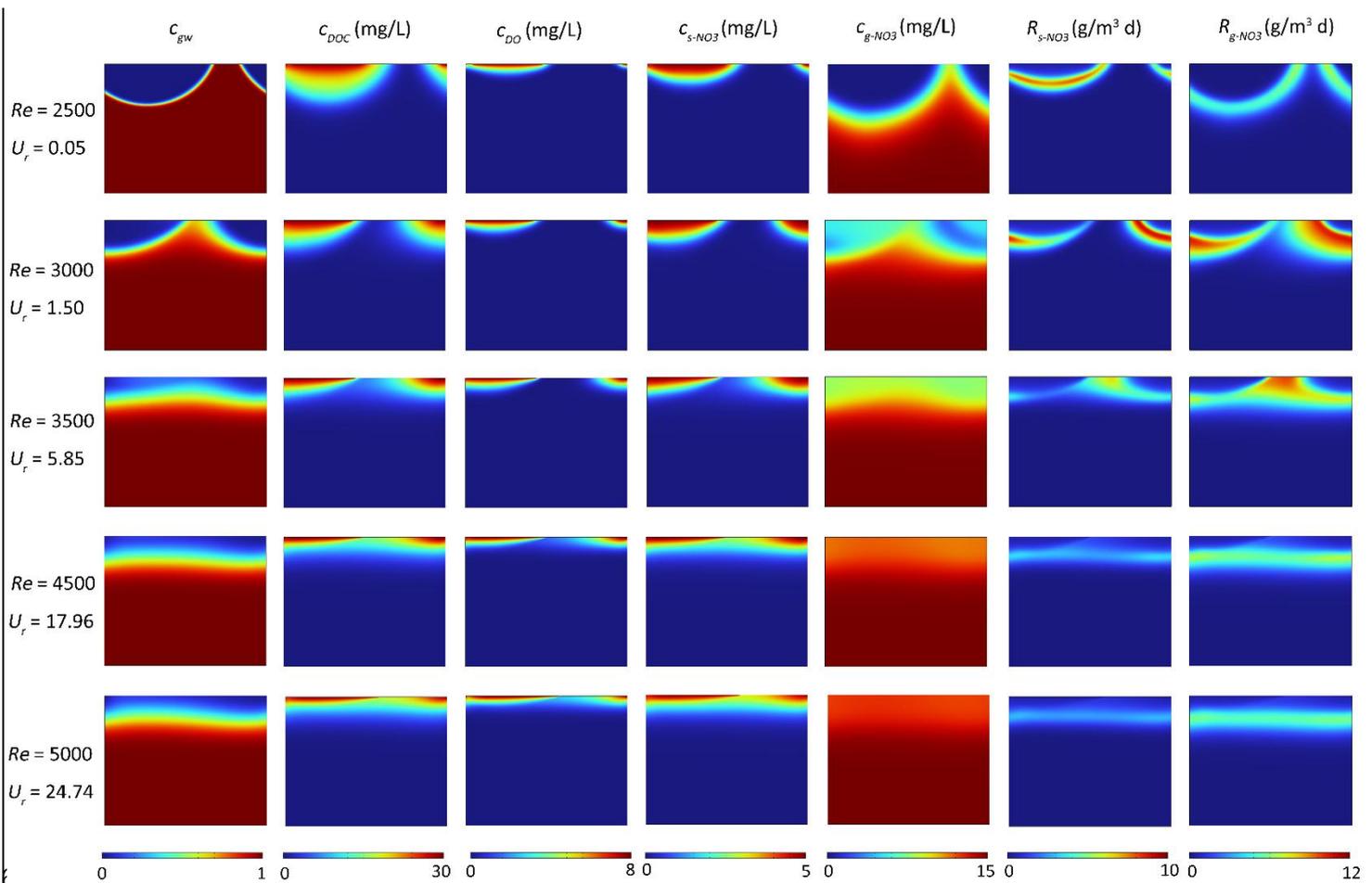
617 below the oxygen plume with the reactive zone in a conchoidal shaped distribution, while MD  
618 denitrification reactive zone develops along the edge of the mixing zone, where DOC from SW  
619 meets nitrate from GW (see row 1 in Figure 46).

620 At a surface water  $Re$  of 3000, bedforms migrate at a low celerity of 1.04 cm/h. This low  
621 migration rate has minimal impact on the plumes of stream-borne  $O_2$  and s- $NO_3^-$ . The outflow  
622 zone of groundwater-borne solutes ( $c_{gw}$  and g- $NO_3^-$ ) exhibits a slight upstream shift,  
623 approaching the middle of the riverbed surface. The size of SW and GW mixing zone (GW  
624 fraction: 16%–84%) increases to 10.06% of the modeled domain. Furthermore, both the NMD  
625 and MD denitrifying rates are enhanced at the start of bedform migration (row 2 in Figure 6).

626 As stream flow velocity increases ( $Re = 3500$ ,  $U = 0.175$  m/s), this changes the pressure  
627 distribution patterns with its zones of high and low pressure. Consequently, the simulated  
628 hyporheic flow cells move downstream, while simultaneously also shrinking in size. The shape  
629 of the SW and GW mixing zone changes distinctly, forming a horizontal band with a wider  
630 range 17.01% of the whole domain. The penetration of stream-derived solutes into the streambed  
631 is reduced, with a more gradual concentration gradient in the horizontal and vertical directions,  
632 whereas the g- $NO_3^-$  plume is uniformly distributed horizontally. Both NMD and MD  
633 denitrification hot spots form in the central position near the sediment-water interface as the  
634 bedform surface (see row 2 in Figure 4).

635





638 **Figure 46.** Effect of bedform migration on riverbed biogeochemistry for  $U_b = 0.6$  and  $D_{50} =$   
639  $0.15$  mm. Shown are profiles of (column 1) conservation solute representing groundwater  
640 fraction ( $c_{gw}$ ), (column 2) DOC concentration ( $c_{DOC}$ ), (column 3)  $O_2$  concentration ( $c_{O_2}$ ),  
641 (column 4)  $s\text{-NO}_3^-$  concentration ( $c_{s\text{-NO}_3^-}$ ), (column 5)  $g\text{-NO}_3^-$  concentration ( $c_{g\text{-NO}_3^-}$ ), (column  
642 6) non-mixing-dependent (NMD) denitrification rate ( $R_{s\text{-NO}_3^-}$ ) and (column 7) mixing-dependent  
643 (MD) denitrification rate ( $R_{g\text{-NO}_3^-}$ ).

644 With increasing stream velocity ( $Re = 3500$ ), bedform migration drives continuous changes  
645 in pressure distribution. The simulated hyporheic flow cells move downstream and decrease in  
646 size at the same time. The shape of the SW and GW mixing zone changes distinctly, forming a  
647 horizontal band with a wider range 10.717-01% of the whole domain (GW fraction: 16%–84%).

648 The penetration of stream-derived solutes into the streambed is reduced, with a more gradual  
649 concentration gradient in the horizontal and vertical directions, whereas the  $g\text{-NO}_3^-$  plume is  
650 uniformly distributed horizontally. Both NMD and MD denitrification hot spots form in the  
651 central position near the sediment-water interface as the bedform moving (row 23 in Figure 6).

652 ~~As stream flow velocity increases ( $Re = 3500$ ,  $U = 0.175$  m/s), this changes the pressure~~  
653 ~~distribution patterns with its zones of high and low pressure. Consequently, the simulated~~  
654 ~~hyporheic flow cells move downstream, while simultaneously also shrinking in size. The shape~~  
655 ~~of the SW and GW mixing zone changes distinctly, forming a horizontal band with a wider~~  
656 ~~range 17.01% of the whole domain. The penetration of stream-derived solutes into the streambed~~  
657 ~~is reduced, with a more gradual concentration gradient in the horizontal and vertical directions,~~  
658 ~~whereas the  $g\text{-NO}_3^-$  plume is uniformly distributed horizontally. Both NMD and MD~~  
659 ~~denitrification hot spots form in the central position near the sediment-water interface as the~~  
660 ~~bedform surface (see row 2 in Figure 4).~~

661 When bedform migration is further increased ( $Re = 4500$ ,  $U_f = 17.96$  and  $5000$ ,  $U_f = 24.74$ ), the  
662 bedform migration fully dominates over the pore water flow, and hence, continuous solute layers  
663 are found in the subsurface (as depicted in row 43 and 54 of Figure 64). The penetration depths  
664 of stream-borne solutes are decreased in comparison to those in slow- to medium- fast migrating  
665 bedforms. The NMD and MD denitrification zones become thin and move upward with  
666 decreased reaction rates. Similar to the conclusions obtained in previous studies (Kessler et al.,  
667 2015; Zheng et al., 2019), ~~the migration of the~~ bedform migration reduces the penetration depth  
668 of solute and the scope of hyporheic exchange cell. We also found that bedform a larger  
669 migration celerity increases the size of mixing zone between surface water and groundwater.

670 The SW and GW mixing zone accounts for 9.3017.74% and 9.1417.86% of the domain area,  
671 respectively (take the GW fraction 16%–84% for example).

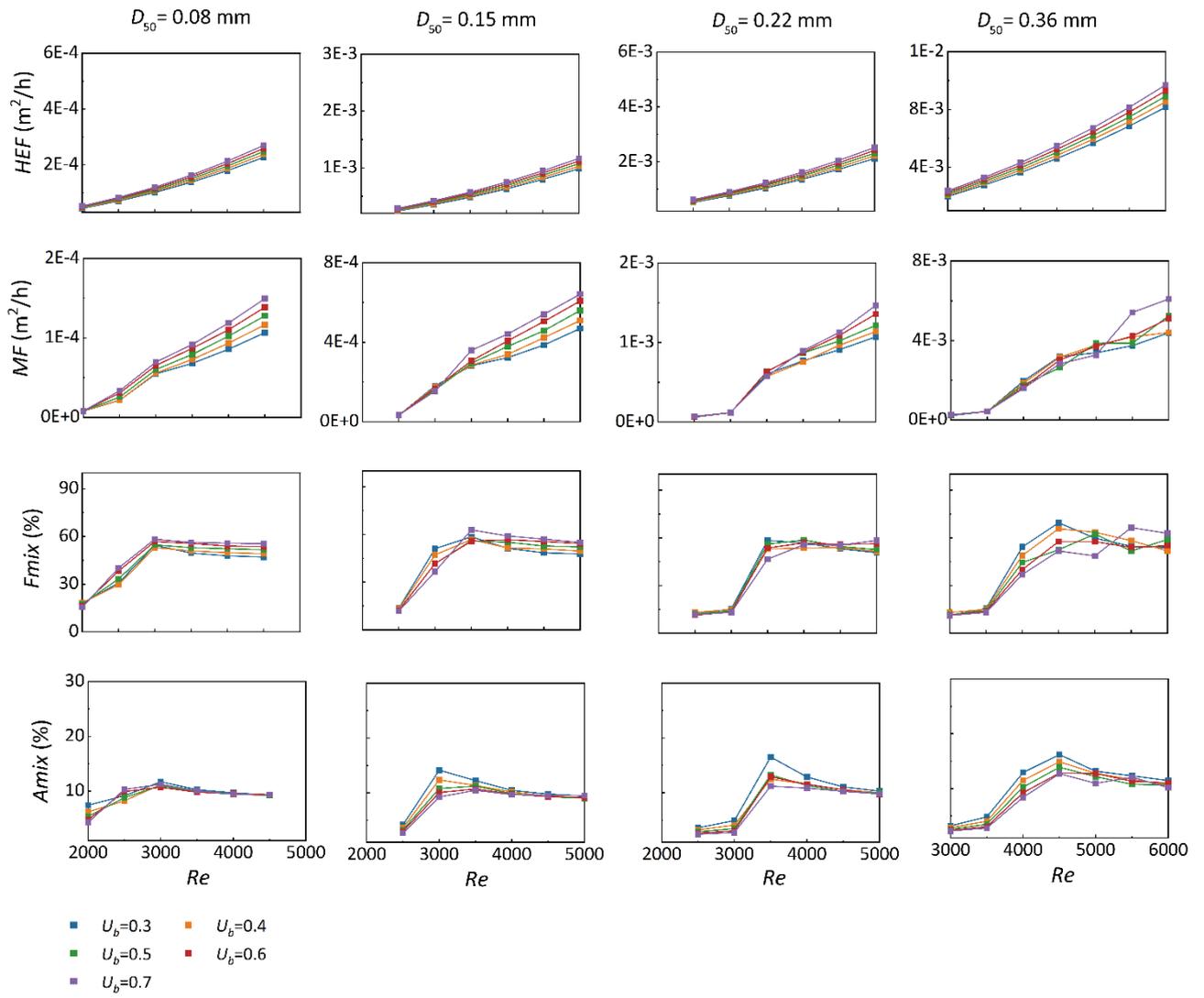
### 672 3.3 Effect of migration celerity on mixing regimes

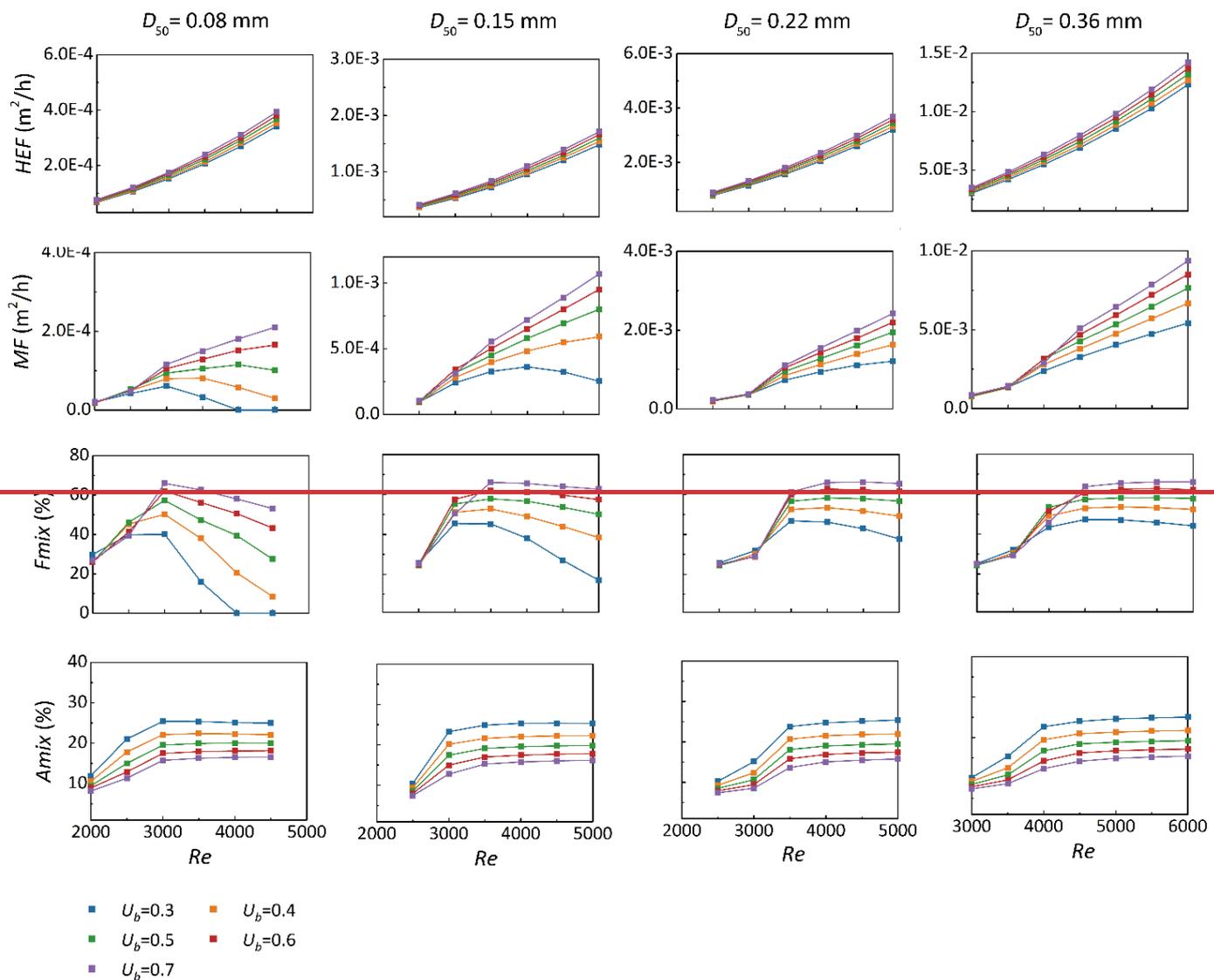
673 The mixing intensity across the riverbedbedform surface and the size of the mixing area  
674 within the subsurface wereare estimated by simulating four different grain sizes and five  
675 upwelling groundwater GW fluxes, under varying stream velocities associated with the  
676 corresponding bedform celerity. The evolutions of the hyporheic exchange flux, net mixing flux  
677 and size of mixing zone with increasing  $Re$  number wereare summarized in Figure 57. The 16–  
678 84% GW fraction range was displayed here to evaluate how migration celerity affects mixing  
679 regimes. In the third row of Figure 5,  $F_{mix}$  represents the ratio of the net mixing flux to the total  
680 hyporheic exchange flux. This total flux is triggered by both pumping and bedform migration,  
681 and is simultaneously influenced by upwelling groundwater.  $F_{mix}$  serves as a metric to quantify  
682 the proportion accounted for by the surface water–groundwater mixing flux within the overall  
683 flux of surface water and groundwater interaction across the sediment–water interface.

684 As shown in Figure 5, the hyporheic exchange flux increases as the stream velocity and  
685 bedform celerity rise across various grain sizes of the bedform, meanwhile the mixing flux  
686 across the sediment–water interface also increases with the increasing stream velocity, except for  
687 some special circumstances. For riverbed consists of very fine and fine sand ( $D_{50} = 0.08–0.15$   
688 mm) under moderate groundwater discharging conditions ( $U_b < 0.5$ ), the mixing flux increases  
689 significantly at the start of bedform migration, and then the mixing flux across the sediment  
690 water interface begins to decline with increasing celerity. In fact, this is because higher  
691 migration velocities of a riverbed with relatively low permeability limits the discharge of

692 ~~groundwater into the river (a horizontal distribution of stream borne solute plume), leading to~~  
693 ~~the mixing of SW and GW primarily occurring within the streambed (see the column 1 in Figure~~  
694 ~~4). Concurrently, only the mixing flux through the water-sediment interface is reduced at this~~  
695 ~~time.~~

696 For medium sand ( $D_{50}=0.22-0.36$  mm), we found that the SW and GW mixing flux  
697 demonstrates a substantial increase relative to stationary bedforms when bedform migration  
698 initiates at moderate velocities. As migration celerity accelerates, the magnitude of mixing flux  
699 gradually approaches a plateau, with only marginal reductions observed at higher migration  
700 celerities. Besides, the size of SW and GW mixing zone also exhibit the similar trends.





702

703 **Figure 75.** The variation of hyperheic exchange flux ( $HEF$ , row 1) and mixing flux ( $MF$ , row 2)

704 across the sediment-water interface, the proportion of mixing flux to hyperheic exchange flux

705 ( $F_{mix}$ , row 3) and the ratio size of mixing zone to whole domain ( $A_{mix}$ , row 4) with stream

706 velocity and associated bedform celerity across different medium grain size when  $U_b = 0.3-0.7$ .

707 As shown in Figure 7, across various grain sizes, both  $HEF$  and  $MF$  increase with

708 increasing stream velocity and bedform celerity. Nevertheless, the mixing fraction exhibits a

709 significant increase at the onset of bedform migration, with a tendency to stabilize or decrease

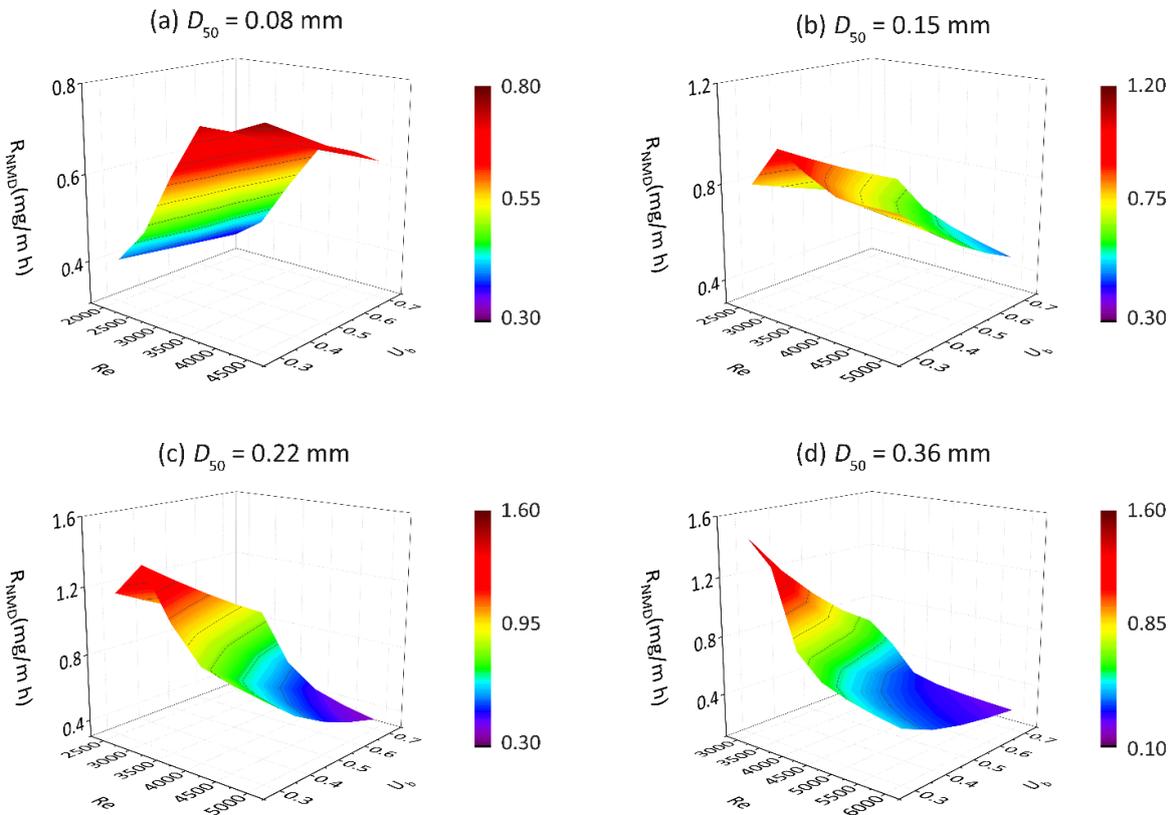
710 moderately as celerity further increases. This phenomenon is attributed to enhanced mixing  
711 between SW and discharging GW along the entire horizontal riverbed in moving bedforms—  
712 contrasting with stationary bedforms, where mixing occurs solely along the typical crescent-  
713 shaped hyporheic exchange flow cells (Figure 6). Additionally, when bedforms start moving and  
714 move at moderate velocities, the SW-GW mixing zone shows a significant increase compared to  
715 stationary ones. As bedform migration celerity accelerates, the extent of the mixing zone  
716 gradually approaches a plateau, with slight reductions observed at higher celerities.

717 Although the GW fraction range determines both mixing flux and scope, it does not alter  
718 the effect of migration celerity on these mixing patterns. A broader groundwater fraction range  
719 induces a larger mixing flux, an increased  $F_{mix}$ , and a wider mixing zone; nonetheless,  $A_{mix}$   
720 persists as a relatively small scope (about 10–15% of the full domain) within the riverbed.  
721 Relatively moderate differences in computed  $MF$  and  $F_{mix}$  are observed between the narrower  
722 range (20–80%) and the baseline range (16–84%). For bedforms of different sediment grain  
723 sizes,  $MF$  represents around 60% of the total  $HEF$  when bedforms are moderately to fast-  
724 moving, compared with approximately 20% when they are stationary or slow-moving.  
725 Additionally,  $F_{mix}$  and  $A_{mix}$  values obtained under various GW fraction ranges were presented  
726 in the Supporting Information (Text S4). Overall, bedform migration controls the shape and size  
727 of the SW and GW mixing zone, enhances the magnitude of hyporheic exchange flux and  
728 mixing flux. The mixing flux and size is also influenced by the upwelling GW flux. As  $U_b$   
729 increases from 0.3 to 0.6, the mixing flux and the proportion of mixing flux to total hyporheic  
730 exchange flux rise significantly. An increase in  $U_b$  reduces the size of the mixing zone because  
731 both the hyporheic exchange flow cell and the mixing zone are confined to shallower depths

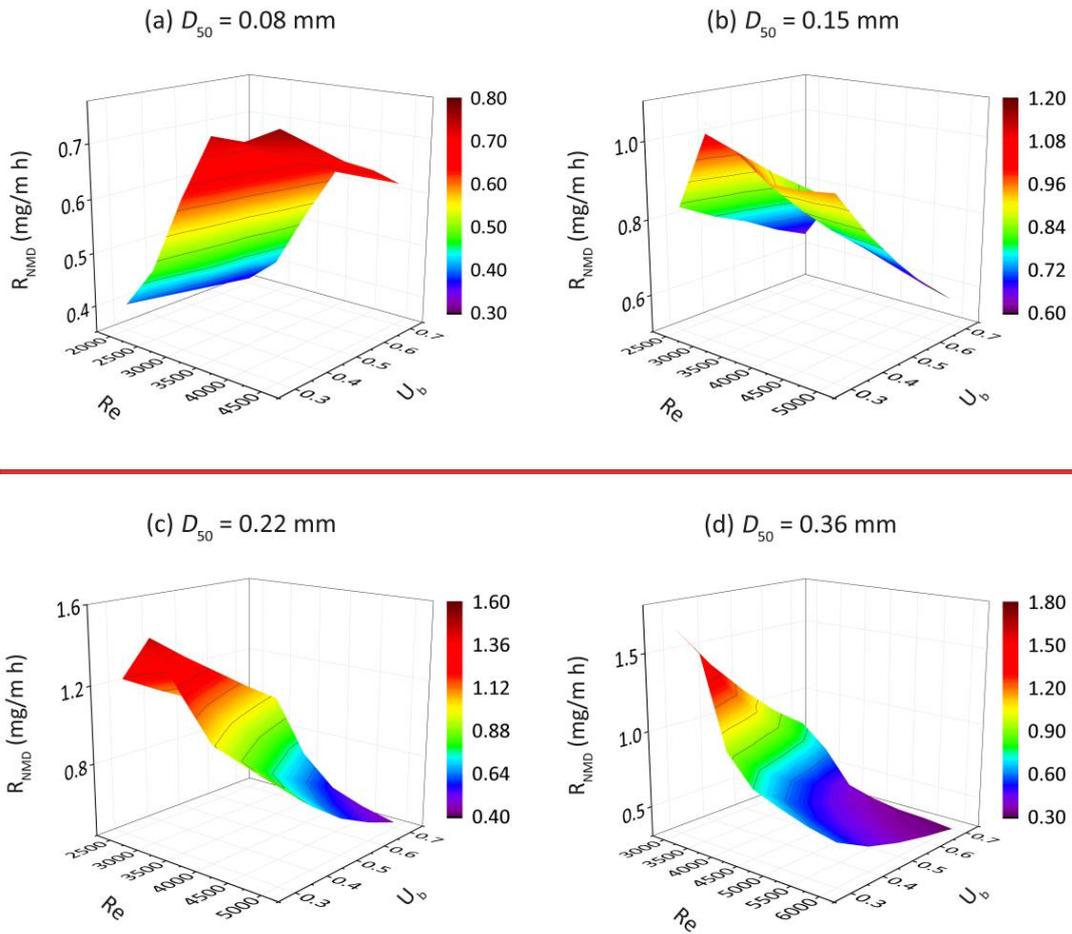
732 within the riverbed due to the larger upward flow.

### 733 3.4 Impact of ripples migration on nitrate removal

734 To assess the impact of ripple migration on the removal of s-NO<sub>3</sub><sup>-</sup> and g-NO<sub>3</sub><sup>-</sup> within  
735 domains of varying medium grain sizes, the influx of nitrate into the riverbed layer and the total  
736 reaction rate within the riverbed layer were determined.



737

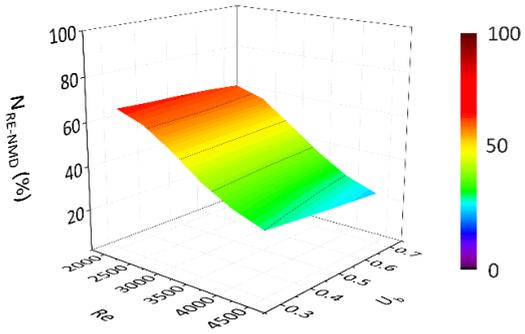


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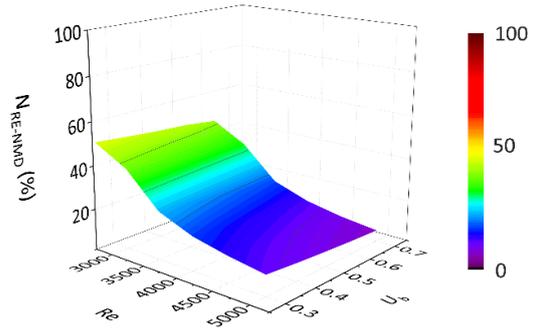
739 **Figure 86.** The non-mixing-dependent denitrification rates ( $R_{NMD}$ ) as functions of  $U_b$  and  $Re$  for

740 different medium grain sizes.

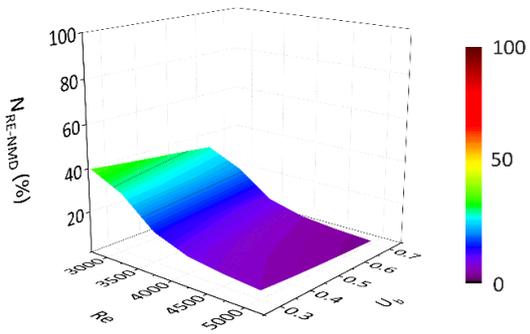
(a)  $D_{50} = 0.08$  mm



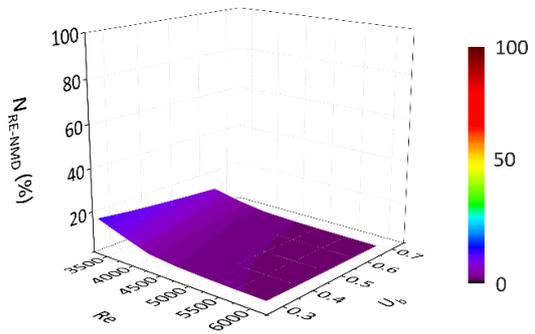
(b)  $D_{50} = 0.15$  mm



(c)  $D_{50} = 0.22$  mm

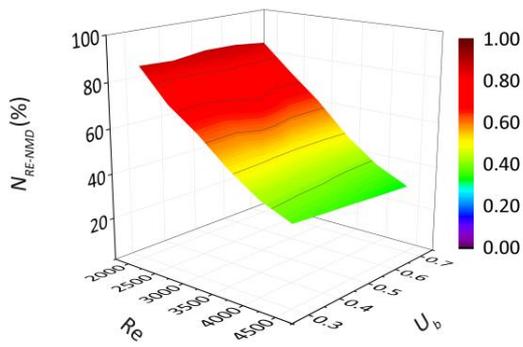


(d)  $D_{50} = 0.36$  mm

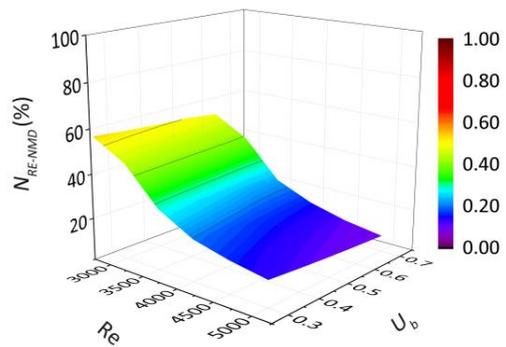


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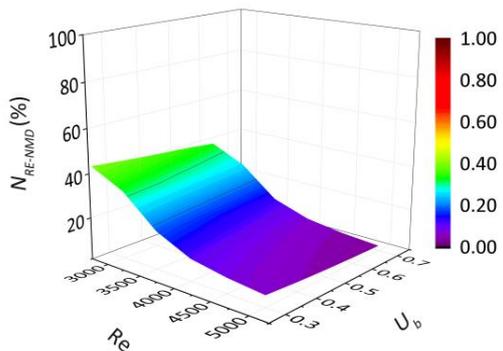
(a)  $D_{50} = 0.08$  mm



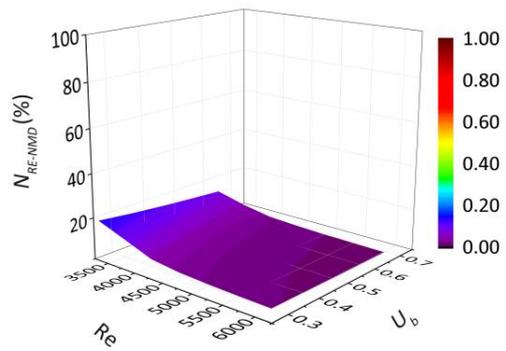
(b)  $D_{50} = 0.15$  mm



(c)  $D_{50} = 0.22$  mm



(d)  $D_{50} = 0.36$  mm

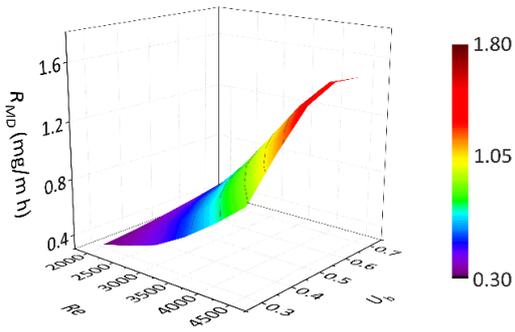


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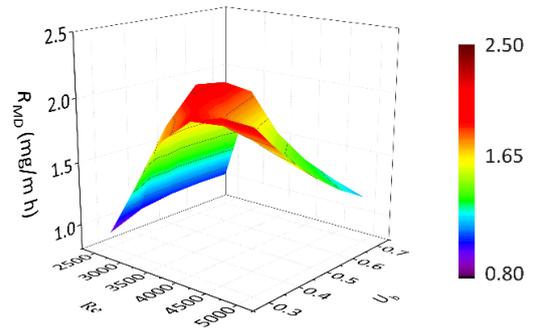
743 **Figure 97.** The removal efficiencies of stream borne nitrate ( $N_{RE-NMD}$ ) as functions of  $U_b$  and  $Re$   
744 for different medium grain sizes.

745 For  $s\text{-NO}_3^-$ , the NMD denitrification rate increases in moving bedforms ~~with both the~~  
746 ~~overlying water velocity and the migration celerity composed of~~ in very fine sand ( $D_{50} = 0.08$   
747 mm). This is likely caused by higher flow velocities driving more reactants into ~~driving longer~~  
748 ~~advective flow paths and increase solute residence times within~~ the sediment, thereby enhancing  
749 denitrification in reaction-limited systems ( $Da > 2.9785$ ). In contrast, in riverbeds of fine to  
750 medium sand ( $D_{50} = 0.15\text{--}0.36$  mm) with higher permeabilities, the ~~reduction~~ rate of  $s\text{-NO}_3^-$  is  
751 negatively correlated with the mean stream velocity when the system becomes transport-limited  
752 ( $Da < 2$ ). This is likely because  $s\text{-NO}_3^-$  nitrate travels fast along shorten flow paths and does not  
753 undergo denitrification within the moving bedforms (Figure 6). Additionally, the migrating  
754 bedforms enhance the delivery of  $s\text{-NO}_3^-$  into the sediment due to increased ~~HEF hyporheic~~  
755 ~~exchange flux~~. Consequently, the removal efficiency of  $s\text{-NO}_3^-$  decreases monotonically across  
756 various medium grain sizes (Figure 97).

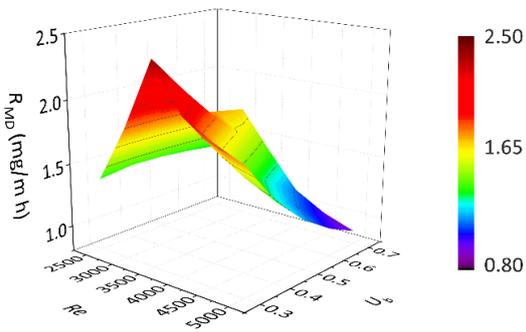
(a)  $D_{50} = 0.08$  mm



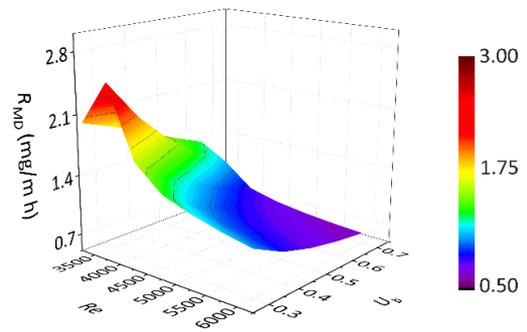
(b)  $D_{50} = 0.15$  mm

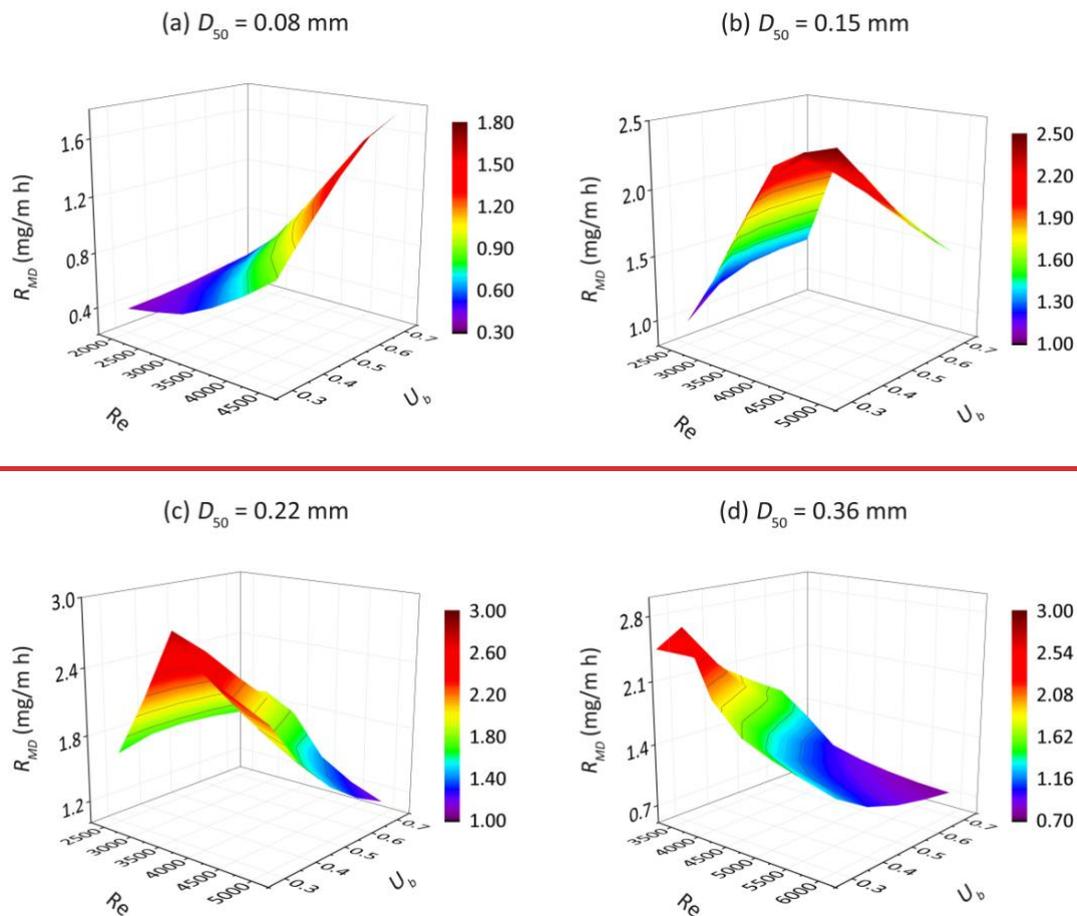


(c)  $D_{50} = 0.22$  mm



(d)  $D_{50} = 0.36$  mm



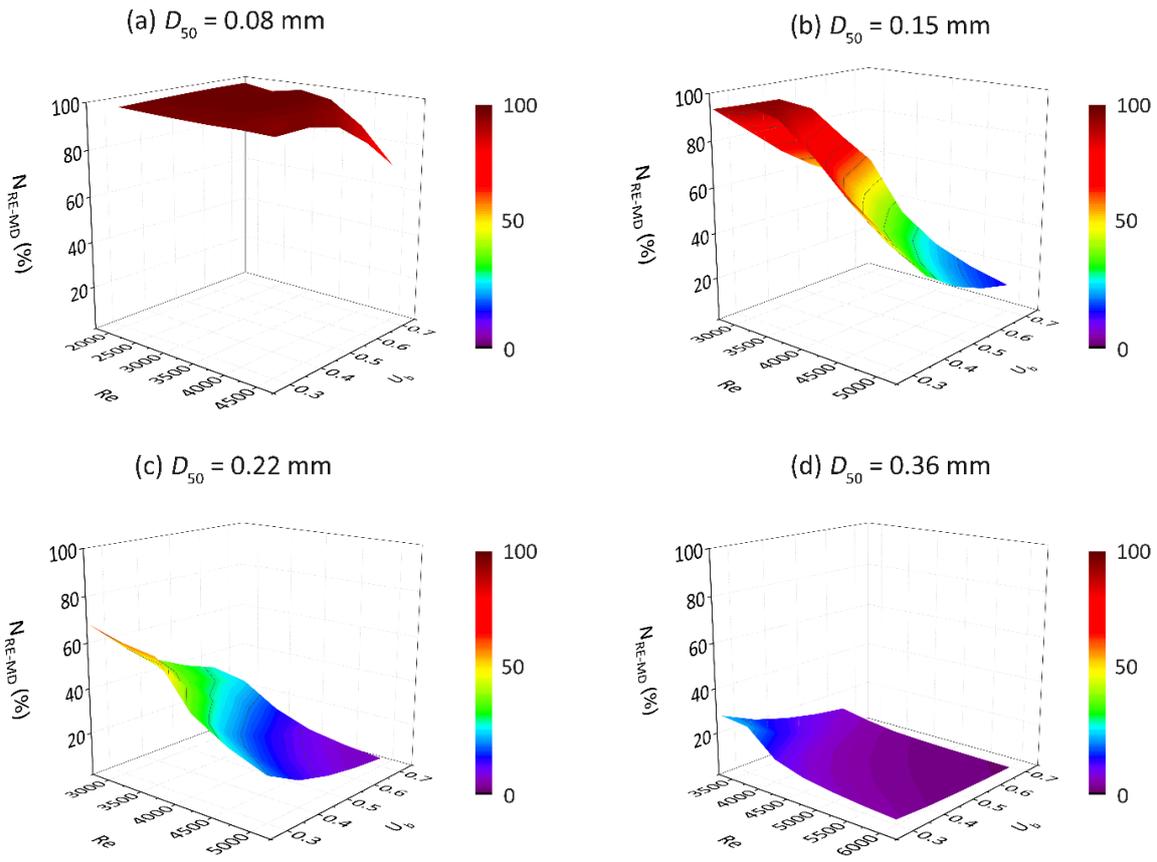


758

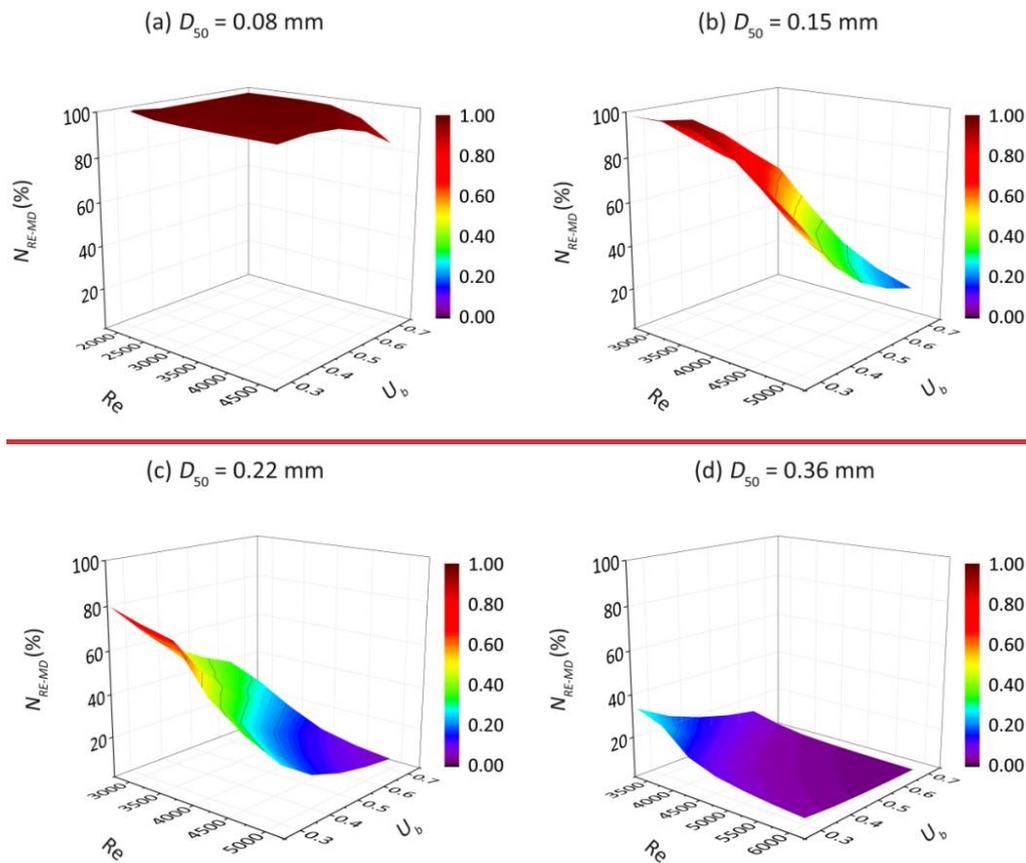
759 **Figure 108.** The mixing-dependent denitrification rates ( $R_{MD}$ ) as functions of  $U_b$  and  $Re$  for  
 760 different medium grain sizes.

761 For  $g\text{-NO}_3^-$ , the increase in MD denitrification is also seen for fine sandy sediments ( $D_{50} =$   
 762  $0.08\text{--}0.22$  mm)  $g\text{-NO}_3^-$  at low to medium  $Re$  when  $D_{50} < 0.36$  mm (Figure 108). Compared to  $s\text{-}$   
 763  $\text{NO}_3^-$ , the advective flow paths and residence times of  $g\text{-NO}_3^-$  in groundwater are longer. —  
 764 Additionally, the enhanced mixing dynamics between SW and GW further facilitates the  
 765 occurrence of MD denitrification as the bedforms start moving. Consequently, the reduction rate  
 766 of  $g\text{-NO}_3^-$  decreases only in riverbeds consisting of medium sand with fast bedform migration  
 767 celerity ( $Re > 4000$ ), as the solute residence time is significantly reduced. Interestingly, the rise  
 768 in the MD denitrification rate compensates for the increased  $g\text{-NO}_3^-$  nitrate-influx in very fine  
 769 sand ( $D_{50} = 0.08$  and  $0.15$  mm) at moderate high stream velocity ( $Re < 4000$ ). Most of  $g\text{-NO}_3^-$

770 that enters the sediment is consumed before entering the overlying water column. For fine to  
 771 medium sand riverbed ( $D_{50} = 0.15 - 0.36 \text{ mm}$ ), the  $\text{g-NO}_3^-$  removal efficiency decreases strongly  
 772 with increasing  $Re$ . The natural protective role of the SW and GW mixing zone in preventing  
 773 nitrate-contaminated GW groundwater from entering rivers is being hindered in fast moving  
 774 bedforms (Figure 911).



775



776  
 777 **Figure 119.** The removal efficiencies of groundwater borne nitrate ( $N_{RE-MD}$ ) as functions of  $U_b$   
 778 and  $Re$  for different medium grain sizes.

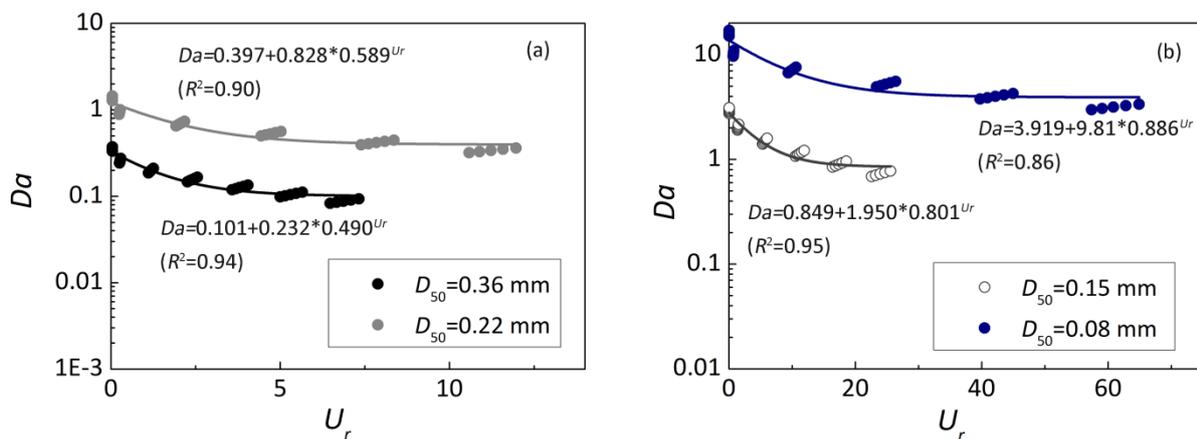
779 **4. Discussion**

780 This study ~~for the first time~~ quantified the effect of bedform migration on ~~surface water~~SW  
 781 and ~~groundwater~~GW mixing process as well as mixing triggered denitrification. Previous  
 782 research has primarily focused on the potential impacts of bedform migration on hyporheic  
 783 exchange driven by streambed morphological features, as well as non-mixing-dependent  
 784 biogeochemical processes where reactants are assumed to ~~be reside~~ predominantly in SW  
 785 ~~surface water~~. However, such studies represent only a small subset of possible streambed  
 786 environmental conditions, focusing exclusively on specific headwater stream conditions (Jiang  
 787 et al., 2022; Kessler et al., 2015; Ping et al., 2022; Zheng et al., 2019). The impact of bedform

788 migration on the conceptual model of bedform-induced hyporheic exchange, which is  
789 influenced by ~~groundwater-GW~~ upwelling and/or ambient lateral ~~groundwater-GW~~ flow in the  
790 mid-stream section of lowland rivers, has received relatively less attention and examination.

791 In streams and rivers that are fed by regional ~~groundwater-aGW~~ and possess undulating  
792 bedforms, ~~SW surface water~~ gets mixed with ~~groundwater-GW~~ throughout the local hyporheic  
793 exchange process. The mixing zone exhibits a typical crescent shape along the periphery of  
794 typical hyporheic exchange cells within a stationary streambed (as reported by Fox et al., 2014;  
795 Hester et al., 2019; Nogueira et al., 2022). The sizes of the ~~surface waterSW-groundwater-GW~~  
796 mixing zone (e.g., thickness and area) occupy a small proportion of the whole HZ. In the  
797 immobile bedform, the thin mixing zone ~~s (16~84% ranges)~~ occupying ~~~510%~~ is consistent  
798 with prior work (Hester et al., 2013; Santizo et al., 2020). However, at the onset of bedform  
799 migration, the mixing pattern, extent, and intensity of SW-GW interactions are altered. During  
800 the initiation of bedform migration, however, the mixing pattern, size, and intensity of surface  
801 water-groundwater interactions undergo modification. A continuous SW-GW mixing zone is  
802 formed within the ripples of the medium- to fast-moving bedform (Figure 64), and the area of  
803 mixing zone increases to approximately ~~10-15~1525%~~ at this time. Besides, the net flux of  
804 ~~surface waterSW~~ and ~~GWgroundwater~~ mixing ~~across the sediment-water interface (or within the~~  
805 ~~riverbed) is also increase~~sd with stream velocity and bedform migration celerity significantly  
806 (Figure 75). As a result, bedform migration controls and determines the hotspots and magnitudess  
807 of the SW and GW mixing. The bedforms are typically assumed to be immobile potentially  
808 making underestimations of ~~SWsurface water-GW~~ ~~-and groundwater~~ mixing flux and mixing  
809 zone in a HZ.

810 Instead of the typical crescent-shaped ~~mixing-dependent (MD)~~ denitrifying zone observed  
811 in stationary bedforms (Naranjo et al., 2015; Hester et al., 2014, 2019), the MD reaction  
812 zonation changes to the layer shape distributed at the fringe of the HEF cells, where mixing  
813 between SW and GW develop to a largest degree (Figure ~~\_65~~). Such a situation exists where the  
814 stream flows into with a relative homogeneous sandy riverbed with low autochthonous organic  
815 carbon content and encounters with nitrate enrich- GWgroundwater. The heterogeneous  
816 streambeds including buried autochthonous organic matter (Sawyer, 2015; Ping et al., 2022),  
817 deposited particulate organic particles (Drummond et al., 2017; Ping et al., 2023), and biological  
818 aggregate (Xian et al., 2022), would complicate the hyporheic exchange process, induce the  
819 rough and irregular shapes and boundaries of HEF cells and mixing zone, and therefore affect  
820 redox microenvironments and biogeochemical zonations. MD denitrifying hotspot would also  
821 ~~develop~~ develop around available sources of DOC.



822  
823 **Figure 102.** Variation of the dimensionless Damköhler number as a function of the  
824 dimensionless parameter  $U_r$ .

825 Previous studies have demonstrated that migrating bedforms constrains the penetration  
826 depths of stream-borne solute, shorten their transport timescale, and reduces the removal

827 efficiency of stream-borne nitrate (Jiang et al., 2022; Kessler et al., 2015; Ping et al., 2022;  
828 Zheng et al., 2019). Our findings indicate that bedform migration exerts more complex  
829 influences on MD denitrification and groundwater-borne nitrate removal. Bedform migration  
830 facilitates increases in the MD denitrification rate, driven by enhanced mixing flux, a higher  
831 mixing proportion, and an expanded mixing zone. At the same time, it shortens the residence  
832 timescale of water and solutes, which is detrimental to the occurrence of denitrification. When  
833  $Re < 4000$  (slow to moderate stream velocities), the MD denitrification rate of moving bedforms  
834 is greater than or comparable to that of stationary bedforms; in contrast, for fast-moving  
835 bedforms ( $Re \geq 4000$ ), the MD denitrification rate decreases significantly. This is likely because  
836 the reduction in MD rate driven by a shortened transport timescale dominates over the increase  
837 caused by enhanced mixing intensity; meanwhile, at this stage ( $\sim Re 4000$ ), the mixing fraction  
838 of the total exchange flux and the extent of the mixing zone both approach plateaus. Fast-  
839 moving bedforms also reduce the HZ attenuation capacity for groundwater-borne nitrate.  
840 Notably, the  $g\text{-NO}_3^-$  removal efficiency in HZs peaks for stationary bedforms and those at the  
841 onset of migration.

842 ~~Our results revealed that the bedform migration also reduces the HZ attenuation ability for~~  
843 ~~groundwater borne nitrate. The causes of fast-moving bedforms' negative impact on  $g\text{-NO}_3^-$~~   
844 ~~removal differ depending on the type of riverbed sediment. However, the reasons for this~~  
845 ~~phenomenon are different for various riverbed sediments.~~ For very fine sand ( $D_{50} = 0.08$  mm), a  
846 larger celerity results in a decline in transport timescale with less impact on removal efficiency  
847 as the system is rate-limited ( $Da \gg 1$ ). For fine to medium sand riverbed ( $D_{50} = 0.15\text{--}0.36$   
848 mm), the transport-limited situation leads to a low denitrification rate with increasing  $U_r$  but

849 constantly decreasing  $Da$  ( $Da < 1$ ). It is important to note that the calculated reaction timescale  
850 ~~we calculated~~ is based on the consumption period of oxygen to a prescribed anoxic threshold.  
851 Under these conditions, the exhaustion of labile DOC would also lead to the cessation of  
852 denitrification (Zarnetske et al., 2011a, 2011b). These results demonstrate that in order to  
853 evaluate the self-purification capacity of the HZ and its function as a natural barrier mitigating  
854 groundwater contamination, riverbed sediment transport dynamics and grain size distributions  
855 need to be considered. Stabilizing bedform configurations in restoration projects would enhance  
856 the natural attenuation capacity of HZs. Additionally, enhancing mixing intensity, along with  
857 extending the water residence timescale, would also facilitate the self-purification of  
858 HZs. ~~Stabilizing bedform configurations in restoration projects would enhance the natural~~  
859 ~~attenuation capacity of the HZ.~~

860 Different from previous studies that demonstrated the magnitude of NMD denitrification  
861 ~~is was~~ often greater than that of MD denitrification (Hester et al., 2014; Trauth and Fleckenstein,  
862 2017). The results in our study show that the total reaction rate of s- $\text{NO}_3^-$  ~~is was~~ smaller than that  
863 of g- $\text{NO}_3^-$  in mobile bedforms. This phenomenon can be attributed to the following two reasons:  
864 First, the concentration of s- $\text{NO}_3^-$  is one-third that of g- $\text{NO}_3^-$ ; Second, the reaction zone is  
865 reduced by migration celerity for NMD denitrification, while the reaction zone for MD  
866 denitrification is increased in the moving riverbed ~~at the onset of bedform migration~~. Mixing  
867 intensity increases with bedform migration, a process that facilitates MD denitrification more  
868 effectively ~~The mixing intensity increases with bedform migration, which facilitates the MD~~  
869 ~~denitrification more effectively~~ (Hester et al., 2019; Nogueira et al., 2024; Trauth and  
870 Fleckenstein., 2017). ~~Moreover, migration celerity increases the influx of nitrate into the HZ by~~

871 ~~enhanced hyporheic exchange, while it has less impact on upwelling GW flux. Hence, the~~  
872 ~~removal efficiency of g-NO<sub>3</sub><sup>-</sup> is also higher than that of s-NO<sub>3</sub><sup>-</sup> in these scenarios.~~ More  
873 attention should be paid to the mixing dynamics and mixing triggered biogeochemical reactions,  
874 which is helpful to put forward appropriate stream restoration plans so as to enhance the health  
875 of the aquatic ecosystem (Hester et al., 2017; Lawrence et al., 2013).

876 The findings of this study, derived from 2D numerical models, are most directly  
877 generalizable to straight, low-curvature streams with periodic bedform distributions in riverbed  
878 sediments. In such stream systems, hyporheic exchange is dominated by streamwise-vertical  
879 flow cells, with lateral (cross-stream) hyporheic flux accounting for a small fraction of total  
880 exchange (Hu et al., 2014; Naranjo et al., 2015). Importantly, straight, low-curvature streams are  
881 prevalent in agricultural and urban downstream gaining reaches, which are characterized by  
882 relatively homogeneous sediment types (e.g., fine sands). This is consistent with the focus of our  
883 study, as these reaches often face nitrate pollution challenges and depend on natural attenuation  
884 to maintain water quality (Hester et al., 2014; Trauth et al., 2017). In practice, riverbeds may  
885 exhibit far more complex three-dimensional (3D) bedform morphologies coupled with sediment  
886 heterogeneity, for example, in highly meandering streams (Gomez-Velez et al., 2017; Nogueira  
887 et al., 2024; Pescimoro et al., 2019). Future studies should establish 3D numerical model and  
888 incorporate stochastic hydraulic conductivity ( $K$ ) fields to explore how sediment heterogeneity  
889 interacts with bedform migration, evaluating whether high- $K$  hotspots enhance or reduce  
890 migration-driven mixing.

891 A critical limitation of this study is the reliance on empirical equations for bedform  
892 migration celerity. The Chezy equation (suited for flat, homogeneous sediment beds) was used

893 to estimate the shear velocity for incipient sediment motion, with the experimentally derived  
894 empirical relationship by Coleman and Melville (1994) further adopted to account for the  
895 additional shear stress induced by ripples. The two-step approach (shear stress for particle  
896 initiation and ripple-scale form drag via empirical formulations) reproduced Wolke's (2020)  
897 results and proved useful for investigating the effects of bedform migration on hyporheic solute  
898 transport (Ahmerkamp et al., 2017; Zheng et al., 2019). We did not simulate turbulent flow over  
899 triangular bedforms nor compute shear stress via computational fluid dynamics (CFD; a  
900 numerical method that would yield more precise pressure profiles and shear stress distributions)  
901 given that the core focus of this study was to investigate how bedform migration affects  
902 hyporheic solute transport and mixing-dependent denitrification within the riverbed layer. As  
903 future work, the empirical approach should be validated using CFD simulations to enhance the  
904 accuracy of bedform-related stress and migration calculations. This validation will facilitate the  
905 quantification of potential uncertainties and refine the mechanistic understanding of bedform  
906 dynamic-hyporheic zone interactions.

907 This study focuses on ripples and, more broadly, shorter-wavelength topographic roughness  
908 elements formed under low subcritical flow conditions in sandy riverbeds (Ashley, 1990;  
909 Gomez-Velez et al., 2015; Raudkivi, 1997). The undulating bedforms maintain dynamic  
910 equilibrium through geometric adjustments, with their geometry remaining unchanged as the  
911 stream velocity fluctuates within a specific range (10–30 cm/s). When stream velocities exceed  
912 this upper threshold, a condition commonly observed in fast-flowing rivers, bedform geometries  
913 can be altered, ultimately leading to bedform erosion (Boano et al., 2013; Harvey et al., 2012).  
914 This process is not accounted for in the current model. If small-scale ripples develop and merge

915 with larger-scale ripples and dunes under high stream velocities, the removal efficiency of s-  
916 NO<sub>3</sub><sup>-</sup> and g-NO<sub>3</sub><sup>-</sup> may be enhanced due to the extended hyporheic flow paths and increased  
917 residence timescales (Harvey et al., 2012; Zomer and Hoitink, 2024). Otherwise, the removal of  
918 s-NO<sub>3</sub><sup>-</sup> would be highly hindered because of shorter residence time and fully oxic condition in  
919 fast moving bedforms and fast flowing rivers, while the removal of g-NO<sub>3</sub><sup>-</sup> would likely be less  
920 affected within the immobile streambeds.

921 The numerical models assume isotropic sediment  $K$ . In fact, natural riverbeds with small-  
922 scale bedforms (e.g., dunes and ripples) commonly exhibit strong anisotropy. For bedforms with  
923 flow-transverse crests, pore pathways parallel to the crests are relatively continuous and straight  
924 yielding high transverse  $K$ . In contrast, pathways perpendicular to the crests are highly tortuous:  
925 water here must move upward through erosional, high-porosity troughs and downward over  
926 depositional, low-porosity crests. This undulating flow path, combined with flow separation and  
927 local energy losses at morphological transitions, substantially increases hydraulic resistance and  
928 lowers longitudinal  $K$  (Dallmann et al., 2020; Salehin et al., 2004). Additionally, spatial  
929 variability in bedform dimensions (e.g., increasing dune wavelength downstream) enhances this  
930 anisotropy by introducing zones of differing flow resistance along the streamwise direction  
931 (Venditti et al., 2005). Given that the model is constructed along a longitudinal section with  
932 spatially consistent, periodic bedforms, the use of an isotropic hydraulic conductivity  
933 assumption is justified. Incorporating anisotropic  $K$  is critical for accurately modeling hyporheic  
934 exchange and related biogeochemical processes in 3D heterogeneous models.

935 In this study, we focused on ripples and, more broadly, shorter wavelength topographic  
936 roughness elements that form under low subcritical flow conditions in sandy riverbeds (Ashley,

937 ~~1990; Gomez Velez et al., 2015; Raudkivi, 1997). The undulating bedforms maintain dynamic~~  
938 ~~equilibrium through geometric adjustments, with their geometry remaining unchanged as the~~  
939 ~~stream velocity fluctuates within a specific range (10–30 cm/s). When stream velocities exceed~~  
940 ~~this upper threshold, a condition commonly observed in fast-flowing rivers, bedform geometries~~  
941 ~~can be altered, ultimately leading to bedform erosion (Boano et al., 2013; Harvey et al., 2012).~~  
942 ~~This process is not accounted for in the current model. If small-scale ripples develop and merge~~  
943 ~~with larger-scale ripples and dunes under moderate stream velocities, the removal efficiency of~~  
944 ~~s-NO<sub>3</sub><sup>-</sup> and g-NO<sub>3</sub><sup>-</sup> may be enhanced due to the extended hyporheic flow paths and increased~~  
945 ~~residence timescales (Harvey et al., 2012; Zomer and Hoitink, 2024). Otherwise, the removal of~~  
946 ~~s-NO<sub>3</sub><sup>-</sup> would be highly hindered because of shorter residence time and fully oxic condition in~~  
947 ~~fast moving bedforms and fast flowing rivers, while the removal of g-NO<sub>3</sub><sup>-</sup> would likely be less~~  
948 ~~affected within the immobile streambeds.~~

949 ~~In our model, dissimilatory nitrate reduction to ammonium (DNRA) was not incorporated~~  
950 ~~in the model~~, given that denitrification is typically regarded as the predominant pathway for  
951 nitrate removal, whereas DNRA plays a secondary role in nitrate transformation (Zarnetske et  
952 al., 2012). Lansdown et al. (2012) and Quick et al. (2016) have demonstrated that approximately  
953 5% of <sup>15</sup>NO<sub>3</sub><sup>-</sup> tracer in river sediment incubations underwent DNRA, while 85% underwent  
954 denitrification. Nevertheless, DNRA competes with denitrification for NO<sub>3</sub><sup>-</sup> and DOC as  
955 electron acceptor and donors within HZs. When an oligotrophic and/or a pristine stream  
956 infiltrate into the streambed and subsequently interact and mix with nitrate-enriched ~~—~~  
957 ~~GW groundwater~~, the MD DNRA would not occur due to the low C/N ratio. ~~MD denitrification—~~  
958 ~~zone shifts upstream toward the overlying water column, leading to a pronounced spatial—~~

959 ~~mismatch between the denitrification zone and the mixing interface. This occurs because DOC~~  
960 ~~is intensively consumed within the HEF cell, such that elevated MD denitrification rates emerge~~  
961 ~~below the DOC and oxygen plumes yet above the mixing zone. Specifically, this phenomenon~~  
962 ~~can be attributed to the critical role of dispersion effect in solute transport and mixing-triggered~~  
963 ~~denitrification, besides advection effect.~~ When ~~ana~~ eutrophication stream with higher DOC  
964 concentration, DNRA would have a greater influence in nitrate transformation because DNRA is  
965 prone to occur in NO<sub>3</sub><sup>-</sup>-limited (that is DOC sufficient) conditions compared to denitrification  
966 (Zhu et al., 2023). The ammonia produced by ~~MD mixing-dependent~~ DNRA would be further  
967 nitrified within the aerobic HEF cell, thereby potentially elevating the risk of nitrate pollution in  
968 ~~SW surface water.~~

969 In ~~this our~~ model, stream velocity and upward ~~GW groundwater~~ flux are considered  
970 constant in the present model, yet they may change in time due to storm events, tidal pumping,  
971 snowmelt, or reservoir hydro-peaking (Liu et al., 2024; Nogueira et al., 2022; Song et al., 2018).  
972 Hester et al. (2019) demonstrated that increasing surface water stage would enhance both NMD  
973 and MD denitrification. Nogueira et al. (2024) and Trauth and Fleckenstein (2017) pointed out  
974 that ~~GW groundwater~~ discharge events increase the magnitude of ~~SW surface water~~  
975 ~~GW groundwater~~ mixing, therefore effecting the prevalence of MD denitrification. The  
976 interactions among morphological dynamics, hyporheic exchange, and biogeochemical  
977 processes under transient conditions are key areas for future research.

## 978 5. Conclusion

979 The numerical model developed in this study was ~~used~~applied to simulate the interaction  
980 and mixing of upwelling groundwater with bedform-induced hyporheic flow, examining how  
981 bedform migration influences ~~surface water~~SW and ~~groundwater~~GW mixing and the processing  
982 of groundwater-borne nitrate within the HZ. Our analysis quantified the mixing flux and the size  
983 of mixing zone, as well as the mixing-dependent denitrification rates and removal efficiencies  
984 across riverbed sediments characterized by varying grain sizes, stream flow velocities, and  
985 groundwater discharge fluxes. These model simulations revealed~~ed~~ed that as bedforms migrate, the  
986 ~~SW~~surface water-~~GW~~groundwater mixing zone and the associated mixing-dependent  
987 denitrification zone progressively evolve into uniform, band-like structures. When turnover  
988 dominates the hyporheic exchange process, the mixing flux increases significantly (an increase  
989 of one order of magnitude). The mixing proportion of total exchange flux and mixing zone  
990 scope also experience significant increases at the onset of migration; as stream velocity further  
991 rises, the mixing proportion (~60%) and mixing size (~10–15% of the riverbed) gradually reach  
992 plateaus. Enhanced SW-GW mixing dynamics facilitates MD denitrification as bedforms begins  
993 migration; however, shortened solute residence timescales resulting from fast bedform  
994 movement limits MD denitrification occurrence.~~For riverbeds composed of fine to medium sand~~  
995 ~~( $D_{50} = 0.15 - 0.36$  mm), both the magnitudes of SW and GW exchange flux and mixing flux~~  
996 ~~increase significantly when turnover becomes the dominant exchange mechanism, while the~~  
997 ~~proportion of mixing flux across the sediment-water interface and the size of mixing zone~~  
998 ~~remain approximately constant at this time. Meanwhile, both the mixing-dependent~~  
999 ~~denitrification rates and removal efficiencies decline significantly with increasing stream flow~~

~~velocities and associated bedform migration rates.~~ Under dynamic bedform conditions, the self-purification capacity of the HZ is reduced for fine to medium sand ( $D_{50} = 0.15-0.36$  mm), compromising its role as a natural barrier against groundwater contamination. Incorporating the identified and analyzed factors can enhance the management of riverbed sediment-associated aquatic systems, particularly for management objectives focused on removing groundwater-borne nitrate. ~~The management of aquatic systems involving riverbed sediments can be enhanced by incorporating the analyzed factors identified here, particularly when management goals encompass the removal of groundwater borne nitrate.~~

#### **Notation**

$S$	Stream slope [-]
$H$	Water depth [L]
$U$	Stream velocity [ $L T^{-1}$ ]
$\lambda$	Ripple wavelength [L]
$u_e$	Bedform migration celerity [ $L T^{-1}$ ]
$l$	Streambed height [L]
$H_d$	Ripple height [L]
$D_{50}$	Median sediment size [L]
$x$	Horizontal coordinate, rightward positive [-]
$y$	Vertical coordinate, upward positive [-]
$k$	Sediment permeability [ $L^2$ ]

$K$	Sediment hydraulic conductivity [ $L T^{-1}$ ]
$h$	Hydraulic head [ $L$ ]
$h_m$	Amplitude of the sinusoidal head variation [ $L$ ]
$m$	Wavenumber of the variation [-]
$g$	Gravity acceleration [ $L T^{-2}$ ]
$c_i$	Concentration of reactive components [ $M L^{-3}$ ]
$c_{gw}$	Groundwater tracer [ $M L^{-3}$ ]
$v$	Seepage velocity [ $L T^{-1}$ ]
$\theta$	Sediment porosity [-]
$\alpha_L$	Longitudinal dispersivities
$\alpha_T$	Transverse dispersivities [ $L$ ]
$D_{ij}$	Hydrodynamic dispersion [ $L^2 T^{-1}$ ]
$D_m$	Molecular diffusion coefficient [ $L^2 T^{-1}$ ]
$\tau$	Tortuosity factor [-]
$V_{AR}$	Maximum reaction rate of aerobic respiration [ $T^{-1}$ ]
$V_{DN}$	Maximum reaction rate of denitrification [ $T^{-1}$ ]
$K_{inh}$	Non-competitive inhibition factor [ $M L^{-3}$ ]
$K_{DOC}$	Half-saturation for dissolved organic carbon [ $M L^{-3}$ ]
$K_{NO_3^-}$	Half-saturation for nitrate [ $M L^{-3}$ ]
$K_{O_2}$	Half-saturation for oxygen [ $M L^{-3}$ ]
$X_{AR}$	Microbial concentration facilitating aerobic respiration [ $M L^{-3}$ ]
$X_{DN}$	Microbial concentration facilitating denitrification [ $M L^{-3}$ ]

$\rho_s$	Sediment density $[M L^{-3}]$
$\rho$	Water density $[M L^{-3}]$
$P$	Hydraulic pressure $[M L^{-1} T^{-2}]$
$u^*$	Bed shear velocity $[L T^{-1}]$
$u_{cr}^*$	Critical bed shear velocity $[L T^{-1}]$
$n$	Manning coefficient $[-]$
$\tau^*$	Shield parameter $[-]$
$\tau_{cr}$	Critical shear stress $[M L^{-1} T^{-2}]$
$\tau_{cr}^*$	Critical Shield parameter $[-]$
$r$	Submerged specific gravity of sediment $[-]$
$R_{O_2}$	Aerobic respiration rate $[M L^{-3} T^{-1}]$
$R_{s-NO_3}$	Non-mixing dependent denitrification rate $[M L^{-3} T^{-1}]$
$R_{g-NO_3}$	Mixing dependent denitrification rate $[M L^{-3} T^{-1}]$
$R_{DOC}$	Dissolved oxygen carbon consumption rate $[M L^{-3} T^{-1}]$
$\nu$	Kinematic viscosity of water $[L^2 T^{-1}]$
$u_s$	Underflow seepage velocity induced by stream gradient $[L T^{-1}]$
$Re$	Reynolds number $[-]$
$U_r$	ratio of bedform celerity to pore water velocity $[-]$
$u_p$	Darcy velocity induced by pumping process $[L T^{-1}]$
$u_q$	Vertical groundwater flux $[L T^{-1}]$
$U_b$	Ratio of vertical groundwater flux to hyporheic exchange flux
$\tau_R$	Biogeochemical reaction timescale $[T]$

$\tau_T$	Water transport timescale [T]
$Da$	Damköhler number [-]
$\Delta x$	Bedform migrating displacement per timestep (L)
$dt$	The length of per timestep (T)
$Da'$	Conversion factor for unit Darcy to $m^2$ [-]
$F_{mix}$	the proportion of mixing flux to hyporheic exchange flux [-]
$A_{mix}$	Surface water and groundwater mixing zone [ $L^2$ ]
$A$	Streambed area [ $L^2$ ]
$M_{NDN}$	Nitrate removed by non-mixing dependent denitrification [M]
$M_{DN}$	Nitrate removed by mixing dependent denitrification [M]
$N_{RE}$	Nitrate removal efficiency [-]
$M_{in}$	Nitrate being introduced into streambed [M]

#### Abbreviations

HZ	Hyporheic zone
SW	Surface water
GW	Groundwater
HEF	Hyporheic exchange flow
MF	Mixing flux
NMD	Non-mixing dependent
MD	Mixing dependent
AR	Aerobic respiration
DN	Denitrification

1010 **Supporting information**

1011 Additional details of the model scenarios and model validation were displayed in the  
1012 supporting information.

1013 **Data availability**

1014 All raw data can be provided by the first author upon request.

1015 **Competing interest**

1016 The authors declare that they have no conflict of interest.

1017 **Author contribution**

1018 Conceptualization: XP, YX

1019 Formal analysis: XP

1020 Funding acquisition: XP, ZW, YX, SK

1021 Investigation: XP, ZW

1022 Methodology: XP, ZW, YX

1023 Writing-original draft: XP

1024 Writing-review and editing: ZW, YX, MJ, SK

1025 Project administration: ZW

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