



# Influence of building collapse on pluvial and fluvial flood inundation of metro stations in central Shanghai

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**Abstract.** Urban flooding poses a significant threat to vulnerable underground infrastructure systems, such as metro stations. Building collapses induced by earthquakes alters urban building layout and coverage, consequently influencing flood inundation and propagation patterns. This study employs GPU-accelerated hydrodynamic simulation to investigate the mechanisms by which building collapse affects subsequent pluvial or fluvial flooding in the Huangpu district of Shanghai. Massive building collapse layouts are randomly generated, on which hydrodynamic simulations are performed and the inundation process of the metro stations are analyzed. The results reveal that pluvial floods are strongly influenced by localized topography distributed across the city. Consequently, building collapse has a more substantial impact on pluvial flooding when more buildings are collapsed. In contrast, fluvial floods are sensitive to the source location (e.g., location of levee breach) and the long travel route. Building collapse can either positively or negatively influence fluvial flooding by constricting or blocking the flow path. This work highlights the complex mechanism of earthquake-flood multi-hazard processes, emphasizing the importance of performing local-to-local analysis when both the hazard (e.g., individual building collapse, fluvial flood) and the hazard-bearing body (e.g., metro station) are localized. To better serve urban disaster prevention and mitigation, more efforts should be directed on developing physics-based high-resolution urban earthquake-flood simulation methods, as well as on acquiring data to drive such simulations.

## 1 Introduction

Climate change and urbanization have increased human exposure to urban flood hazards, particularly in the East Asia (Cao et al., 2022; Rentschler et al., 2023). To enhance urban resilience to flood hazards, there is an urgent need to understand the physical processes of flood propagation under various current and future scenarios. Hydrodynamic modeling is an important tool for urban flood studies. Recent advancements in high-performance parallel computing (HPC) have facilitated city-scale, high-resolution (on  $< 5m$  resolution digital elevation model, DEM) rapid hydrodynamic simulations (Guo et al., 2021; Morales-Hernández et al., 2021; Sanders and Schubert, 2019; Schubert et al., 2022a). Combining HPC technology and traditional hydrodynamic models, the high resolution spatial-temporal evolution of the inundation depth and extent can be reproduced or predicted, thereby aiding in the design and improvement of flood mitigation strategies.

When considering urban flood resilience, metro systems are the weak link. Underground infrastructures are inherently more vulnerable to floods, and evacuating passengers from inundated metro tunnels poses immense challenges. Megacities typically



have well-developed metro systems, but flooding of the urban metro system has been reported worldwide (Aoki et al., 2016; Forero-Ortiz et al., 2020; Toda et al., 2009). In China, megacities including Beijing, Shanghai, Guangzhou, Wuhan and Shenzhen have more or less experienced flooding of their metro lines (Lyu et al., 2019, 2020; Wang et al., 2021). In July 2021, a tunnel of the Zhengzhou (China) metro Line 5 was inundated due to heavy rainfall and inadequate emergency response measures, resulting in 14 fatalities (Yang et al., 2022). This devastating incident has raised nationwide concerns regarding comprehensive flood risk evaluation and the urgent need for robust mitigation strategies tailored specifically for urban metro systems.

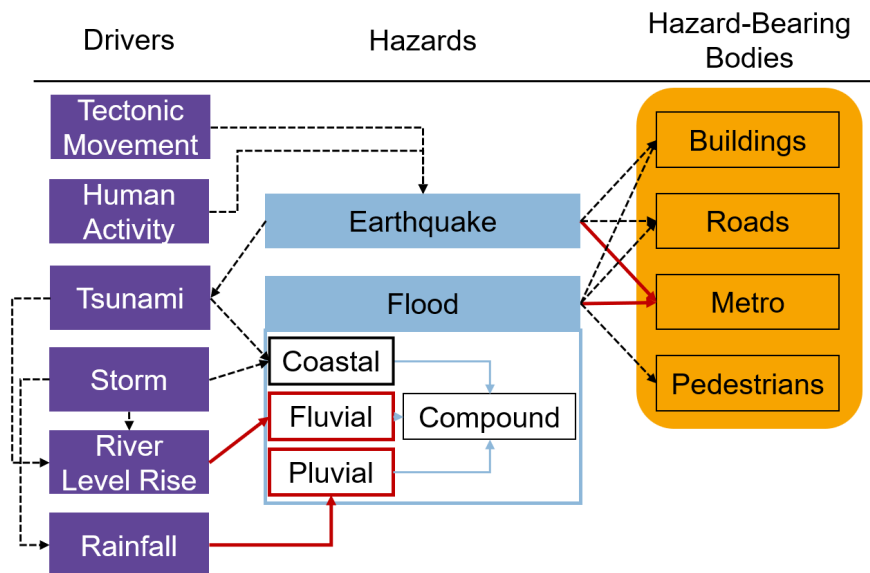
Modern metro stations and tunnels feature multiple flood prevention equipment such as flood gates, sand bags and pumps. The entrance of the metro stations are often elevated to avoid flooding. These flood prevention measures are expected to work well under most circumstances. However, it remains unclear if these measures are fully functional under extreme multi-hazard scenarios. Multi-hazard refers to scenarios where a system or infrastructure (e.g., a metro station) is subjected to multiple hazardous events, either concurrently or sequentially. The compounding effects of multi-hazards can potentially lead to more severe consequences compared to the impact of individual hazards. Compound flood is a common multi-hazard scenario. In 2013, typhoon Fitow triggered heavy rainstorm in Shanghai, China. At the same time, fluvial flood from the upstream attacked Shanghai. An astronomical high tide further exacerbated the compound flood, which finally caused 2 deaths and a loss of approximately 150 million US dollars. Geological hazards also compound the impacts of urban floods (Gill and Malamud, 2014). For example, it has been shown that land subsidence exacerbates flood depth and extent (Johnston et al., 2021; Navarro-Hernandez et al., 2023). Another example is the sequential occurrence of a magnitude 7.8 earthquake in April 2015 and a severe flood event in August 2017 within the same region of central Nepal. The infrastructure damaged during the seismic event had not been fully rehabilitated when the flooding occurred, leading to more devastating damage compared to if the area was attacked by a single disaster (Gautam and Dong, 2018). For coastal cities, earthquake could induce tsunami, which turns into coastal floods. After the earthquake, the flood resilience could be compromised due to two main factors: (i) the flood-prevention structures (e.g., flood walls) and equipment (e.g., pumps) could be damaged during the earthquake, and (ii) the collapsed buildings could block the evacuation routes. The confluence of these two factors can exacerbate the severity of coastal flooding impacts (Goda et al., 2019; Ito et al., 2020; Román-de La Sancha et al., 2022; Takabatake et al., 2022).

Urban multi-hazard studies have attracted increasing attentions in recent years. However, existing studies often focus on risk assessment that is based on historical data or simplified simulations (e.g., Depietri et al., 2018; Owolabi and Sajjad, 2023). With the advances in HPC and urban flood simulation, it is possible to reproduce large-scale, high-resolution flood propagation process under multi-hazard scenarios. This allows for an investigation on the physical mechanisms how multi-hazards affect urban flooding compared with pluvial or fluvial flood hazard alone. With better depiction of the multi-hazard flooding process, risk assessment methodologies can be enhanced too. However, an in-depth exploration of improved risk assessment techniques is beyond the scope of the present manuscript.

In this work, we use high-resolution hydrodynamic simulations to estimate the evolution of pluvial/fluvial flood depth at 17 metro stations in the Huangpu district located in the central region of Shanghai. We compare the flooding dynamics in the actual Huangpu district to scenarios that incorporate multiple random realizations of building collapse. In these simulated



scenarios, certain buildings are assumed to have collapsed, obstructing roads and altering the urban layout. This approach serves as an approximation of post-earthquake conditions, enabling the analysis of the multi-hazard process involving the cascading impacts of seismic activity and subsequent inundation. We aim at understanding if, how and why earthquake-induced building collapse affect the inundation of the metro stations. The findings will serve as the cornerstone for multi-hazard risk assessment of urban metro systems. As depicted in Fig. 1, the primary focus is on quantifying the hydrodynamic consequences of collapsed structures on flood propagation and accumulation at metro stations. Topics such as how the earthquake is triggered, how the buildings are collapsed and how the flood enters the metro station are beyond the scope of this manuscript.



**Figure 1.** A flowchart of possible earthquake-flood multi-hazard mechanisms. The red lines delineate the focus of this manuscript.

The rest of this manuscript is arranged as follows: Section 2 describes the study site (Section 2.1), the numerical simulation methods (Section 2.2) and the building collapse scenarios (Section 2.3). Section 3 reports and analyzes the simulation results.

Section 4 discusses the implications, limitations and future works of this study. Section 5 draws the conclusions.

## 2 Methods

### 2.1 Study Area

The study area is the Huangpu district located in the center of Shanghai. Shanghai is one of the largest cities in China, with a population of approximately 25 million. Huangpu district has an area of about 20 square kilometers and a population of 0.66 million. Huangpu district is bounded by the Suzhou River in the north and the Huangpu river in the east and south. The west



boundary connects the Jingan and Xuhui districts of Shanghai (Fig. 2). As of 2023, the Shanghai metro system has 19 metro lines, in which 8 lines pass through the Huangpu district, forming 17 stations (labeled as station 1 to 17 in Fig. 2).

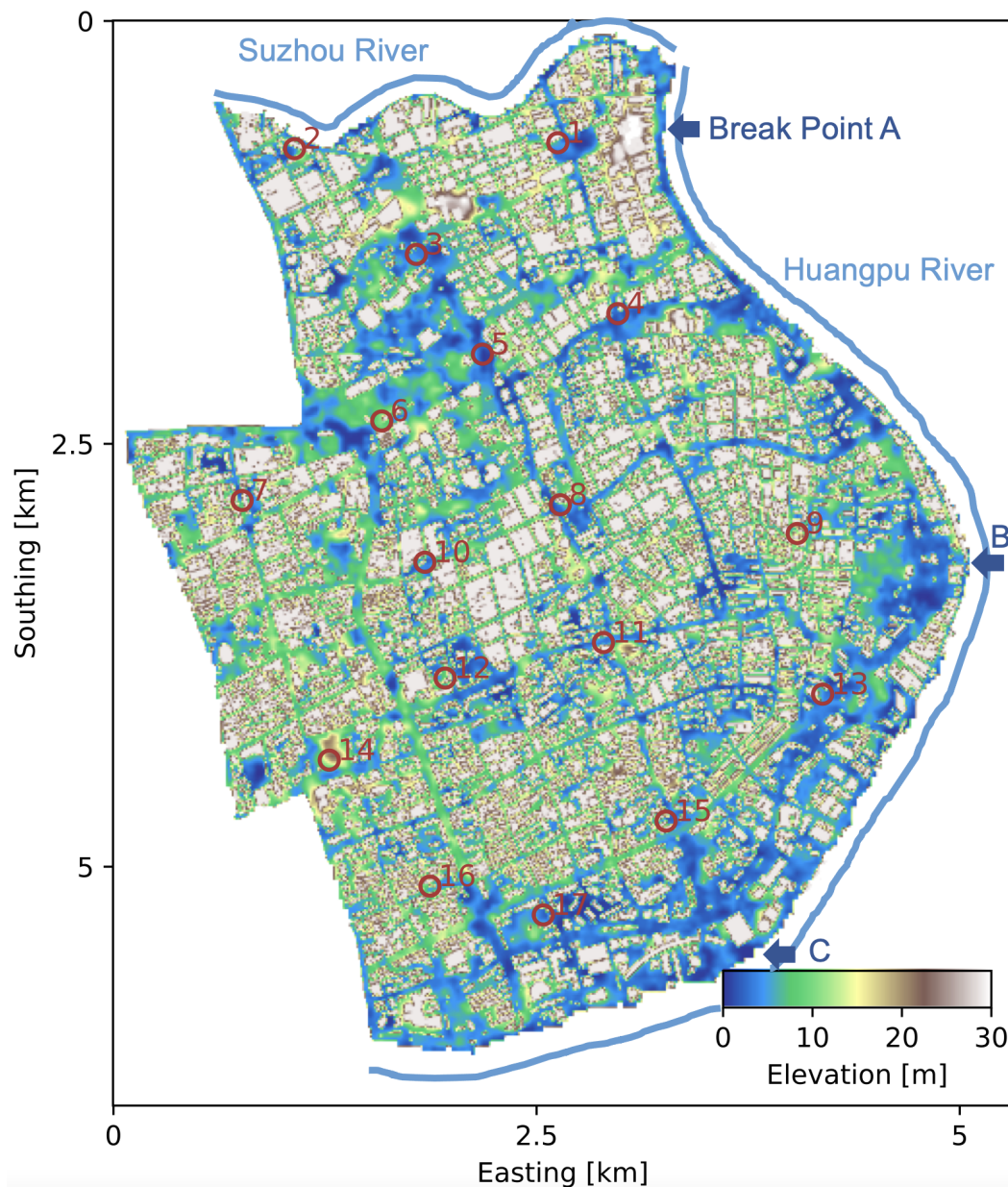
Multiple drivers could lead to or aggravate flooding in Shanghai, including typhoon, astronomical high tide, upstream fluvial flood, low-lying topography, sea level rise and land subsidence. These factors make Shanghai one of the most vulnerable cities to flood in the world (Balica et al., 2012). Shanghai adopts four levels of flood defense systems: the sea walls, the river flood walls, the pump stations and the pipe drainage system. The current defense system is generally effective, but might be inadequate under occasional extreme conditions or future climate change scenarios (Ke et al., 2021; Yin et al., 2021; Zhou et al., 2017). For example, Shanghai experienced a short but intense rainfall event on July 21th of 2023, with a maximum rainfall rate exceeding 100mm per hour (Fig. 3), leading to mild waterlog. The reported inundation depth reach 25cm at multiple locations in the city. It remains important to better understand the flooding mechanisms in Shanghai and keep improving flood prevention system and flood response strategies correspondingly.

## 2.2 Numerical Simulation

In this work, the flooding of the Huangpu district is simulated with the SERGHEI-SWE model (Caviedes-Voullième et al., 2023). SERGHEI-SWE is an open-source, high-performance hydrodynamic model. It solves the two dimensional shallow water equations (SWE) on Cartesian grids using Godunov-type finite-volume methods (Morales-Hernández et al., 2021). SERGHEI-SWE is a parallel SWE solver that achieves performance portability across a variety of computational backends, such as OpenMP, CUDA and HIP. This feature is achieved through the Kokkos framework (Trott et al., 2022). It also supports distributed memory parallelization through MPI. GPU computing have dramatically reduced the computational cost for large-scale hydrodynamic simulations. Caviedes-Voullième et al. (2023) has shown that using SERGHEI-SWE, for a computational domain with 0.5 million grid cells, hydrodynamic simulations on a workstation GPU remain faster than that on 128 CPU threads. Thus, SERGHEI-SWE with GPU computing capability is an ideal tool for completing massive flood simulation scenarios (e.g., the multiple random building collapse scenarios used in this study) within an acceptable time frame.

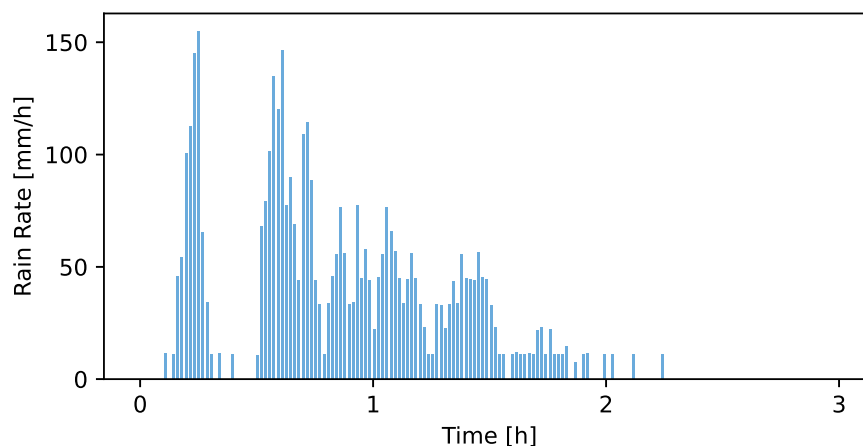
To build the hydrodynamic model for the Huangpu district, data on the DEM, surface roughness and boundary conditions are required. Flood simulation results are sensitive to the DEM (Xu et al., 2021). The 30m resolution DEM of the study area is freely available online, but this resolution is too coarse to resolve individual buildings and road sections, which have significant influence on the flood propagation pathways. Thus, the DEM is downscaled onto 5m grids, resulting in 1.36 million grid cells. It should be noted that DEM downscaling does not improve the accuracy of the DEM itself, but allows for better delineations of the building outlines. The building shape and height data are integrated into the DEM, replacing the original coarse-resolution building pixels. The final DEM can be visualized in Fig. 2.

The road layout data is integrated into the land use file, which provides spatially-distributed surface roughness for computing the bottom drag. In this work, the land use of the study area is simplified to three types: buildings, roads and others. They are assigned with a Manning's roughness coefficient of  $0.012s/m^{1/3}$ ,  $0.016s/m^{1/3}$  and  $0.03s/m^{1/3}$  respectively. Arguably, 5m grid resolution does not resolve the road shoulders. Existing approaches such as the artificial porosity model and the elevated edge model could identify the subgrid-scale road shoulders (e.g. Guinot et al., 2018; Hodges, 2015; Li and Hodges, 2019), but



**Figure 2.** The DEM of the study area. The red markers indicate the 17 metro stations in the study area. The dark blue arrows are the hypothetical break points of the flood wall. For better visualizations, buildings are assigned with a uniform elevation of 29m on this figure, but their original elevation is used in the simulations.

110 they are not adopted herein because the focus of this manuscript is on comparing the pre- and post-collapse scenarios. Indeed, the influence of the road shoulders on flood propagation does not depend on the building collapse status. Building drag force



**Figure 3.** The rainfall data recorded at the Tongji University main campus between 2:30pm and 5:30pm, July 21th of 2023.

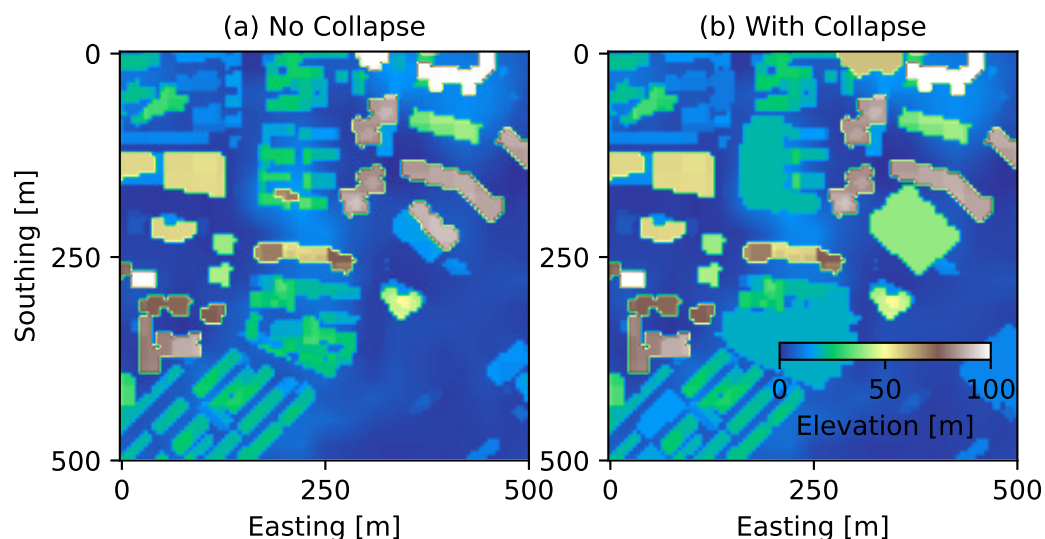
is neglected in the simulations. Rainfall is allowed to fall on the building roof, then flow to its neighboring cells following the topographical gradient. Detailed roof storage or drainage devices are not considered due to the lack of data. Soil infiltration and drainage through the stormwater pipelines are both ignored. Admittedly, this would overestimate the flood inundation depth. However, under extreme flood events, the soil and the drainage pipes are likely saturated, meaning that they have minor influence on surface hydrodynamics (e.g., Wilfong et al., 2024). More detailed discussions on the roles of infiltration and drainage can be found in Section 4. The computational domain is initially dry. The west boundary of the domain is open and water is allowed to exit the domain freely. All other boundaries are closed to represent fully functioned flood walls along the rivers.

The flood simulations conducted in this study encompass two distinct scenarios: fluvial and pluvial flooding events. To model fluvial flood, assume three locations on the flood wall are breached (e.g., due to earthquake) and a constant water depth of 3m is enforced at the three breakpoints (marked on Fig. 2). To model pluvial flood, assume the rainfall rate equals the recorded precipitation on July 21th of 2023 (Fig. 3). Both scenarios are modeled for 3 hours. The inundation extent, depth, particularly the depth at the 17 metro stations, will be monitored and analyzed.

### 2.3 Building Collapse Model

The earthquake risk of Shanghai is generally low. Historically, Shanghai rarely experiences an earthquake with a magnitude greater than VII. However, the large population density, tunnel constructions in the soft soil, massive old civil houses as well as more than 150 skyscrapers (i.e., buildings taller than 150m) make Shanghai vulnerable to potential earthquake attacks. It is estimated that in the Nanjing Road (located in Huangpu district) where plenty of old houses are made of brick and timber, 7.2% of buildings will be seriously damaged under an intensity VI earthquake, and 24.8% of buildings will be seriously damaged under an intensity VII earthquake (Cole et al., 2008).





**Figure 4.** A comparison between (a) the original DEM and (b) the DEM with a few buildings collapsed.

In the present study, earthquake-induced building collapse is not explicitly modeled. Instead, we generate  $n$  realizations with  $r\%$  randomly selected buildings collapsed. From modeling perspective, this is equivalent to perform hydrodynamic simulations with  $n$  different DEM and roughness inputs representing various building damage scenarios. Herein,  $n = 100$  and  $r = 10, 20$  or 135 40 respectively. By analyzing hydrodynamic simulation results from 100 possible building collapse scenarios, the inundation patterns of the metro stations can be statistically characterized. Then, typical building collapse and metro station inundation scenarios will be examined in more detail, revealing deterministic flood propagation mechanisms underlying the statistical observations.

Similar to Takabatake et al. (2022), building collapse is modeled by distributing 50% of the original building volume, 140 estimated from the building footprint area and height data, to its surroundings. This results in shorter structures with a broader spatial occupancy after collapse (Fig. 4). Although it is an idealized representation that neglects factors such as building strength and seismic force magnitude, this approach effectively characterizes the obstruction and alteration of flood propagation paths by collapsed buildings, which is the primary focus of this study. The simplified modeling of post-collapse building geometry captures the essential effects on flood dynamics without introducing unnecessary complexities.

### 145 3 Results

Before illustrating the simulation results, it is necessary to define a few quantitative metrics that aid the analysis. Herein, three metrics are defined: the average deviation of water depth ( $\epsilon_{\text{avg}}$ ), the percent of flooded scenarios ( $p_{\text{flooded}}$ ), and the percent of the aggravated scenarios ( $p_{\text{agg}}$ , see Eq. 1-3).



$$\epsilon_{\text{avg}} = \frac{\sum_{i=1}^n \left[ \overline{h_i(t)} - h_{\text{ref}}(t) \right]}{n} \quad (1)$$

$$150 \quad p_{\text{flooded}} = \frac{n_{\text{flooded}}}{n} \quad (2)$$

$$p_{\text{agg}} = \frac{n_{\text{agg}}}{n_{\text{flooded}}} \quad (3)$$

In Eq. (1)-(3),  $h_{\text{ref}}(t)$  is the simulated depth for the reference scenario (i.e., without building collapse) at a given metro station,  $h_i(t)$  is the simulated depth of the  $i$ th random building collapse scenario, the overbar indicates time averaging over the 3-hour simulation period,  $n = 100$  is the total number of testing simulations,  $n_{\text{flooded}}$  is the total number of testing scenarios in which a given station is flooded (i.e., the maximum water depth exceeds 1cm) and  $n_{\text{agg}}$  is the number of testing scenarios with greater flood depth than the reference scenario at the given station.

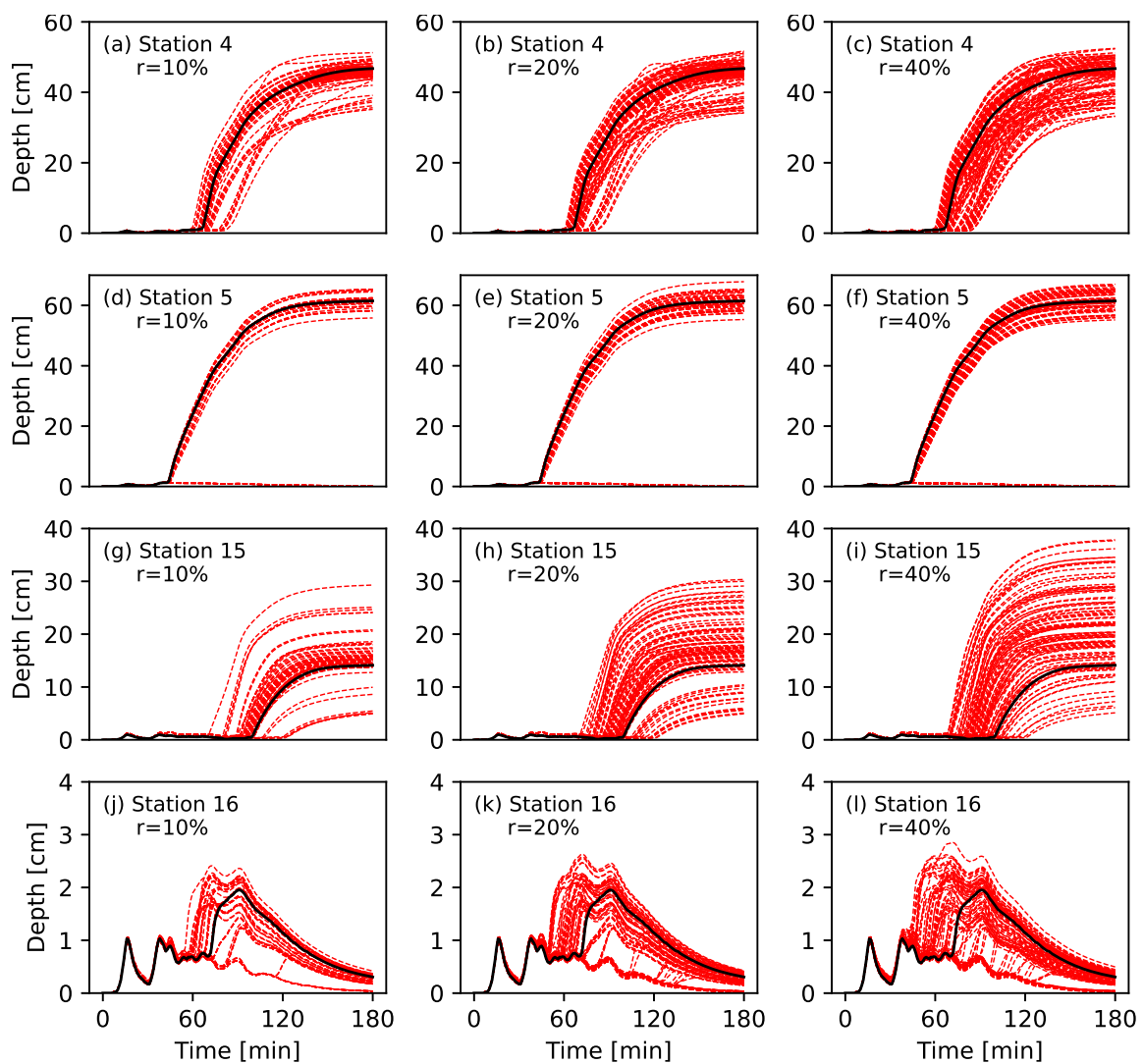
### 3.1 Pluvial Flood

Figure 5 shows the inundation depth at metro stations 4, 5, 15 and 16 under pluvial flood scenarios. The thick black curve represents the reference scenario without building collapse. The red curves are the 100 random building collapse scenarios. Other stations are either not flooded, or the water is too shallow to cause any catastrophic impact. It can be seen that building collapse has influence on the inundation depth at all four stations displayed. For each station, different building collapse scenarios could either increase or decrease the inundation depth compared with the reference scenario. Station 4, 5 and 15 show similar inundation patterns, where the water depth is small initially, then surges to tens of centimeters. It indicates that these stations are located in low-lying areas that receive water from their surroundings. Station 16 is different in that the water depth declines when rainfall weakens after about 90 minutes, meaning that station 16 both distributes water to its surroundings, and receives water from its surroundings. The latter is evidenced by the different depth evolution patterns with respect to different building collapse scenarios.

Interestingly, Fig. 5 shows that, at station 5, despite most building collapse scenarios exhibit an increasing water depth to about 60cm, a few scenarios are almost not flooded (characterized by the near-zero water depth). Detailed examination (not shown) reveals that station 5 is close to a building. When this building is randomly chosen to collapse, station 5 is completely buried and is unaffected by flood. This situation is not impossible in the real world because there exists metro station entrances inside the buildings in Shanghai. When such buildings collapse to bury the entrance, although flood water is more difficult to invade the station, passengers are more difficult to evacuate too. It implies that for a complete analysis of multi-hazard risk of the metro stations, the locations of each entrance should also be considered separately, but this is beyond the scope of the present study.

As more buildings are collapsed ( $r$  increases), the envelopes of the inundation depths expand, implying that more and more building collapse scenarios diverge from the reference scenario. It can be seen from Table 1 that  $\epsilon_{\text{avg}}$  increases with  $r$  at all three stations, indicating that the change of building layout (due to building collapse) has strong impact on the flood water propagation. The same conclusion was also mentioned in Bruwier et al. (2020). At all three stations displayed and with all  $r$





**Figure 5.** Water depth evolution at metro station 4, 5, 15 and 16 for the pluvial flood scenarios. The black line is the original DEM without building collapse. The red lines are the 100 random realizations with buildings collapsed.



180 values tested, 100% of the testing scenarios are flooded. That is, although building collapse affects the magnitude of flooding at metro stations, it does not change the inundation status. This is in contrast to the case of fluvial flood, which will be discussed later in Section 3.2 The  $p_{agg}$  values exhibit different behaviors as  $r$  increases at the three stations. For station 4, building collapse generally alleviate the flood risk as  $p_{agg}$  remains less than 50%. Station 15 shows the opposite behavior (with  $p_{agg} \geq 80\%$ ), where building collapse mostly enhance flood risk. For station 5,  $p_{agg}$  decreases from 67% to 26% as  $r$  increases. Clearly, 185 it is difficult to evaluate the increased/decreased flood risk from an ensemble point of view, as it is the specific buildings that collapse (which are different for different scenarios) determine the subsequent flood propagation and water accumulation patterns.

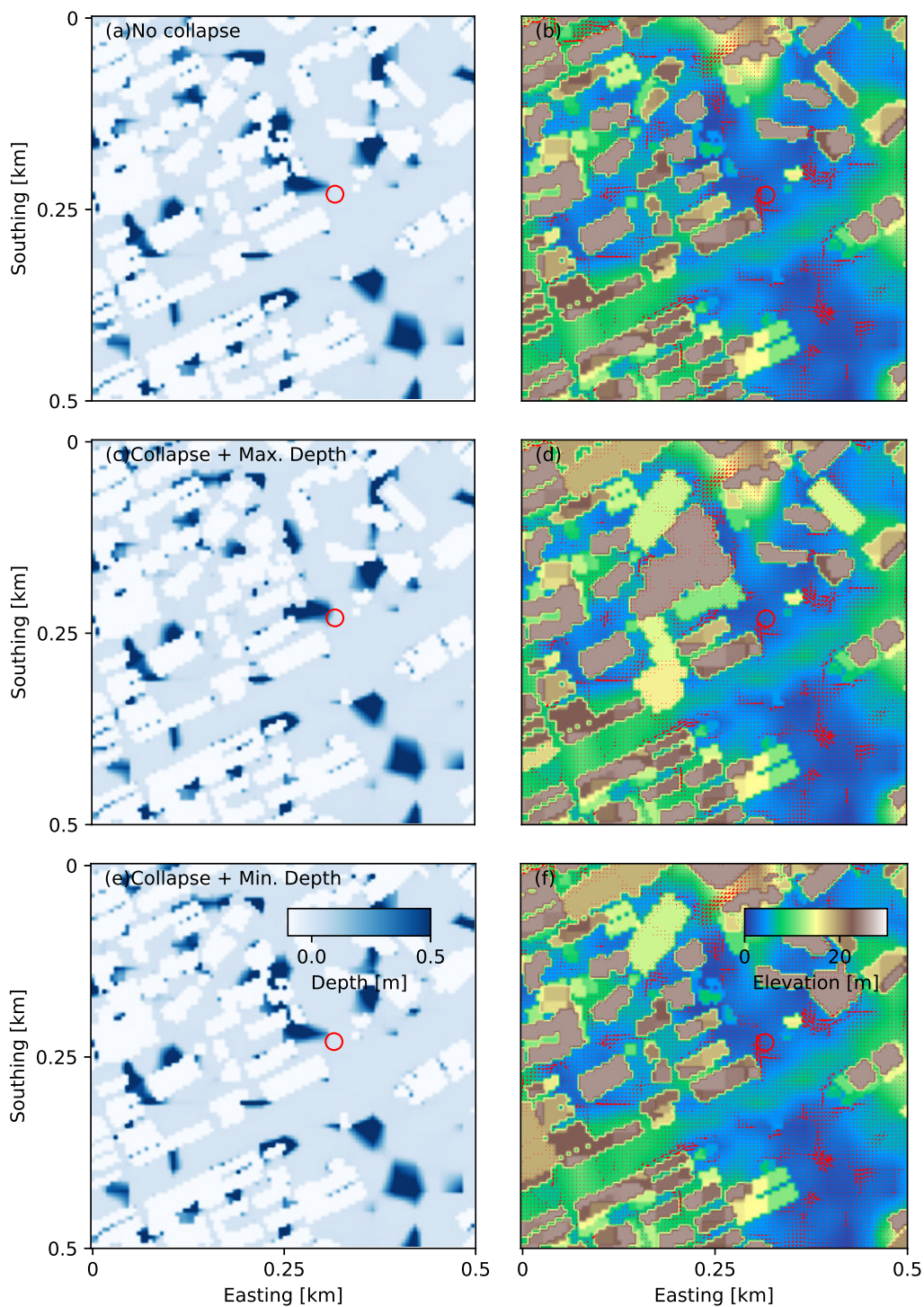
**Table 1.** The mean deviation from the reference scenario, the proportion of scenarios being flooded, and the proportion of scenarios with aggravated flood disaster (i.e., greater depth than the reference) among the 100 random building collapse and pluvial flood scenarios.

	Station 4			Station 5			Station 15		
	$r = 10\%$	$r = 20\%$	$r = 40\%$	$r = 10\%$	$r = 20\%$	$r = 40\%$	$r = 10\%$	$r = 20\%$	$r = 40\%$
$\epsilon_{avg}$ [cm]	1.43	2.54	3.42	4.16	9.35	13.6	1.22	2.99	4.95
$p_{flooded}$	100%								
$p_{agg}$	45%	32%	36%	67%	37%	26%	85%	80%	85%

To further uncover the mechanisms how building collapse affect inundation, three scenarios are selected and analyzed in detail, which are the reference scenario (no building collapse) and the scenarios with the largest and smallest inundation depth 190 (30cm and 5cm respectively) at metro station 15 ( $r = 20\%$ ). As can be seen from Fig. 6, with pluvial flood, water accumulates in local topographic depressions, resulting in discrete inundated areas across the study region. In all three scenarios, an inundation patch is present in the vicinity of station 15, posing a potential flood risk to this metro station. However, Fig. 6(d) sees buildings collapsed on the northwest of station 15, leading to less depression area that stores water (comparing Fig. 6c with Fig. 6a) and reduced road width between buildings that enhances flow speed (comparing Fig. 6d with Fig. 6b). Both factors facilitate 195 flood propagation towards station 15. On the contrary, in Fig. 6(f), building collapse occurs away from station 15, which has minor influence on the inundation of station 15 because pluvial flood is strongly affected by the local topography. Clearly, the influence of building collapse on pluvial flood propagation is highly localized. Flooding of a metro station is not expected to aggravate if the buildings near the station do not collapse. This finding also explains why  $p_{flooded}$  stays at 100% under all circumstances (Table 1), as the localized influence of building collapse can hardly affect the city-wide occurrence of pluvial 200 flood.

### 3.2 Fluvial Flood

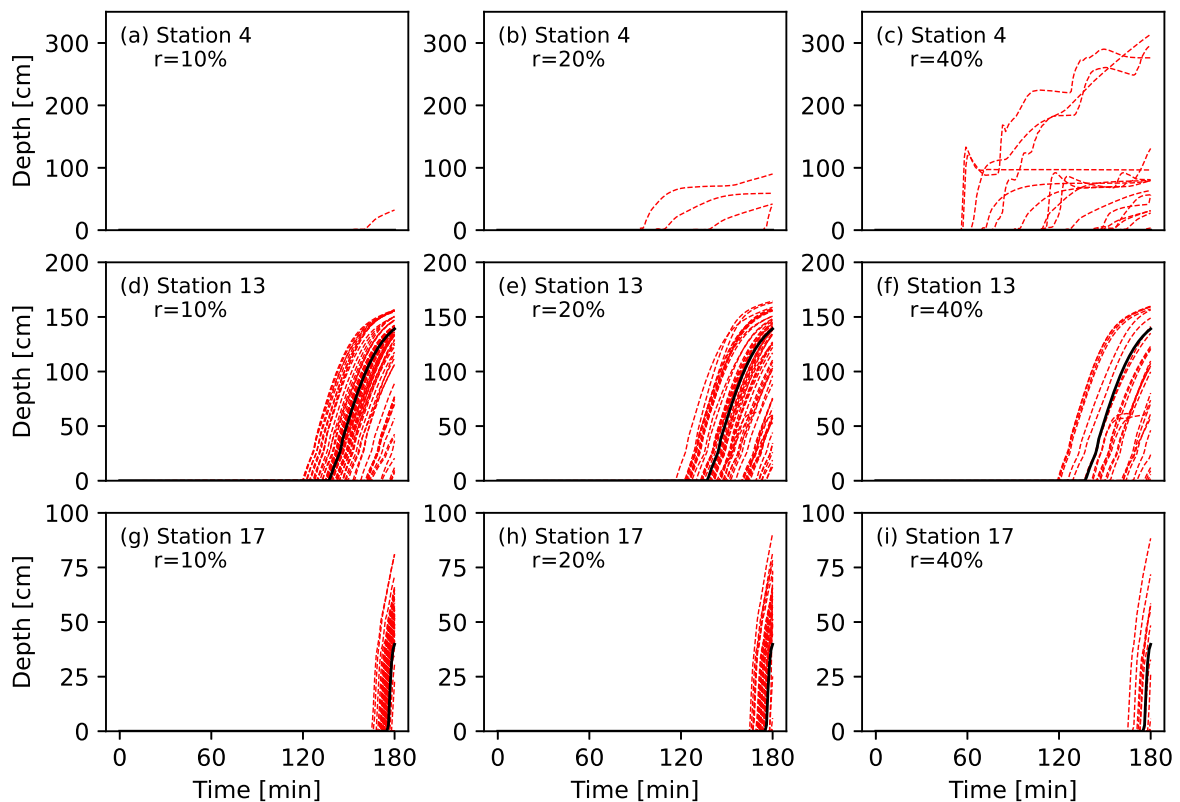
Figure 7 shows the inundation depth at metro stations 4, 13 and 17 under fluvial flood scenarios. Other stations are either not flooded, or the depth is too small (e.g., less than 1cm). This is expected because the three stations displayed are close to the break points of the flood wall (Fig. 2). Station 4 is not flooded in the reference scenario, and hardly flooded when  $r = 10\%$



**Figure 6.** Simulated water depth (left column) and velocity (right column, on top of DEM) at 1 hour for selected pluvial flood scenarios near metro station 15 ( $r=20\%$ ). The top row is the reference scenario without building collapse (the black curve in Fig. 5h). The middle row is associated with the building collapse scenario with the largest inundation depth at station 15, whereas the bottom row represents the scenario with the smallest inundation depth. The red circle is the location of station 15.



205 and 20%. Only a few realizations exhibit non-negligible inundation depth (Fig. 7b). When  $r$  is increased to 40%, 17 out of  
the 100 random realizations are flooded (Table 2). Clearly, building collapse enhances the flood exposure of station 4 and  
the exposure exacerbate as  $r$  increases. Station 13 exhibits the opposite behavior, where a large  $r$  reduces flood exposure. At  
 $r = 40\%$  (Fig. 5f), only 28 out of the 100 scenarios show positive flood depth. With  $r = 10\%$  and 20%, however, these numbers  
are 78 and 62 respectively (Table 2). A similar trend is observed at station 17, where a large  $r$  reduces flood exposure (only 10  
210 scenarios are flooded at  $r = 40\%$ ).



**Figure 7.** Water depth evolution at metro station 4, 13 and 17 for the fluvial flood scenarios. The black line is the original DEM without building collapse. The red lines are the 100 random realizations with buildings collapsed.

A notable difference between fluvial and pluvial flood is that, unlike pluvial flood scenarios where  $p_{\text{flooded}}$  always reach 100%, for fluvial flood, both water depth and  $p_{\text{flooded}}$  exhibit variability between different stations, different  $r$  values, and different random realizations (Table 2). In terms of the mean deviation ( $\epsilon_{\text{avg}}$ ), the trends of  $\epsilon_{\text{avg}}$  and  $p_{\text{flooded}}$  are the same for station 4 and 17. With more testing scenarios flooded, the mean deviation increases. However, this trend is reversed at station  
215 13, where the lowest  $p_{\text{flooded}}$  ( $p_{\text{flooded}} = 28\%$  at  $r = 40\%$ ) is accompanied with the largest deviation ( $\epsilon_{\text{avg}} = 22.29\text{cm}$ ). Again, these results illustrate the complex interactions between flood propagation and the collapsed buildings.



**Table 2.** The mean deviation from the reference scenario, the proportion of scenarios being flooded, and the proportion of scenarios with aggravated flood disaster (i.e., greater depth than the reference) among the 100 random building collapse and fluvial flood scenarios.

	Station 4			Station 13			Station 17		
	$r = 10\%$	$r = 20\%$	$r = 40\%$	$r = 10\%$	$r = 20\%$	$r = 40\%$	$r = 10\%$	$r = 20\%$	$r = 40\%$
$\epsilon_{\text{avg}}$ [cm]	2.30	4.32	10.02	13.72	20.02	22.29	2.11	1.60	1.36
$p_{\text{flooded}}$	1%	4%	17%	78%	62%	28%	61%	33%	10%
$p_{\text{agg}}$		100%		46%	37%	22%	92%	88%	70%

Figure 8 shows three selected scenarios to better understand the flooding process of station 13. At the end of the simulation, the water depth at station 13 reaches about 140cm for the reference scenario. As can be seen from Fig. 8(b), the flood water originates from break point B (Fig. 2). The intruding water travel southward following the terrain slope. Then it change direction  
 220 towards the west when encountering elevated terrains, and turn back to south again when hitting buildings. After penetrating through a few building blocks, the river water finally reach station 13. Figure 8(c) and (d) display the scenario with the deepest water depth at station 13. An obvious distinction is that some buildings along the Huangpu River are collapsed. The collapsed buildings squeeze the roads between them, which serve as a primary path of the intruding water. As a result, the flow velocity is higher and station 13 is flooded with greater depth. Figure 8(e) and (f) show the scenario with the lowest inundation depth  
 225 at station 13. Although the flow path is squeezed by the collapsed buildings as well, additional buildings are collapsed next to station 13 in the northeast, which blocks the original southward flow path penetrating these buildings. As a result, the flooding of station 13 is weakened and postponed.

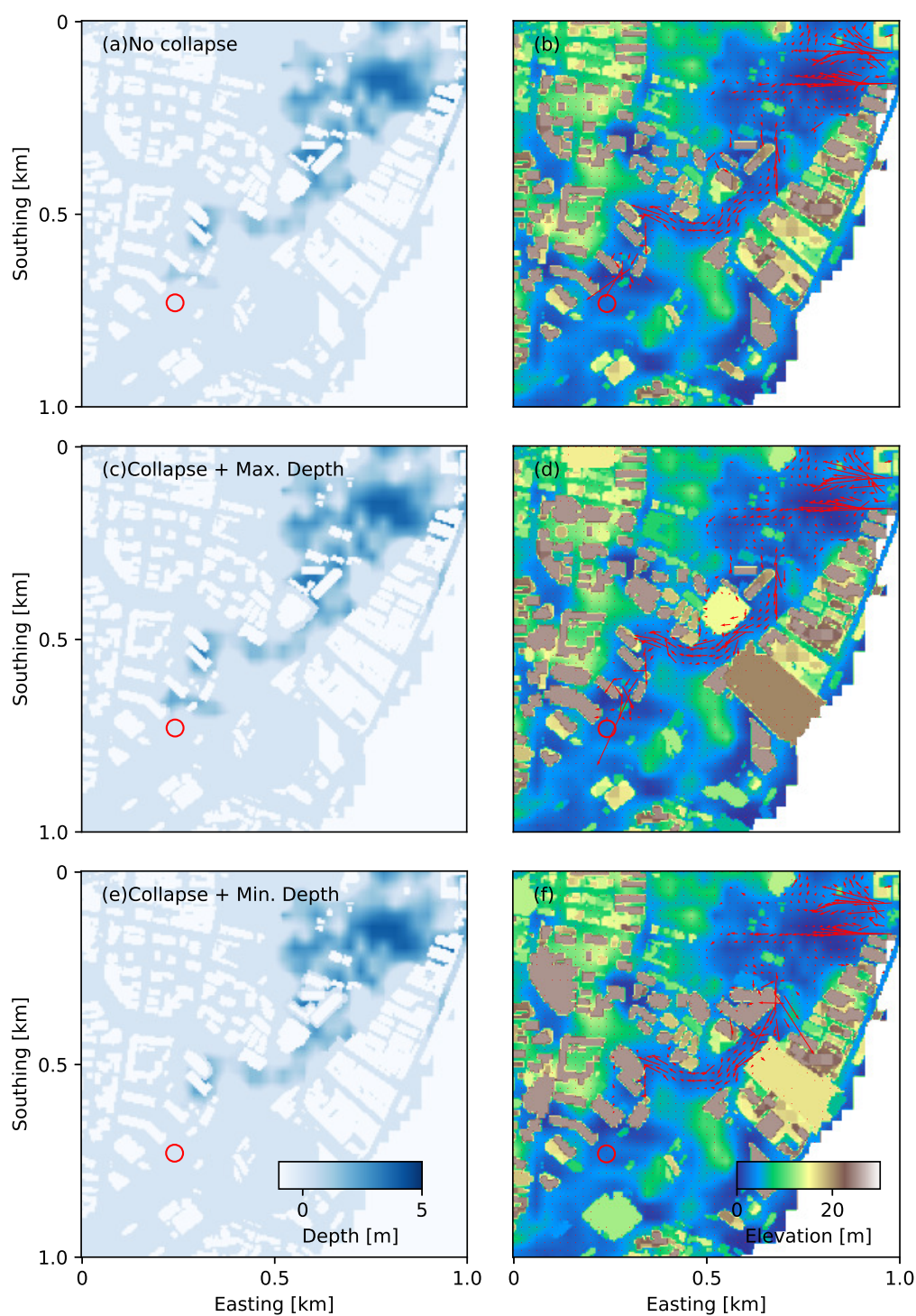
## 4 Discussions

### 4.1 Pluvial vs. Fluvial

230 From the results shown in Section 3, pluvial and fluvial floods exhibit distinct behaviors in the event of building collapses. Pluvial flood occur in low-lying areas all around the study area. Building collapse do not alter the flood status ( $p_{\text{flooded}}$  always equal 100%) of a given location (i.e., a metro station), but greater variance of the inundation depth is detected as more buildings collapse. Fluvial flood occur close to the rivers. Its occurrence is sensitive to building collapse. As  $r$  increases, the total number of flooded scenarios could either increase (station 4) or decrease (station 13 and 17).

235 The distinctions between pluvial and fluvial floods arise from their different origination, accumulation, and propagation patterns. Pluvial floods originate from heavy rainfall, which is spatially distributed. Rainwater accumulates in local topographic depressions, leading to massive short travel paths, which can be visualized from the velocity field of Fig. 6. Most flow paths remain unaffected by building collapse unless the collapsed building is in close proximity to the depression. As  $r$  increases, there is a greater probability that a chosen location is next to the collapsed building, which explains why more scenarios diverge  
 240 from the reference at higher  $r$ .





**Figure 8.** Simulated water depth (left column) and velocity (right column) at 2 hours for selected fluvial flood scenarios near metro station 13 ( $r=20\%$ ). The top row is the reference scenario without building collapse (the black curve in Fig. 7e). The middle row is associated with the building collapse scenario with the largest inundation depth at station 13, whereas the bottom row represents the scenario with the smallest inundation depth.





On the contrary, fluvial flood originate from point sources (i.e., the break points along the river bank), forming only a few travel paths from the source. However, since the invading river water is driven by strong pressure gradient, it travels through longer distances. The long travel path implies a greater probability for the flow to be interfered by the collapsed building, and the very few paths means that once it is interfered, the subsequent flood propagation will be dramatically influenced. Thus, a large  $r$  does not necessarily affect the inundation of a metro station, but when it does, the inundation statues of the station could be completely altered.

## 4.2 Global vs. Local

Hydrodynamic simulation is the predominant method for urban flood studies as it replicates and forecasts the spatiotemporal evolution of inundation depth and flow fields. This capability allows for a detailed examination of the complete flooding process at finer spatial and temporal resolutions. However, this advantage is not always fully exploited as many urban flood studies focus on the “global” flood variables and the “global” influencing factors. The former are the variables that characterize flood of the entire study area (e.g., inundation extent, flood volume, mean depth and outflow discharge), and the latter are the factors that are applied to the entire study area (e.g., rainfall intensity, building coverage, drainage capacity) (e.g. Bermúdez et al., 2018; Bruwier et al., 2020; David and Schmalz, 2020).

Buildings in megacities often exhibit strong spatial heterogeneity. Taking Shanghai as an example, historical buildings from the early 20th century, civil apartments from the post-World War II era, and some of the world’s tallest skyscrapers coexist. These structures differ in age, height, construction, materials, usage, and earthquake resistance. Consequently, they are expected to react differently when subjected to seismic activity, indicating that earthquake-induced building collapse could be localized. Similarly, urban flood vulnerability is also localized. Metro stations are inherently more vulnerable than most other urban infrastructures. Thus, acknowledging that the collapse of a building at location A can hardly affect the flooding of location B that is kilometers away, this study adopts a local-to-local approach when analyzing results. For example, the water depth at each station is examined together with detailed building collapse patterns nearby. If we only look at global variables, for example, the inundation extent shown in Fig. 8(a), (c) and (e), minimal differences are visible between different scenarios.

Herein, we want to emphasize the importance of performing high-resolution flood simulations with HPC-enabled hydrodynamic models, which allows investigating key localized flood processes. This is particularly important in terms of risk assessment and mitigation because it is often the local spots (metro stations, bridge tunnels, narrow channels, etc.) that suffer the most severe flood disasters (e.g. Hénonin et al., 2015; Vermeij, 2016).

## 4.3 Limitations

With SERGHEI-SWE, the present study models the flooding of the central district of Shanghai under extreme rainfall and levee breaching events. Urban flood process also involves infiltration into the subsurface, and drainage through the stormwater pipes (e.g. Bermúdez et al., 2018; Cardoso et al., 2020; Hossain Anni et al., 2020; Schubert et al., 2022b). These two processes are neglected in this work due to the lack of data on the geological conditions, the water table and the pipe layout of the study area. Indeed, data scarcity is a major challenge when incorporating underground infrastructures with surface hydrodynamic



simulations. Although empirical methods are available to approximate the amount of soil infiltration and pipe drainage (e.g. 275 Bruwier et al., 2020; Xu et al., 2023), they are not applied herein because Shanghai as a coastal city features very shallow water table (in the range of 1-2m). Under extreme flooding conditions the soil could reach saturation rapidly. The pipes could reach their full capacity too. Figure 5 demonstrates that the maximum inundation depth of the July 21th flood ranges between 30 to 60cm among the metro stations, which is higher than the reported values in the media (around 25cm), but within a reasonable range considering the absence of drainage pipes. We argue that the mechanism how building collapse affect flood 280 propagation, which is the focus of this work, is not sensitive to soil infiltration or pipe drainage. As part of the ongoing work, SERGHEI-SWE is being developed to integrate a variably-saturated groundwater solver, and a stormwater drainage model. The fully coupled surface-subsurface-pipe flow model should be able to provided a physically more complete picture of the urban flooding process in the future.

In this work, building collapse is simplified as changes in the building geometry, and earthquake vulnerability (i.e., which 285 building will collapse during an earthquake) is assigned randomly. This method treats earthquake and building strength as “global” variables, which is not aligned with our local-to-local strategy (Section 4.2). Indeed, more advanced approaches inevitably require detailed information on the buildings in the study area, including the building height, age, usage, material and so on (Xin et al., 2021). We argue that such information are not necessary for the present study because we put more attention on the flood propagation mechanisms and processes. However, in future multi-hazard studies, these data should be obtained 290 and integrated with advanced building collapse models to provide more realistic, city-wide building collapse estimations.

In the present study, metro stations are simplified to single pixels on the DEM. However, real metro stations in Shanghai often contain multiple exits spanning several road blocks. Consequently, the flooding status and flood resistance at each exit could vary. Although further refining the grid resolution to resolve the local topography at each metro exit remains challenging, a multi-scale approach might be feasible in the future. For example, based on the hydrodynamic simulation results at relatively 295 coarse grid resolutions (e.g., the 5m resolution used herein), a finer resolution simulation could be performed in a smaller region near the metro station. This would characterize the detailed flooding processes at each exit. The multi-scale modeling approach, coupled with a physics-based building collapse model, would enable a true local-to-local analysis and evaluation of the multi-hazard risk posed to metro stations.

## 5 Conclusions

300 In this study, GPU-accelerated hydrodynamic simulation is employed to simulate pluvial and fluvial flood propagation before and after earthquake-induced building collapse, aiming at understanding the mechanism how building collapse influences flood inundation of urban metro stations. Taking Huangpu district in central Shanghai as the study area, the following conclusion are drawn from the simulation results:

### 1. Pluvial flood

305 Pluvial flood occurs over a broad spatial extent. The travel paths of the flood water are relatively short due to the widely distributed local topographic depressions that serve as natural drainage points. Thus, only the buildings collapsed near



the metro station can affect its inundation by interfering the relevant propagation paths of the water that flood the station. It also follows that the impact of building collapse on metro station flooding is positively correlated to the proportion of buildings that have collapsed.

## 310 2. Fluvial flood

Fluvial floods originate from point sources. The propagation of fluvial flood is characterized by long and consistent trajectories spreading from the source and following the topographic gradient. Building collapse only affect fluvial flood when the collapsed buildings are located close to the propagation path of the flood water. However, since these paths are generally longer than those for pluvial floods, the inundation of a metro station could be influenced by buildings collapsed some distance away. Moreover, an increasing proportion of building collapse does not necessarily enhance the flood risk because there is a large probability that the collapsed buildings are situated far from the flood source or its travel path. The key factor is the spatial relationship between the flood propagation path, the location of building failures and the location of the hazard-bearing body of interest (e.g., a metro station).

This study provides novel insights into the complex compound mechanism involving earthquake-induced building collapse and subsequent flooding under a multi-hazard context. While the findings contribute to advancing multi-hazard evolution process and risk analysis, it is important to acknowledge that the explicit modeling of earthquake processes and their physical impacts on individual buildings was beyond the scope of this work. Future research efforts should focus on developing comprehensive physical-process-based multi-hazard simulators capable of capturing the intricate dynamics of such compound events at a finer scale.

### 325 *Code and data availability.*

The SERGHEI-SWE model is an open-sourced software (Caviedes-Voullième et al., 2023). The topography, building and road layout data of the study area are all publicly available. The rainfall data is measured by the authors and is available upon request.

### *Author contributions.*

330 ZL conceptualized the research aim and scope; ZZ prepared the data; ZL performed the numerical simulation; ZL and HL analyzed the results; ZL and HL wrote the manuscript draft; CD and SJ reviewed and edited the manuscript.

### *Competing interests.*

The authors declare no competing interests.

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