

Hydrodynamic and morphodynamic patterns on a mid-channel intertidal bar in an estuary

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Abstract. Many estuaries have mid-channel bars that emerge during low tide. In contrast to fringing tidal flats, these bars are fully surrounded by channels. This may lead to different hydrodynamics and sediment dynamics, but how is unclear, and whether simple models for the dynamics of fringing flats are applicable to mid-channel bars is likewise unknown. These insights are needed for large-scale morphological prediction of the effects of human interference and sea level rise. Using [a unique dataset consisting of](#) six months of current velocity data at 16 locations on a mid-channel bar, we analysed the hydrodynamics and calculated a proxy for sediment transport. [Taking advantage of the large number of measurement locations, detailed flow patterns were derived.](#) The data show that the spatially non-uniform morphology influences the hydrodynamics and sediment transport in multiple ways. At the deeper parts of the mid-channel bar the ebb- or flood-dominance is determined by the surrounding ebb- and flood channels. This causes changes in ebb or flood dominance over short distances. On the tidal flat, the presence of a higher area on the seaward bar head causes large-scale circulation cells. The flow bends around the emerged areas and causes larger cross-shore flows than expected for alongshore uniform tidal flats. These large-scale flow patterns also determine sediment transport patterns and possibly explain how the mid-channel bar can increase in height. Although the measured data show that velocity can veer over depth, this does not result in large changes in sediment transport direction, suggesting that depth-averaged models are reasonable approximations for predicting sediment transport on and morphological change of intertidal areas even on mid-channel bars.

1 Introduction

Intertidal areas in estuaries and coastal lagoons are an essential habitat for many benthic and bird species and provide ecosystem services such as coastal protection, wildlife conservation, economic activities, and other social benefits (Costanza et al., 1997; Dyer and Huntley, 1999; Pinto et al., 2010; Barbier et al., 2011; Coco et al., 2013; Plag and Jules-Plag, 2013). However, human use, species invasion, sea level rise, and climate change pose threats to the long-term survival of these areas and their ecosystem services (Kennish, 2002; Leuven and Kleinhans, 2019; de Vet et al., 2020; van Dijk et al., 2021; Xie et al., 2022). The morphology of intertidal areas plays a crucial role in sustaining its ecosystem functioning, as it determines, along with the imposed variations in water level due to tides and storms, the exposure time and the total wetted area. To preserve the ecosystem services of intertidal areas, their morphology needs to change with changes in local sea level conditions. For the

25 prediction of these morphological changes, we need to have knowledge of the sediment transport patterns in intertidal areas in
relation to the hydrodynamic conditions ~~and~~ (tidal range, current flow, and wave exposure), sediment composition, and human
activities. Additionally, the question is whether frequently used 1D or 2D models are sufficient to predict these changes.

Intertidal areas with different hydrodynamic settings can be found in a wide range of environments. For sediment transport
patterns, the main directions of tidal flow and wave exposure are important (Dyer, 1989; van Rijn, 1993). On the landward side
30 of the tidal basins and near barrier islands, we find intertidal areas where the tide-induced flow velocities are mainly across
the tidal flats. The shape and morphological evolution of these systems, also known as open tidal flats, are mainly determined
by a balance between wave-driven processes that tend to erode the tidal flats and tides that bring sediment on them (Hir et al.,
2000; Roberts et al., 2000; Friedrichs, 2009). One-dimensional (1D) models and concepts have been used to understand how
these flats develop as a function of changes in external forcing, sediment supply (Bearman et al., 2010), and direct human
35 interventions such as building dikes and land reclamation (Gregory Hood, 2004). The main trends in flow, sediment transport
and morphology are in the cross-flat direction, and therefore the 1D-assumption that the tidal flat is uniform in the alongshore
direction is often a reasonable approximation.

In many estuaries we also find fringing tidal flats. These are located between a tidal channel and the bank of the estuary.
Here, the alongshore tidal flows are much larger (Hanssen et al., 2024) and the morphological evolution is determined by the
40 alongshore and cross-shore tidal flows, wind-driven flows, and waves. For these systems, the tidal flats are often assumed to be
alongshore uniform, resulting in the use of 1D models and concepts (Waeles et al., 2004; Maan et al., 2015; Zhou et al., 2016).
As the estuary width often fluctuates on top of a landward decreasing trend, the width and therefore the area of the fringing
tidal flats are not constant (Leuven et al., 2017). Hence, alongshore variability in the morphology of fringing tidal flats can be
expected, but how this influences the local alongshore and cross-shore tidal flows and the morphological evolution has not yet
45 been studied in detail.

A third type of intertidal area is found at mid-channel bars, typically situated in estuaries, but also in coastal lagoons. These
mid-channel bars differ from fringing tidal flats in the sense that they are bounded by two channels instead of one. These
channels form ebb and flood dominant channels (van Veen, 1950; Jeuken, 2000; Leuven et al., 2016), in which one of the
channels may have net sediment transport in the ebb direction and the other in the flood direction, often resulting in a tendency
50 for sediment to recirculate around a mid-channel bar. Furthermore, the mid-channel bars have a finite length and are therefore
by definition not alongshore uniform. The flow must curve around the heads of the bar, which may cause upslope veering of
the flow towards what is effectively an inner bend of the channel around the bar head. Such secondary flows may drive upslope
sediment transport that is absent on fringing tidal flats. Another difference from fringing tidal flats is that the mid-channel bar is
flooded and drained from multiple directions. Once fully submerged, tidal differences between the channels may affect large-
55 scale flow patterns. These differences with fringing tidal flats are likely to have implications for the hydrodynamics, sediment
transport, and morphological evolution of a mid-channel bar.

While much understanding has been gained about flow and sediment transport on open and fringing tidal flats ~~in estuaries,~~
e.g. the Jade, and Weser estuary (Gelfort et al., 2011; Benninghoff and Winter, 2018; Lefebvre et al., 2021), San Pablo Bay
(Allen et al., 2019), the Yangtze (Wan et al., 2014; Zhu et al., 2017), the Gironde estuary (Billy et al., 2012), the Seine estuary

60 [\(Grasso and Le Hir, 2018\)](#), [the Belon estuary \(Michel et al., 2021\)](#), and [Oosterschelde \(de Vet et al., 2020\)](#), three-dimensional (3D) flow patterns on mid-channel bars are neither well known nor quantified. Measurements are sparse and costly, and morphodynamic models often use depth-averaged flow velocities (either 1D or 2DH) for reasons of computational efficiency to calculate sediment transport and morphological change, even though flow patterns may in fact be 3D. As a result, the patterns developing in 2D morphological models depend strongly on sediment transport parametrization for veering of the flow and the bed shear stress vector, and for slope effects on sediment transport that need to be calibrated to obtain realistic channels and bars (Baar et al., 2019). Whether this 2D approach is a sufficiently valid approach for these mid-channel bars is unclear.

This study aims to increase our understanding of the hydro- and morphodynamics and their impact on the sediment transport on mid-channel bars. Moreover, we will compare the results for mid-channel bars conceptually with those of open and fringing tidal flats to gain insight into the differences between these three types of intertidal areas. For this, we ~~measured~~[analysed](#) the current velocities on a spatial grid at 16 locations for a period of 6 months on a mid-channel tidal bar in the Western Scheldt estuary in the Netherlands. The results are compared to simple models developed for open and fringing flats, which serve as a starting point for understanding differences with mid-channel bars. [From this comparison we will infer whether the use of 1D and 2D models is sufficient to predict morphological changes of an intertidal bar, or that more sophisticated 3D models are needed.](#) The measured cross-shore and alongshore velocities were used as input into a simple sediment transport predictor to understand how the hydrodynamic forcing and tidal flat morphology determine the sediment balance of a mid-channel tidal bar. [Although flow patterns on intertidal shoals have numerically been studied \(de Vet et al., 2018; Elmilady et al., 2019, 2020\), the specific role of the non-uniform bathymetry was not elaborated in these studies. As the field site is macro-tidal and limited in fetch we assumed that the effect of waves on the large-scale sediment transport patterns is small in comparison with that of currents. Therefore, the effect of waves on the sediment transport was omitted in this study.](#)

80 2 Methods

2.1 Field site

The Western Scheldt, which is a part of the Scheldt estuary that is located in Belgium and the Netherlands (Meire et al., 2005), is a funnel shaped estuary and has a complex geometry with mid-channel bars. It is characterized by a dual channel system of evasive ebb and flood channels, fringing tidal flats, submerged mid-channel bars such as the Spijkerplaat, mid-channel tidal bars such as 'Hooge ~~Plaaten~~[Platen](#)', sills, and short-cut channels that connect an ebb dominant channel with a flood dominated channel, see Fig. 1(a). Where ebb and flood channels meet, sills are present between the channel heads (van Veen, 1950). The ebb dominated channel is used as the main shipping channel to the ports of Flushing, Terneuzen, Ghent and Antwerp and one of the main human interventions in the Western Scheldt is deepening and maintenance of the main shipping channel to grant access to these ports. Therefore, approximately 10.5 Mm³ sediment is dredged annually from the sills and deposited in designated areas in the Western Scheldt ~~.(van Dijk et al., 2021).~~

The field site is an intertidal flat, located in the northern part of the mid-channel bar 'Hooge Platen in the Western Scheldt. A measurement campaign was carried out from February 2019 to July 2019. The field site has a semi diurnal tide with an average

tidal range of 3.83 m, 4.45 m at spring tide and 2.99 m at neap tide (Rijkswaterstaat, 2018). The dominant wind direction in the area is southwest to west. ~~This~~ As is depicted in Fig. 1(a) this mid-channel tidal bar is surrounded by multiple channels: in the south ~~the ebb dominant channel 'Vaarwater langs Hoofdplaat', east the ebb dominant channel 'Pas van Borssele', west the flood dominant channel 'Wielingen', and north the flood dominant channel 'Schaar van Spijkerplaat'. Located south of the sill in the 'Schaar van Spijkerplaat', and attached and east ebb dominant channels, and in the west and north flood dominant channels.~~ Attached to the field site in the northeast is a spit that separates the ~~'Schaar van Spijkerplaat' flow~~ from the adjacent channel in the eastern part of the field site. The mid-channel bar is fully submerged during high tide, except for the area in the southwest ~~named 'De Bol',~~ which is submerged only when the water level is above 3 m NAP (NAP = Dutch ordnance datum, approximately mean sea level). An elevation map of 'Hooge ~~Plaaten~~ Platen' is shown in Fig. 1(b). The higher parts, i.e. the southern rim and 'de Bol' are overgrown. At the other areas the bed composition of the top layer consists of a mixture of sand and silt. In general, the bed is smooth, with local small ripples (1-2 cm). Several smaller creeks run from higher elevated areas towards the channel. Halfway the field site a large creek is present with a width of several meters and depth up to 1.5 m. The field site consists of a mixture of sand and finer sediments. The sediment characteristics in the top layer varies from $D_{50} = 95 \mu\text{m}$ in the south-east, which is also higher up the mid-channel bar, to $D_{50} = 196 \mu\text{m}$ in the north-west, the lowest parts of the bar. The percentage of the sediment fraction with a size smaller than $63 \mu\text{m}$ varies from 22.8% in the south-east to 1.4% in the north-west. In Fig. 1(c) the difference in bed level between 2009 and 2019 is shown. The figure shows that the mid-channel tidal bar is locally elevating up to 10 - 15 cm per year which is much faster than the local sea level rise of 2 mm per year. ~~This rapid elevation decreases the intertidal area and increases~~ Peak flow velocities decrease as the bed level becomes higher. As a result, finer sediments will settle easier, thereby increasing the percentage of silt in ~~the bed~~ these areas.

2.2 Data gathering

A detailed overview of the field site including the position of the transects and the 16 sensors is shown in Fig. 2. At transects T1 to T4 velocity profiles were measured at four locations per transect: one subtidal location in the channel and three on the intertidal flat (-1 m NAP, 0 m NAP and +1 M NAP). In this paper, each measurement location is referenced by the one-digit transect number of its alongshore location from west to east, followed by the one-digit number of its cross-shore position on the transect: 1 is deepest, 4 is most elevated.

Velocity profiles in the channel were recorded during the period of 15 February to 5 July 5 2019 using upward looking Teledyne RD Instruments Workhorse Monitor 1200 kHz mounted at a height of 0.6 m in a frame that was placed at the sea bed. Each instrument was configured with a cell size of 0.5 m and a blanking distance of 1.04 m. Every 10 minutes the average of the preceding ten minutes data were stored. Bottom depth at the locations of the Teledyne RD Instruments Workhorse Monitor 1200 kHz was obtained through the sonar sensor mounted on the vessel during placement of the instruments. Note that location '41' had a less clear signal because it was positioned at a more shallow site. Combined with the ADCP's configured cell size and blanking distance the flow velocity during low water levels could not always be determined accurately. The velocity profiles on the mid-channel tidal bar were recorded during two periods: the first period from 15 February to 12 April 2019, after which the batteries were renewed, and the second period from 24 April to 5 July 2019. All sensors at the intertidal flat were upward

looking Nortek Aquadopp Profiler 2 MHz sensors, mounted in the bed and equipped with an external battery pack to extend the measurement duration. Each instrument was configured with a cell size of 0.1 m and a blanking distance of 0.1 m. Data were recorded during an averaging interval of 6 minutes and stored at an interval of 10 minutes.

130 During each field visit the height of each sensor head as well as the bed elevation of the transect was recorded with a Leica GPS1200+ system (Leica Geosystems, Switzerland) with Netherlands Positioning Service (NETPOS) as reference network. These transects are plotted to visualize small-scale morphological changes during the measurement campaign. Furthermore, three sediment samples of the top layer (5 cm) at random locations around the sensor were taken. These samples were mixed into one container and frozen to be analysed in the lab. For the overview of the intertidal bar morphology we used LiDAR bed
135 level data recorded at 5, 7, and 10 April, 2019 and a multibeam dataset for the area below -2 m NAP, recorded at April 18, 2019.

Wave data at the location WIEL, available every 30 minutes, and water level data at the locations VLIS, BRES and TERN, available every 10 minutes) were obtained from the measurement network of Rijkswaterstaat. Wind and pressure data at the location VLIS, available every 10 minutes, were obtained from the Royal Dutch Meteorological Institute. See Fig. 1(a) for the
140 position of these locations and the section 'Data availability' for the web addresses.

2.3 Data filtering, reduction and analyses

Current velocity data were validated in ~~two~~three steps. First, the local water level at the sensor was used to remove all data from the cells that were not fully submerged. Second, all data from the sensor starting from 1 measurement cell below the surface to the highest measurement cell was also removed. Third, all data were checked for consistency in flow magnitude and flow direction by using a threshold value of 90 counts or higher for the average amplitude signal of the different beams of the sensor. Last, all remaining data were visually checked for irregularities and discontinuities, due to e.g. turbulence or contamination. This data was manually removed. The local water level used, was obtained by interpolating the M2-tide parameters phase, amplitude, and mean sea level of the locations 'VLIS' and 'TERN' (Society, 1987). ~~Second, all data were manually checked for consistency in flow magnitude and flow direction. From the validated flow velocity data we retrieved the near-bed (first bin in the vertical), near-surface (uppermost bin fully below the surface) and depth-averaged velocities. The latter was obtained by averaging over the bins from near-bed to near-surface on the condition that more than half of the number of bins contained valid data.~~
145
150

To gain insight into the distribution of the current velocities during a full tidal cycle, the following steps were made. First, the measurements near the bed, near the surface and averaged over depth were referenced to the time of local high water in
155 classes of 1 minute, resulting in 721 classes. Second, classes with a number of samples less than a predefined threshold were removed. Third, within a moving window of 30 minutes, the 50% and 95% values of the current magnitude and the circular mean (Fisher, 1993) of the current direction were calculated. By this data reduction all tidal cycles can be compared and statistics can be determined. The resulting series used for the analysis of flow velocity variation in the profiles for the 50% and 95% near-bed (NB), depth-averaged (DA) and near-surface (NS) velocities are referenced to as NB₅₀, NB₉₅, DA₅₀, DA₉₅,
160 NS₅₀, NS₉₅. The circular mean of the direction data is referred to as D_c . To discriminate between tidal and sub-tidal variations

in water levels or flow velocities (only for data in channel), a Godin filter was applied on the time series (Foreman and Henry (1989)).

2.4 Sediment transport

To understand intertidal bar development, the gradients in sediment transport vectors are important. A proxy for the cumulative sediment transport at each location was calculated based on either the near-bed or the depth-averaged velocities. Sediment transport predictors for sandy suspended sediment typically have a dependence on the local flow velocity to the power 3 to 5. We left out the critical shear stress in the proxy, since this does not alter the general pattern of sediment transport, but only amplifies the flood dominant character of the system. Apart from a scaling factor α that depends on the sediment size, among other things, the heuristic behaviour of the sediment transport rate is $Q_s = \alpha |U|^2 U \text{ ms}^{-1}$. We calculated this quantity for each location in along-shore (u) direction and cross-shore (v) direction and summed over i to obtain a proxy for the cumulative sediment transport as

$$Q_u = \frac{\sum_{i=1}^N \mathbf{u}_i (u_i^2 + v_i^2)}{N} \quad (1)$$

$$Q_v = \frac{\sum_{i=1}^N \mathbf{v}_i (u_i^2 + v_i^2)}{N} \quad (2)$$

The total number of samples N for the locations will be different, due to the fact that not all locations are fully submerged during a tidal cycle. Also some locations were buried during one or more periods due to sediment displacement. Sediment transport was calculated with the near-bed as well as the depth-averaged current velocity, in a rotated Cartesian reference frame: the u-direction is taken in the large-scale alongshore direction of the tidal flat, 115° with respect to the North (clockwise, positive towards the East). The cross-shore v-direction is oriented at 25° with respect to the North.

3 Results

3.1 Hydrodynamic boundary conditions

Figure 3 displays the time series of the hydro-meteorological conditions. During the measurement campaign several neap-spring tidal cycles with calm weather and storm conditions were captured. The water level at the location 'VLIS' varied between a maximum of 2.93 m NAP and a minimum of -2.73 m NAP, and the average wind speed was 6.2 m/s coming from WSW direction. The first three weeks of March were characterized by stormy weather. During this period western wind speeds up to 25 ms^{-1} (9 Beaufort) were registered, the peak significant wave height at the location 'WIEL' was up to 2.5 m. The setup of the water level during this period had peak values of 0.6 m at the field site. At 27 April and 7-8 June, wind speeds were over 13.5 ms^{-1} (7 Beaufort), coming from a SW to W direction.

3.1.1 Large-scale flow patterns

In this paragraph we analyse the large-scale flow pattern. This pattern is calculated with the use of the peak flow values during
190 flood and ebb, averaged over the measurement period. Local differences in the flow are obtained from the depth-averaged time series DA₅₀, DA₉₅ and circular mean direction of each location. The local observations are analysed followed by a discussion of the large-scale flow pattern emerging from all stations.

The flow during average conditions, represented by the DA₅₀ time series of each location is displayed in Fig. 4. In general, the magnitudes are largest in the channel and decrease higher up on the intertidal flat. The magnitudes at the locations of transect
195 T1 on the intertidal flat are noticeably smaller than their counterparts on transects T2, T3 and T4. Peak flood flow ~~typically occurs~~ is typically reached 50-60 minutes before high tide ~~-, with the exception of~~ which is in agreement with the peak flow in the alongshore direction. The exceptions are locations '14' and '24'. At these two locations a clear peak during flood is missing: ~~the maximum flow occurs~~ flow is maximum at the moment the sensor is ~~being flooded and decreases afterwards.~~ flooded and subsequently decreases. At both locations the cross-shore component has a considerable effect on the current magnitude. Slack
200 water occurs for most locations up to 45 minutes after high tide. Exceptions are the locations on the mid-channel bar of transect T1 ('12', '13', '14') and the locations of transect T2 at higher altitude ('23', '24') where slack water occurs before high tide. The peak ebb flow occurs 1.5 to 2 hours after high tide. Noticeable are the differences between locations in the channel: the timing of the peak ebb flow at the locations '31' and '41' occurs more than 1 hour later and the magnitude is much larger than at the locations '11' and '12'. Locations higher up on the intertidal flat have their peak ebb flow earlier than the locations in
205 the channel.

The flow direction is shown in Fig. 5. In the channel the flow is bidirectional and the transition from ebb to flood and vice versa occurs within 30 minutes. On the lower parts of the intertidal flat the flows are approximately bidirectional, the variance around the mean direction is more scattered. Higher up on the intertidal flat the flow direction is even more variable in time and at some locations a rotation is present in the transition from flood to ebb. On transect T1 the direction during flood rotates
210 counter-clockwise towards the ebb direction and remains during ebb parallel to the local depth contour. On the intertidal flat at transect T2 the flow rotates clockwise, opposite to the direction of transect T1.

To analyse the effect of spring tides and events with large setup on the flow pattern, the DA₉₅ series are compared to the DA₅₀ series. In the channel the differences are minor: only the magnitudes are larger, but the timing of peak flood, peak ebb, and slack tide is similar. On the intertidal flat the differences are more pronounced: the DA₉₅ magnitudes are much larger and
215 the timing of peak flood, peak ebb and slack tide with respect to local high tide can be different: at location '14' the magnitude decreases towards the moment of slack tide, which is contrary to average conditions. At location '32' a pronounced maximum flow occurs 3 hours after high tide, which is ca. 1 hour later than during average conditions.

Figure 6 shows the large-scale velocity pattern at an average ebb and average flood (averaged for each location over all measured peak ebb and flood flows). Note that at each location the length of the time-series is different, because of the sensor
220 falling dry or missing data. The overall morphology-inferred pattern of ebb and flood dominant channels as shown in figure 1 is confirmed by the hydrodynamic observations: the majority of the locations are flood dominant as expected. Exceptions are

location '31' which is clearly ebb dominant, and location '41' which has equal ebb and flood flow magnitudes. During flood the flow bends around the higher part at the westward side of the mid-channel bar. This causes relatively weak alongshore and strong cross-shore flood flows at the higher parts of transects 1 and 2. The peak flood flows in the channel are larger than
225 on the mid-channel bar and for transects 3 and 4 the flows are mainly directed along local isobaths. At peak ebb the velocity direction changes approximately 180 degrees, although there are some exceptions ('12' and '24'). For most locations the near-bed, depth-averaged and near-surface velocities differ only a few degrees in direction, but for locations '11', '12' and '21' the difference in direction is more pronounced. At all locations the magnitudes increase from the bed to the surface.

3.2 Sediment transport

230 The sediment transport in the along-shore and cross-shore direction is first presented and the large-scale sediment transport pattern during ebb and flood is subsequently analysed. Our main focus is the transport of coarser material, therefore the near-bed and the depth-averaged sediment transport is addressed.

Figure 7 shows the cumulative along-shore and cross-shore sediment transport as a function of time at all locations based on the near-bed and depth-averaged velocities. At the channel locations '11' and '21' the net transport is in the direction of
235 the flood current. These locations have a small component onto the mid-channel bar. Locations '31' and '41' show opposite behaviour: the net transport is in the ebb direction, while location '31' has a component away from the mid-channel bar. The cumulative transport shows a steadily increasing or decreasing trend over time, suggesting that the transports are dominated by the tides. Locations '41' and '42', located in the flood-dominated channel terminus, show a strong modulation due to the neap-spring tidal cycle with reversing net direction of transport in calm conditions. Higher up the tidal flat the transport is
240 much more influenced by episodic events. All locations on the tidal flat clearly show the role of the large storm event in March 2019. The cumulative alongshore transport shows a sudden change during this event. After this event the behaviour is more gradual and for location '24' even in the opposite direction. The sediment transport is directed onshore for transect T2, but offshore in the upper parts of transect T1, suggesting a recirculation of sediment in line with the mean flows. At all transects the cumulative sediment transport ~~near-the-bed-is-smaller-then-the~~ proxy applied to the near-bed flow velocity is smaller than that
245 applied to depth-averaged ~~transport-in-along-shore-in~~ alongshore as well as cross-shore direction. This is a direct consequence of the smaller velocity in the lowest cell compared to the depth-averaged velocity. While a lower magnitude is expected, our interest is the change in direction of near-bed flow due to three-dimensionality of the flow around the bar.

The large-scale sediment transport pattern for mean flow during ebb and flood is shown in Fig. 8. The NB_{50} and the DA_{50} shown in this figure are used to calculate the sediment transport given by equations 1-2. During flood, net sediment is trans-
250 ported onto the mid-channel bar at transects T1 and T2. In the eastern part of the field site, i.e. transects T3 and T4, the net sediment transport in the flood direction mainly takes place along the isobaths, with the exception of location '31' where sediment is transported into the ebb direction. The results show that differences in magnitude as well as direction between depth-averaged and near-bed transport are present, although the differences in the direction are small ($< 2^\circ$).

3.3 Development of the bed surface elevation in the cross-shore transects

255 ~~During each~~ Each month, except in March 2019 due to severe weather conditions, the field site was visited and the height of the bed along the transects was recorded (Fig. 9). At transect 1 we observed sedimentation during the period February to April. Nearby location '13' the sedimentation amounted to 0.57 m and towards location '14' to 0.25 m. These locations also had the strongest veering. After this period the sediment deposited did not erode, the area between the two was filled with sediment resulting in an accretion of this area of 0.35 m during the measurement period. Transect 2 accreted gradually but
260 mildly between February and July with at most 0.17 m along the transect. At transect 3 and 4 the bed height on the tidal flat gained a height during the measurement period of maximal 0.10 m. In the profile below -1 m NAP at transect 3 the ridge that was present in February eroded during the measurement period. In Fig. 1(c) the elevation over a period of 10 years is shown. Comparing this with the elevation measured during the field campaign, we see that an above average accretion has occurred at transect T1 and to a lesser extent at transect T2. The height development along transects T3 and T4 is in agreement with the 10
265 year average.

4 Discussion

First we discuss the horizontal patterns of cross-shore and along-shore currents and contrast these with the potentially simpler shore-connected tidal flats as conceptualised in a commonly used mathematical model for cross-shore flow and a mass balance for the along-shore flows for the areas defined by our measurement stations. Next, the vertical profiles of flow velocity are
270 compared between the stations to assess the potential effect of veering on sediment transport. At last we discuss how the morphology and external forcing affect the large-scale sediment transport patterns.

4.1 What drives the cross-shore flow?

The cross-shore flow velocities are important for sediment transport onto and off the tidal flat. Since we know that the mid-channel bar has a 3D morphology and is not alongshore uniform, we are interested whether a 1D model is suitable for such a location.
275 Therefore, to further understand what drives the cross-shore current, we compare observed depth-averaged cross-shore currents with those of the 1D model as described in (Friedrichs and Aubrey, 1996). In this model, it is assumed that water levels are spatially uniform over the tidal flat and that the morphology of the tidal flat is alongshore uniform. The depth-averaged cross-shore tidal current $U_T(x, t)$ can then be described by:

$$U_T(x, t) = \frac{x_f(t) - x}{h(x, t)} \frac{d\eta(t)}{dt} \quad (3)$$

280 where

$x_f(t)$ = is the boundary between the wetted and exposed portions of the tidal bar

x = the cross-shore position on the profile

t = the time

$h(x, t)$ = the water depth, which is the difference between the water level and the bed level

285 $\eta(t)$ = water level, assumed to be spatially uniform.

We applied this model to each transect to calculate the resulting cross-shore velocities on the intertidal flat. The bed profiles were taken from the LiDAR dataset recorded in April 2019 and were smoothed with an averaging window of 100 meters. The water levels for each transect were based on the time series of channel locations ('11' to '41'; see methods section). Since the
290 1D model does not account for full submersion of a transect, which occurred during several tidal periods, the maximum value that x_f could obtain was set to the distance of the highest point measured from $x = 0$, i.e. the channel location. The measured depth-averaged velocity was split into a cross-shore and along-shore direction. The local along-shore direction was taken parallel to the local isobath derived from the smoothed LiDAR bathymetry, the cross-shore direction was taken perpendicular to the along-shore direction, positive towards the tidal flat according to Friedrichs and Aubrey (1996).

295 The correspondence between modelled and measured cross-shore velocities was quantified by linear regression, the coefficients are shown in Table 1. Most locations have $R^2 > 0.6$, whereas the locations on transect T1 located behind the elevated area, i.e. locations '13', and '14' and the locations near the channel of transects three and four, i.e. location '32', and '42' have $R^2 < 0.50$. Although at many locations the correlation coefficient $R^2 > 0.65$, the slope (a) is in general much smaller than 1, typically around 0.5, which means that the measured cross-shore velocities are larger than the modelled ones. For transect T1
300 and T2 this is in line with the observation that the flow rotates onto the tidal flat thereby adding momentum to the cross-shore component with higher velocities as a result. It is known from field visits during this campaign that the bed-level is not uniform and differs in composition spatially and temporally. We did look into the relation between flooding of the intertidal bar and the effect on the flow, but no relation between the two was found. ~~An underestimation of the model can be caused by the fact that in contraction to the model's definition, the bed is concave upward instead of linear. Also, the modelled depth-averaged~~
305 ~~cross-shore velocities are derived from the equation of conservation of mass under the assumption of uniform along-shore flow and spatially uniform water levels, i. e. $\frac{\partial hu}{\partial x} = 0$. The underestimation of the cross-shore current velocities is likely caused by the fact that the along-shore current is not zero.~~

4.2 What drives the along-shore flow?

To gain better understanding of the role of along-shore flow in filling and emptying of the intertidal flat, we solved the mass
310 balance for all quadrangular areas between the measurement positions at the field site. These areas are depicted in Fig. 2 with dashed lines and numbered one to nine. The mass balance of an area 'A' is calculated based on the definitions shown in Fig. 10

Table 1. Coefficients of linear regression between the measured cross-shore velocities and a 1D numerical model

Transect	Location	R^2	a	b
1	2	0.68	0.21	-0.084
	3	0.13	0.17	-0.013
	4	0.08	0.34	-0.024
2	2	0.74	0.36	0.008
	3	0.87	0.58	0.005
	4	0.95	0.69	-0.013
3	2	0.40	0.44	0.005
	3	0.69	0.61	-0.002
	4	0.66	1.00	-0.017
4	2	0.21	0.40	-0.002
	3	0.81	0.69	-0.010
	4	0.91	0.45	0.003

(a). The discharge Q through each boundary with length L is calculated as:

$$Q_1 = \frac{h_{11}\mathbf{u}_{11}\hat{n}_1 + h_{21}\mathbf{u}_{21}\hat{n}_1}{2} \Delta L_1 \quad (4)$$

$$Q_2 = \frac{h_{11}\mathbf{v}_{11}\hat{n}_2 + h_{12}\mathbf{v}_{12}\hat{n}_2}{2} \Delta L_2 \quad (5)$$

$$315 \quad Q_3 = \frac{h_{12}\mathbf{u}_{12}\hat{n}_3 + h_{22}\mathbf{u}_{22}\hat{n}_3}{2} \Delta L_3 \quad (6)$$

$$Q_4 = \frac{h_{22}\mathbf{v}_{22}\hat{n}_4 + h_{21}\mathbf{v}_{21}\hat{n}_4}{2} \Delta L_4 \quad (7)$$

The mass balance equation for a single area reduces to:

$$\sum_{i=1}^4 Q_i = -A \frac{\partial \eta}{\partial t} \quad (8)$$

where A is the surface of the area. Note that the area in practice is not square since the measurement locations are not placed
320 at an equal distance. Figure 10(b) shows the results for the area between the locations ‘22’, ‘32’, ‘23’, and ‘33’: the sum of the discharge calculated over the boundaries is in agreement with the discharge derived from the water level variation times the surface of the area.

We analysed the along-shore and cross-shore discharge to understand their contribution to the total discharge of the area. Figures 11(a), (b) show the times series. During flood the net flow in the along-shore direction is positive (eastern direction),
325 whereby the area is emptied. In the cross-shore direction the net flow is negative (southern direction), filling the area. Since during flood the net influx must result in an incremental water level, the net cross-shore discharge must compensate the loss in along-shore direction resulting in a large cross-shore discharge at the boundary ‘Q2’. During ebb the net flow in the along-shore

direction is negative, i.e. the area is filled from the east, while the net flow in the cross-shore direction is positive (outflow to the north) and thus empties the area. Based on the flux quantities we conclude that part of the along-shore discharge at 'Q3' is added to the cross-shore discharge at 'Q2'. The net discharge created by the along-shore flow is compensated by the cross-shore discharge resulting in an increment of the cross-shore velocity. This explains the larger cross-shore flow velocities during flood and ebb than those based on the 1D approach presented earlier where the along-shore flow did not contribute to the discharge.

Although the results derived for this area are satisfactory, the method fails at the other areas. A possible explanation is that the areas defined by the measurement locations are rather large compared to the observed rotational patterns as shown in Fig. 6, so that we lack information on flow behaviour in between the measurement locations.

4.3 Large-scale sediment transport patterns on mid-channel bars and their dependence on external forcing hydrodynamics and morphology

In this section the effects of the hydro-meteorological conditions and the morphology of a mid-channel bar on the large-scale sediment transport patterns are described. First, we consider the effect of wind speed on the current flow in relation to the sediment transport. Second, we relate the local morphology to the large-scale sediment transport patterns.

The results of the proxy for sediment transport showed that at locations '11', '21', and '31' the cumulative transport is a steady process, while at location '41' and at the mid-channel bar it has a more episodic character. For the locations on the mid-channel bar a probable cause is the stronger influence of wind on the shallower parts of the tidal flat (Hir et al., 2000; de Vet et al., 2018; Colosimo et al., 2020). To quantify the effect of wind on the peak velocities we compared the peak current velocities during situations with almost no wind (wind speed less than 3 Beaufort) with those occurring during periods with high wind speed (more than 6 Beaufort). To do so, we proceeded with the following steps. First, we divided the dataset containing peak velocities into two subsets: dataset one (DS1) containing peak velocities during wind speeds smaller than 3.3 ms-1 (3 Beaufort), and dataset two (DS2) containing peak velocities with wind speeds larger than 10.8 ms-1 (6 Beaufort) coming from a westerly direction (SW to NW). For both datasets the tidal range associated with the peak velocity was also registered. Second, for each station-measurement location a linear fit was made between the tidal range (X) and the peak velocities of DS1 for ebb as well as flood. Finally, we compared the (Y). Third, the obtained regression coefficients were used to calculate the peak velocities of DS2 with the peak velocities calculated with the linear fit of DS1. Figure 12 shows that wind has given the tidal range in the dataset. Finally, to determine the effect of higher wind speed represented in DS2, the fraction between measured and calculated peak velocities was calculated. These are shown for flood and ebb on a logarithmic scale in Figure 12. We see the most pronounced effect on the current flow at the shallowest parts of the tidal flats, especially during flood. This strong increase of flow velocities throughout flood high up the tidal flat also explains the more episodic character of the proxy for sediment transport at the highest parts of the tidal flat. We did not find flow reversal caused by wind effects as described by (Colosimo et al., 2020) on tidal flats, likely due to the fact that the strong wind effects came from the west and were parallel to the dominant flood direction. During ebb when wind and tide are in the opposite direction, there is a reduction of the peak flow, although at the most shallow locations the current flow is amplified, despite the ebb current. The mean tidal range for both

periods is almost the same: 3.72 m and 3.68 m for DS1, resp. DS2. Therefore, changes in the peak velocities will be mainly caused by the wind stress.

The mid-channel bar is located within an area of flood- and ebb-dominant channels which influences the ebb and flood dominance at the mid-channel bar itself: while locations '11' and '21' are clearly flood dominant, there is a sudden spatial transition to ebb dominance for location '31'. This could be caused by flooding of the northern sand spit during ebb from the adjacent ebb-dominant channel, thus affecting the flow direction at the locations '31' and '41'. Although this cannot be confirmed due to a lack of measurements we hypothesise that multiple channels with a different flow regime will affect the net flow on a mid-channel bar on local scales. This subject that has not been studied previously. A change in the global flow patterns due to the tidal gradient between adjacent channels during flooding of the mid-channel bar as suggested by (de Vet et al., 2018) was not found. Local morphology introduces veering of the flood current and is therefore important to the sediment transport onto the mid-channel bar. We hypothesise that these patterns, initiated by local morphology, will also occur on the edges of fringing tidal flats if these are constrained by groynes or transverse dikes. Seaward stretching dams will have in impact on the along-shore current and thus influences the sediment transport (Wang et al., 2019).

5 Conclusions

In this paper we analysed current velocities measured on a mid-channel bar at 16 locations during a period of 6 months to gain insight into large-scale flow patterns. To understand how a mid-channel bar is filled and emptied, we compared the velocities with the result of a 1D model for fringing tidal flats. The effect of hydrometeorological conditions and local morphology was obtained by analysing the large-scale flow and sediment transport patterns. This led to the following conclusions:

In contrast to open and fringing tidal flats, the cross-shore current is not the sole responsible driver for filling and emptying a tidal flat on a mid-channel bar. Instead, the along-shore current strongly affects the large-scale flow patterns on a mid-channel bar. This is clearest in the veering at the head of the mid-channel bar, thereby adding momentum to the cross-shore current. As the finite length of a mid-channel bar implies that the along-shore flow cannot be uniform, we deduced from the data that the along-shore flow is also driven by gradients in water level caused by local morphology. This result points to the effect of the bar shape on the flow pattern, which invalidates the common assumption that tidal flats are spatially uniform and only the cross-shore current needs to be considered. Whether the assumption of ~~nonuniformity~~ non-uniformity of the along-shore flow along fringing tidal flats holds when dams or groynes are present requires further research.

We ~~observed differences in the magnitude between near-bed and depth-averaged~~ used a proxy to estimate the sediment transport. ~~Differences in direction between~~ Based on this proxy we observed differences in the direction and the magnitude between values nearest to the bed, i.e. the near-bed and lowest measurement cell, and the depth-averaged ~~transport found~~ value. Although magnitude differences in the vertical profile can be solved by a 2DH ~~models will overestimate the bed-load transport for mid-channel bars and other non-uniform tidal flats. Since external forces on the short term can have an impact~~

on the sediment transport directions, a representative time period for the calculation is a constraint when modelling model, the question is if and how 2DH models differences in direction over the vertical profile can be solved or that more sophisticated models are needed.

The commonly used cross-shore flow model valid for uniform tidal flats and a model for along-shore flow calculated from the mass balance are of limited value because these models neglect rotation of the flow between the measuring points. As local morphology and tidal conditions determine these flow patterns, the question is how representative local measurement points can be for specific areas. This point is important when validating numerical models with local measurements but low model grid resolution: the placement of instruments should be in accordance with the field site. If this area is alongshore uniform, a cross-shore line-up is sufficient. When the main spatial changes are dominated by the alongshore velocity component, a line-up in the alongshore direction is needed. When the study area is fully 2D, the instruments may be placed in a cross line-up, or as an array as used in this study.

During events that amplify current flow, i.e. storm conditions with high wind velocities coming from the net flow direction, sediment transport is also amplified. In coastal regions and estuaries such as the Western Scheldt where sediment is relocated by dredging and disposal, knowledge of the sediment pathways to preserve or maintain intertidal habitat area is important. For further studies that take morphological changes and sediment transport into account it would be useful to identify possible sediment sources including disposal sites of dredged material, and measure, including locations of disposal sites and quantities of disposed sediment. It is advised to look into the effect of waves on the large sediment transport patterns, numerically as well as through measurements. Measurements are also advised on the sediment transport in suspension and on the bed. Besides providing valuable information, data on sediment transport and local wave information provide valuable information to validate model results.

Based on our results, we conclude that velocity patterns and concomitant sediment transport on a mid-channel bar are more complicated than assumed in simple models. Our analyses of hydrodynamics and morphodynamics show that specific morphological features associated with mid-channel bars result in deviations from commonly assumed flow patterns on tidal flats. Elucidating these processes and assessing how important the intricate patterns are for sediment transport requires a combination of numerical modelling and higher-resolution datasets with matching resolutions measurements that cover the spatial variations of the study area.

Data availability. Datasets of current velocity, bed elevation profiles and water levels of the measurement locations are available at Zenodo (<https://doi.org/10.5281/zenodo.15017660>). Wave data and water level data are available at the website of Rijkswaterstaat (<https://waterinfo.rws.nl>). LiDAR bed level is available at Rijkswaterstaat (<https://rijkswaterstaatdata.nl>). Wind en pressure data is available at the website of the Royal Dutch Meteorological Institutue (<https://www.knmi.nl>).

Author contributions. MS: data analysis, code development, writing (original draft, review and editing). MvdV: data analysis and writing
425 (review and editing). GR: review. MK: supervision, review and validation.

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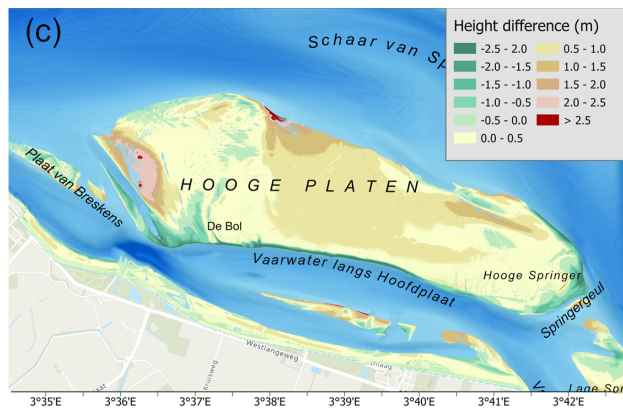
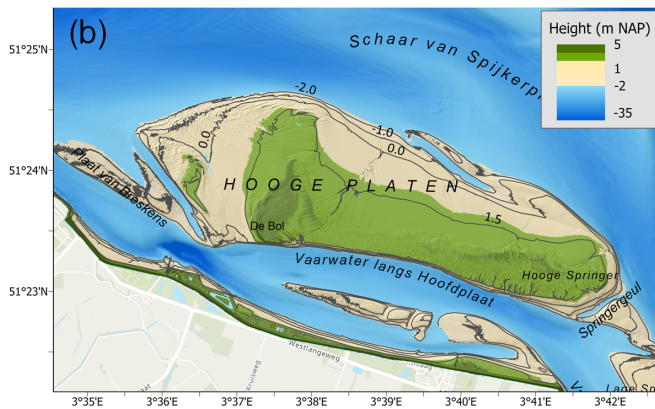
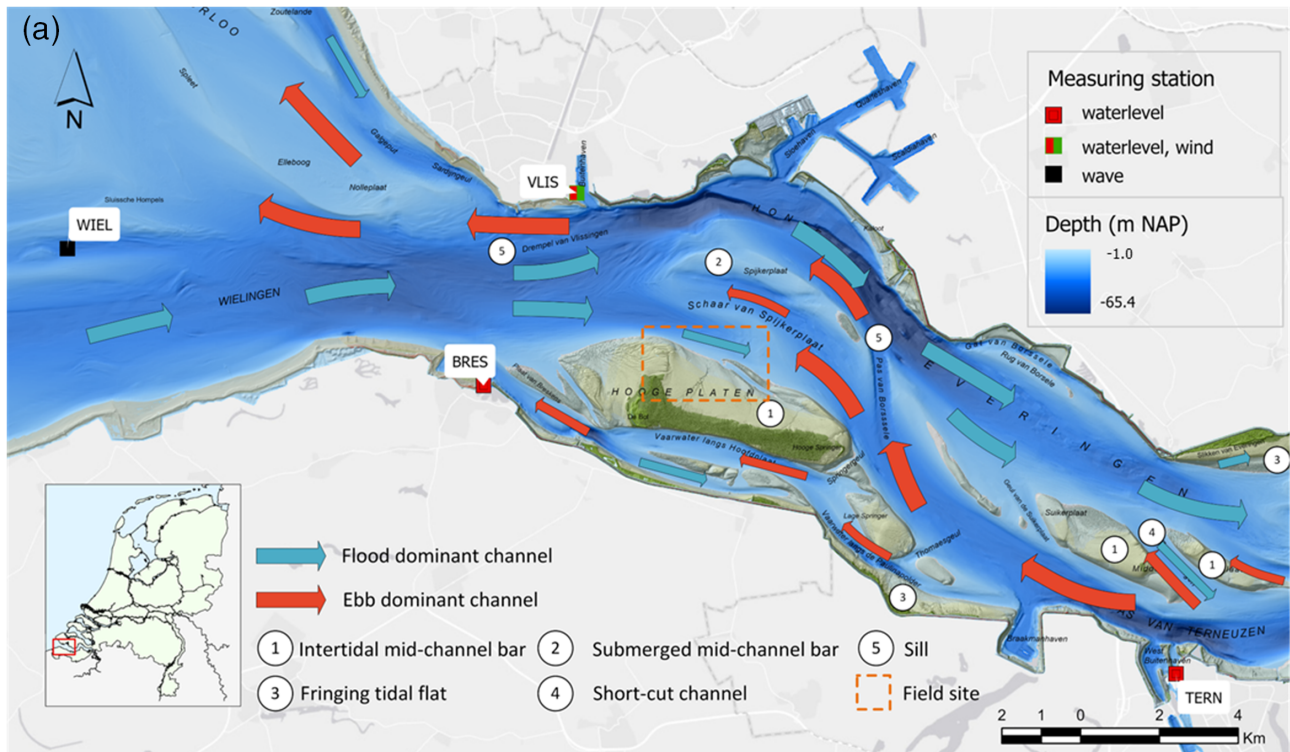


Figure 1. (a) Overview of the western part of the Western Scheldt in the Netherlands. The ebb and flood patterns are derived from model calculations (ScalWest) with a representative spring tide as boundary condition and the bathymetry of the year in question. (b) Height of the mid-channel bar 'Hooze Platen'. (c) Sedimentation/erosion between 2009 and 2019 in meters. Height is relative to Dutch Ordnance Level (NAP).

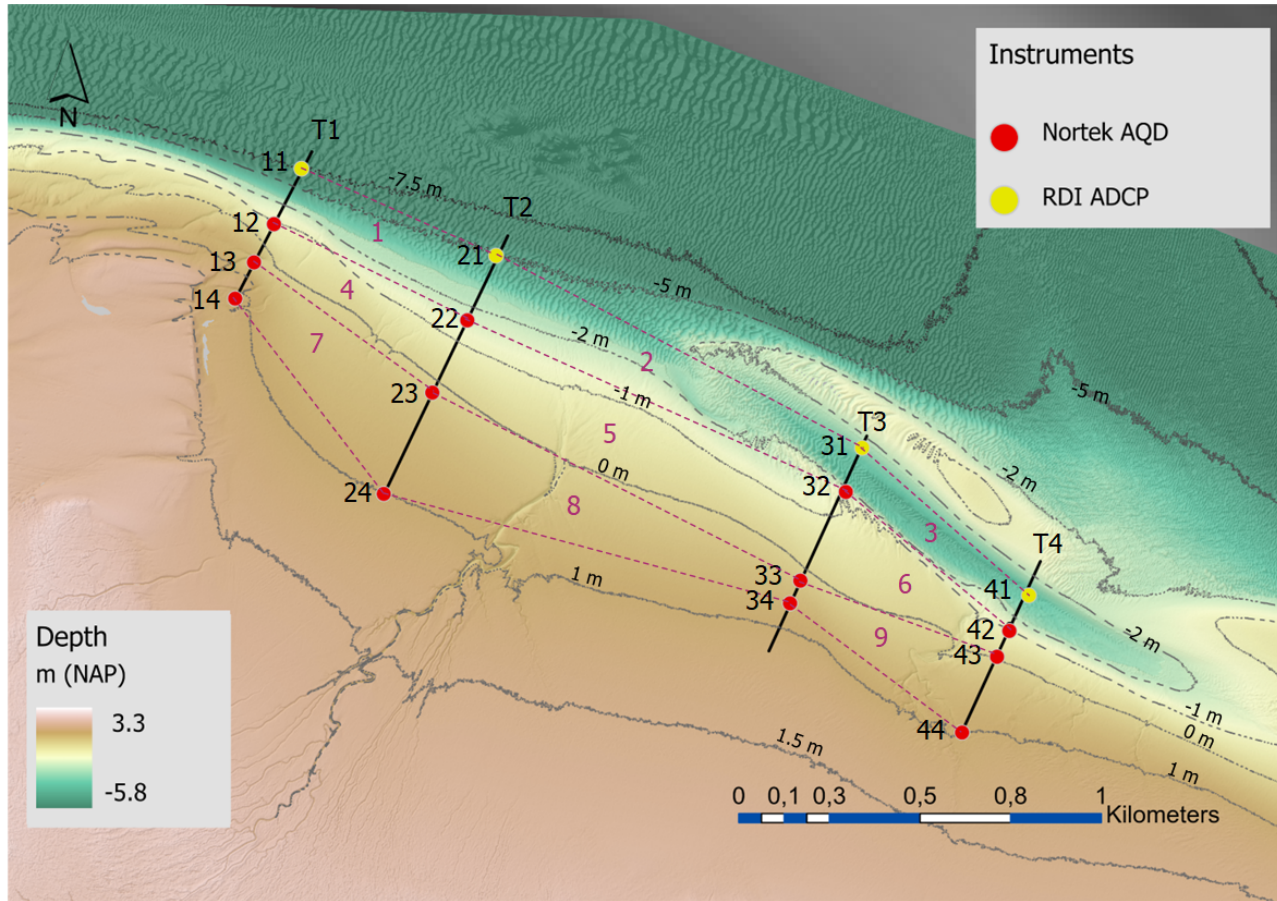


Figure 2. Overview of field site located at the [mid-channel intertidal bar](#) 'Hooge Platen' in the Western Scheldt. Four transect (T1 to T4) with associated instruments are shown. Instruments are labelled by their transect number (first position) and their position on the transect (second number). For the areas 1 to 9 the continuity equation is solved based on the measurements.

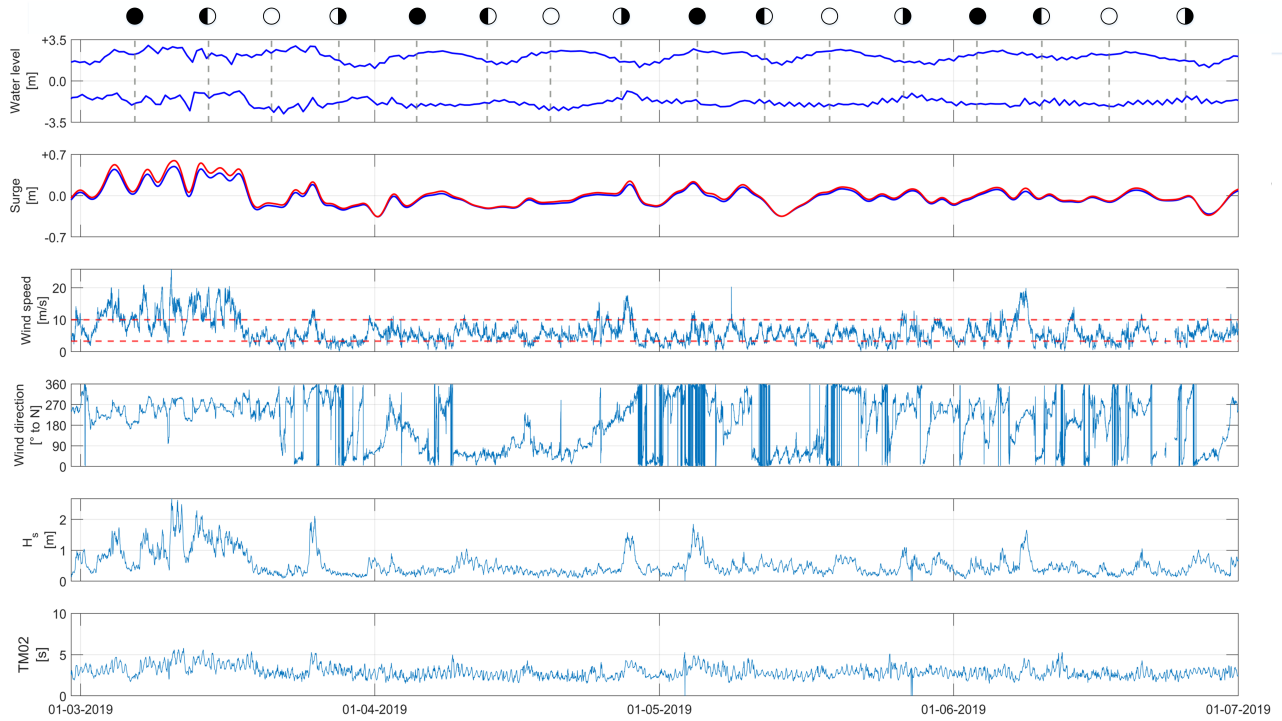


Figure 3. Time series of the hydro-meteorological conditions. (a) high and low water level at location '11'; water level setup at (b) external (WIEL, blue) and local (11, red); (c) wind speed (VLIS), wind speeds of 3 and 8 Bft are displayed as red dashed lines; (d) wind direction (VLIS); (e) Significant wave height (WIEL). (f) Wave period (WIEL). The phases of moon are shown above this figure, the locations are shown in Fig. 1(a) and ~~Fig~~Fig. 2.

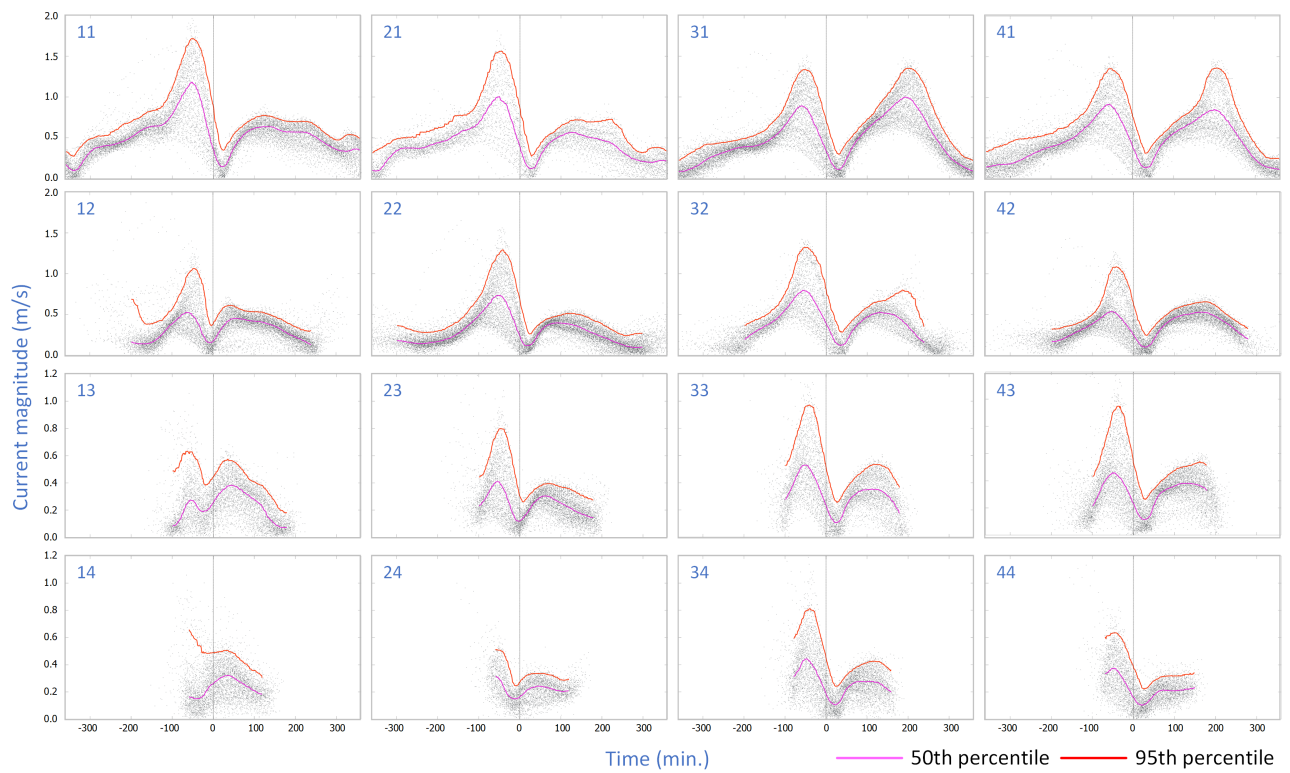


Figure 4. The depth-averaged current magnitude relative to time of high water for each location with 50th percentile (magenta), and the 95th percentile (red). Time of high water is indicated with the dashed line.

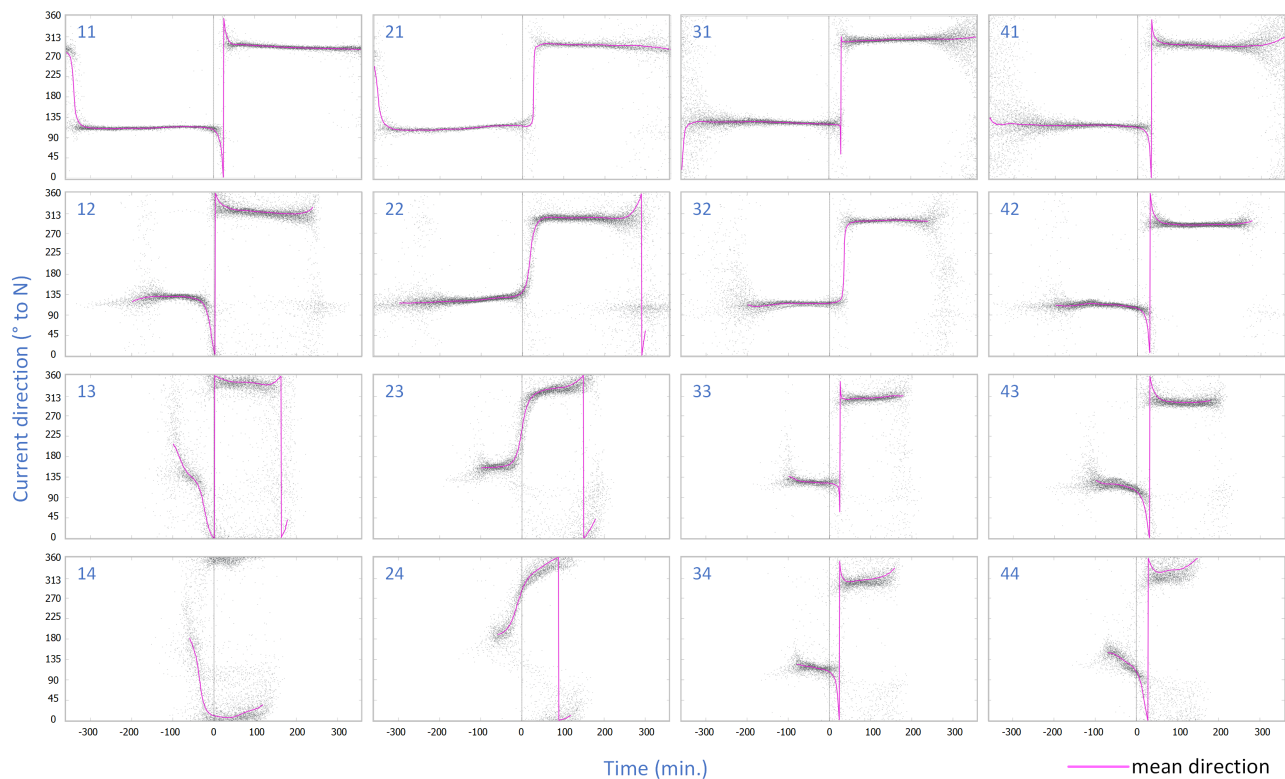


Figure 5. The depth-averaged current direction relative to time of high water for each location with the circular mean (magenta). Time of high water is indicated with the dashed line.

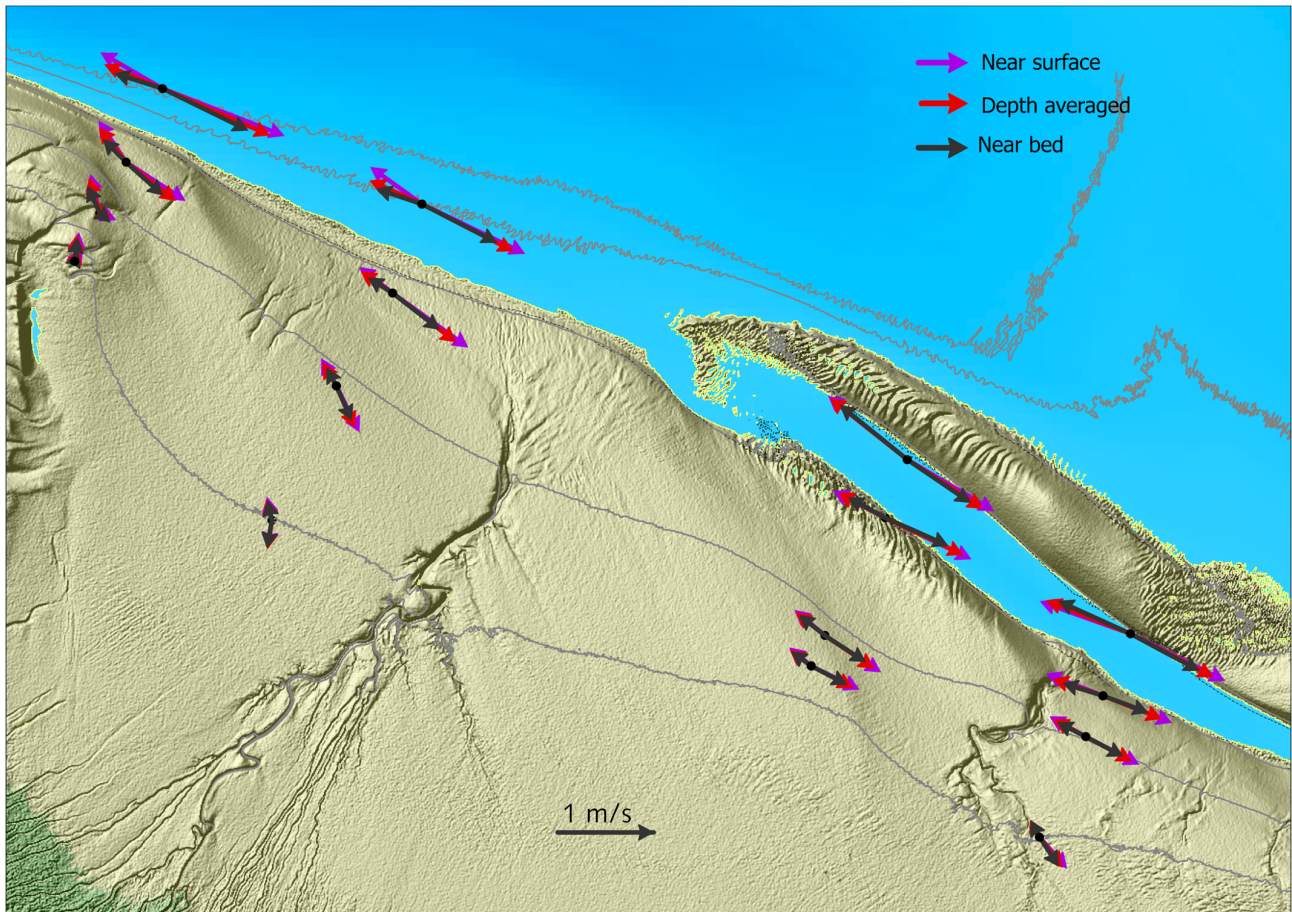


Figure 6. Average peak flood and peak ebb velocity for depths: near-bed, depth-averaged and near-surface.

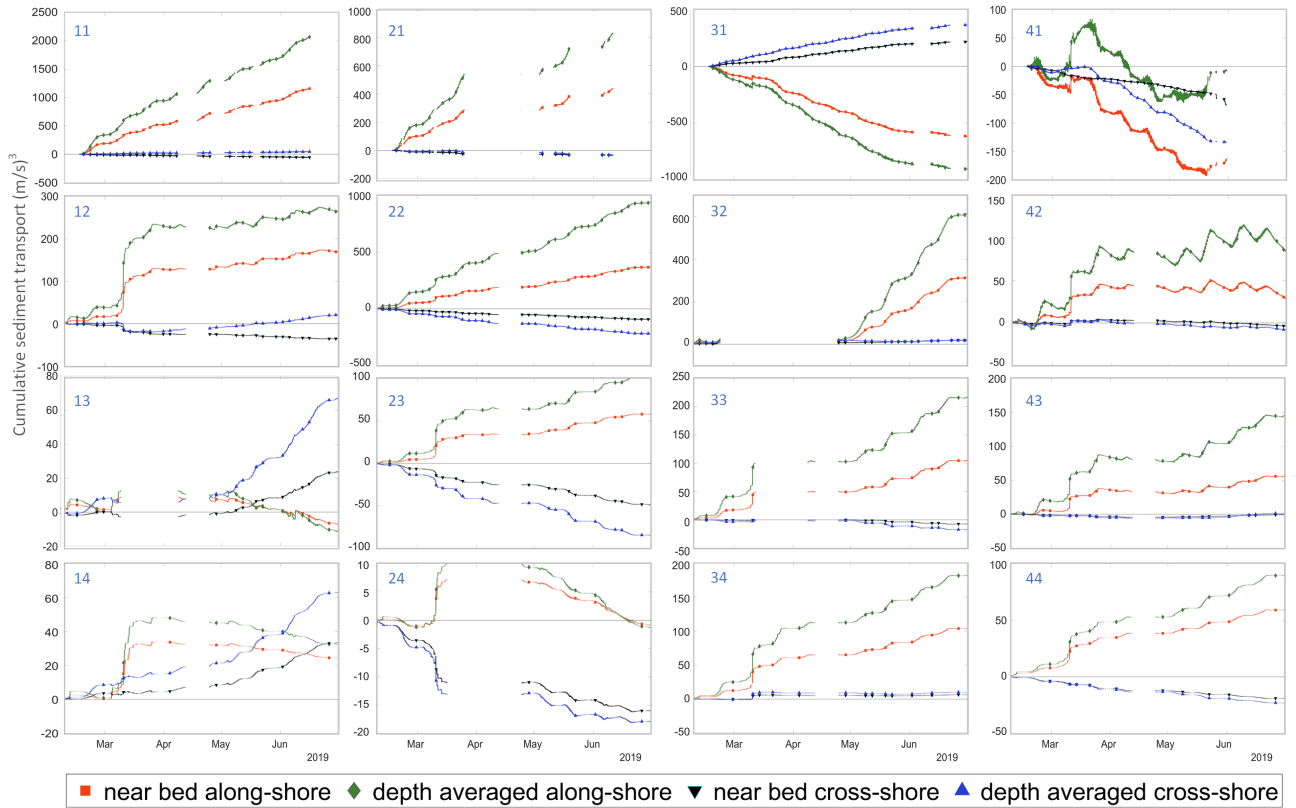


Figure 7. Cumulative sediment transport near-bed and depth-averaged in along- shore direction (positive towards east) and cross-shore direction (positive towards north).

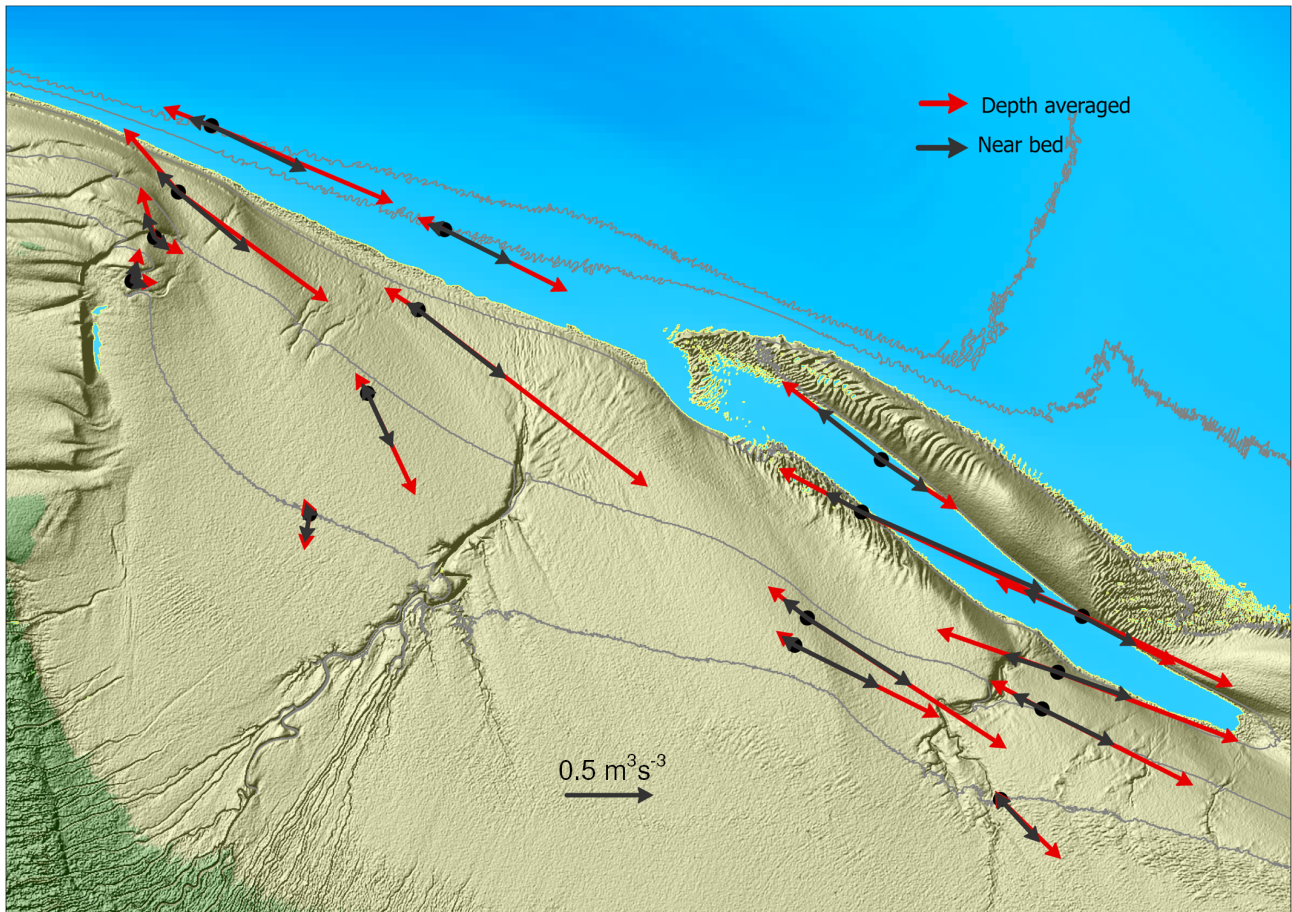


Figure 8. Sediment transport during ebb and flood for the mean flow near-bed and depth-averaged.

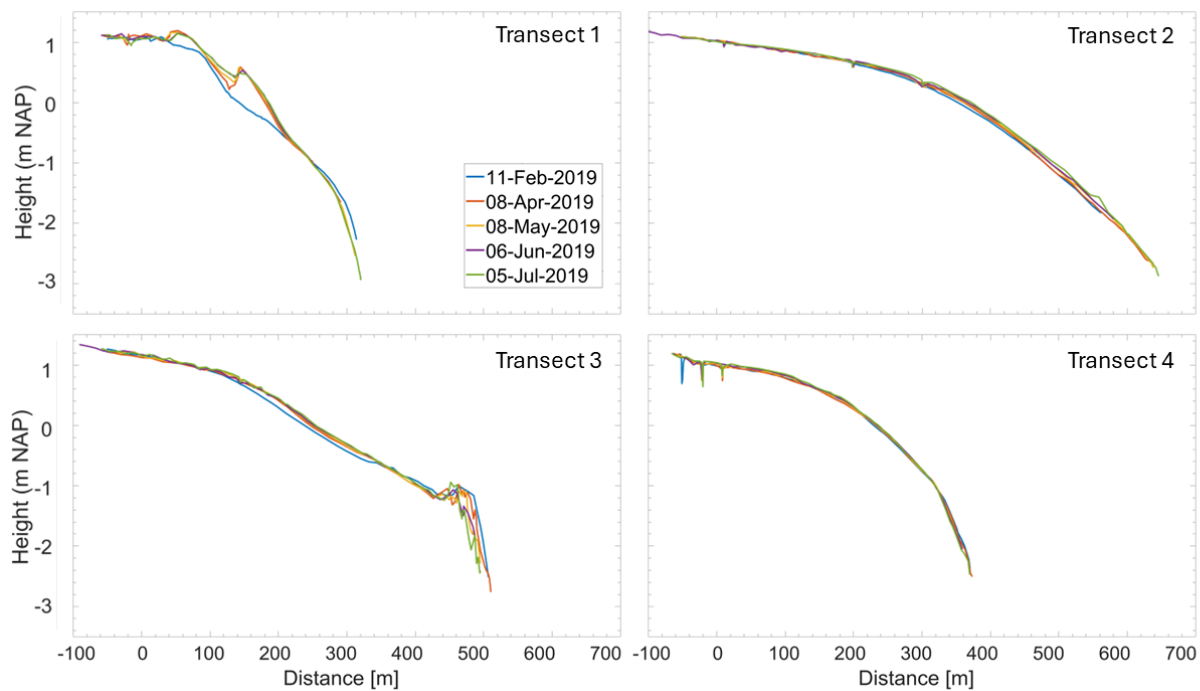


Figure 9. Height along the transects measured during the field campaign.

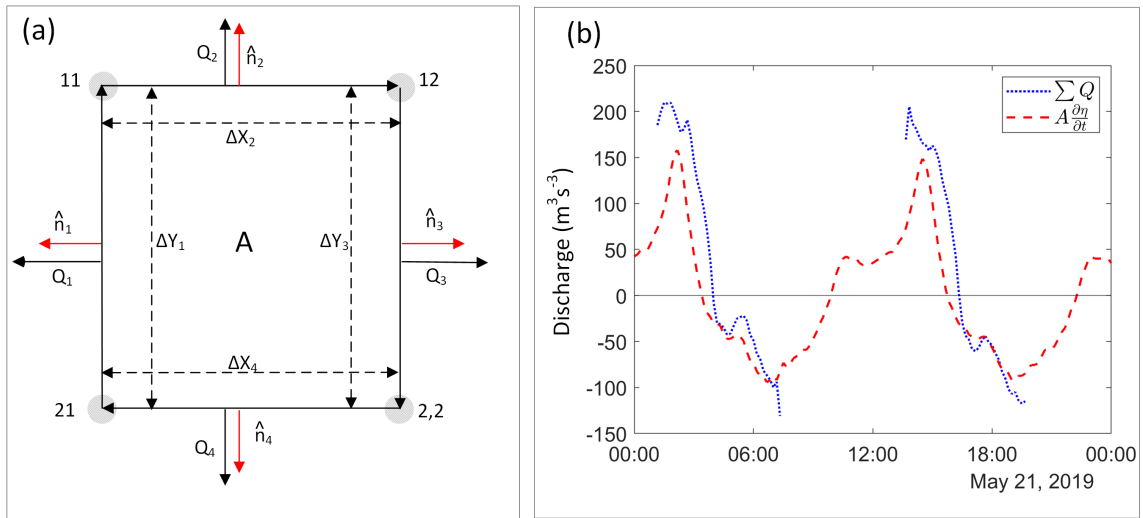


Figure 10. The mass balance of area 5 (see Fig. 2 for locations), in panel (a) the methodology is shown. Panel (b) displays the total discharge.

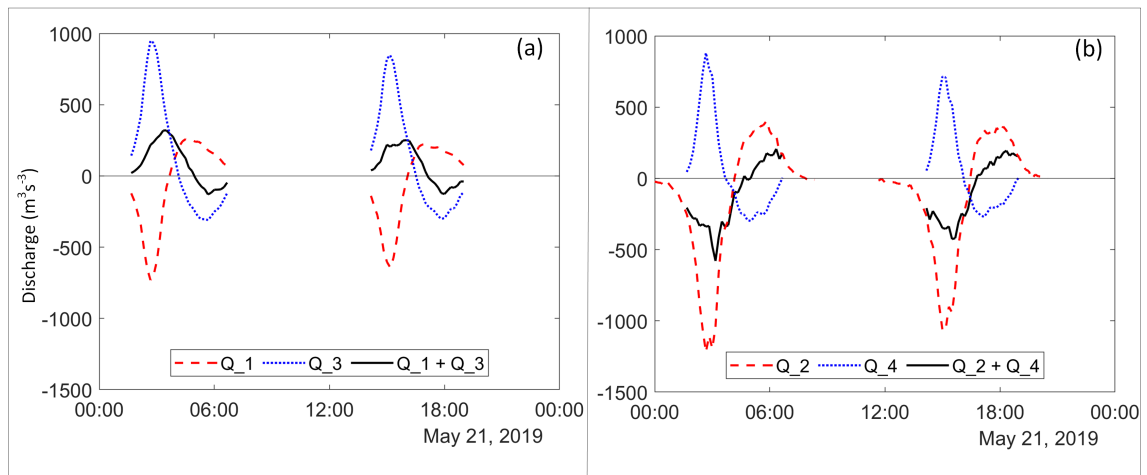


Figure 11. Discharge of area 5, in (a) along-shore direction, and (b) cross-shore direction.

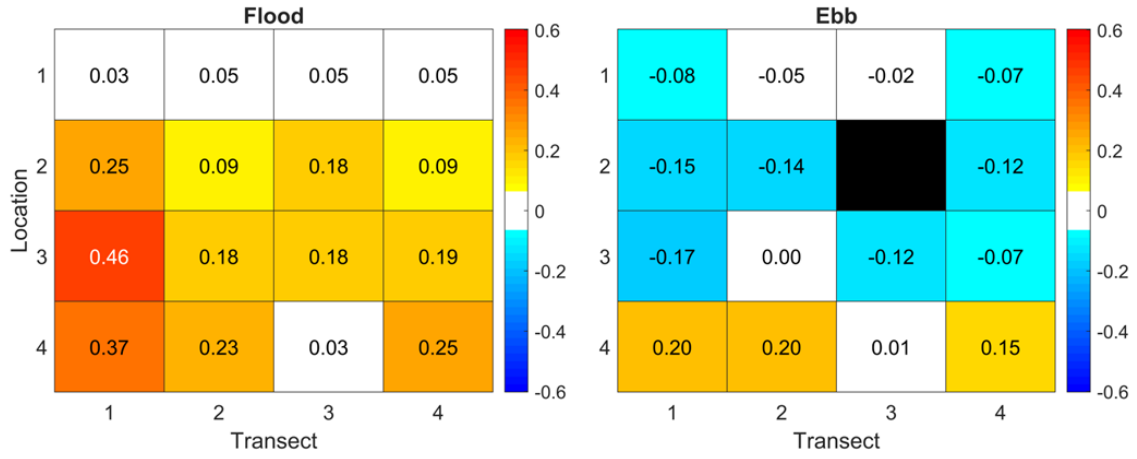


Figure 12. The $\log_{10}$ ratio between effect of wind on the measured and modelled current peak velocity. The parameters to calculate for flood and ebb calculated as $\log_{10} \left(\frac{U_m}{U_c} \right)$, with U_m the modelled-measured peak velocity were obtained from the linear regression between tidal range velocities at wind speeds above 8 Beaufort, and U_c the peak velocities calculated with the use of the coefficients derived from situations with wind speed speeds less than 3 Beaufort.