



Accounting for Uncertainties in Forecasting Tropical Cyclone-Induced Compound Flooding

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- Abstract. Tropical cyclone impacts can have devastating effects on the population, infrastructure, and on natural habitats. 13 14 However, predicting these impacts is difficult due to the inherent uncertainties in the storm track and intensity. In addition, 15 due to computational constraints, both the relevant ocean physics and the uncertainties in meteorological forcing are only 16 partly accounted for. This paper presents a new method, called the Tropical Cyclone Forecasting Framework (TC-FF), to 17 probabilistically forecast compound flooding induced by tropical cyclones, considering uncertainties in track, forward speed, 18 and wind speed/intensity. The open-source method accounts for all major relevant physical drivers, including tide, surge, and 19 rainfall, and considers TC uncertainties through Gaussian error distributions and autoregressive techniques. The tool creates 20 temporally and spatially varying wind fields to force a computationally efficient compound flood model, allowing for the 21 computation of probabilistic wind and flood hazard maps for any oceanic basin in the world, as it does not require detailed 22 information on the distribution of historical errors. A comparison of TC-FF and JTWC operational ensembles, both based on 23 DeMaria et al. (2009), revealed minor differences of <10%, suggesting that TC-FF can be employed as an alternative, for 24 example, in data-scarce environments. The method was applied to Cyclone Idai in Mozambique. The underlying physical model showed reliable skill in terms of tidal propagation, reproducing the storm surge generation during landfall and flooding 25 near the city of Beira (success index of 0.59). The method was successfully applied to forecast the impact of Idai with different 26 27 lead times. The case study analyzed needed at least 200 ensemble members to get reliable water levels and flood results three 28 days before landfall (<1% flood probability error and <20 cm sampling errors). Results showed the sensitivity of forecasting, 29 especially with increasing lead times, highlighting the importance of accounting for cyclone variability in decision-making
- 30 31

and risk management.





32 1 Introduction

Tropical cyclone (TC) induced compound flooding, which occurs when storm surge, heavy rainfall, high tide, and river discharge coincide, can have devastating impacts on coastal communities (Wahl et al., 2015). This type of flooding is particularly concerning as it can result in higher water levels and increased inland flooding, leading to damage and loss of life (e.g., Resio & Irish, 2015). The increased frequency and severity of compound flooding events are expected to worsen due to the effects of climate change (e.g., Lin et al. 2012; Mori and Shimura, 2023) as well as on-going coastal development and population growth. Mitigation and preparedness strategies require a sound toolbox for assessing TC-induced compound flooding on coastal communities that enhance short to long-term decision-making.

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Operational and strategic risk analyses are instrumental in analyzing and mitigating potential environmental risks. Operational risk analysis, typically associated with short-term forecasting (~several days), provides immediate response and preparedness for imminent disasters, ensuring the safety and protection of people and property (Roy & Kovordányi, 2012). Conversely, strategic risk analysis focuses on long-term climate variability assessments, delivering insights into hazards and their socioeconomic and environmental impacts, thus facilitating informed policy decisions and adaptation strategies (e.g., Nederhoff et al., 2021). Though distinctly different, both perspectives are critical for comprehensive climate risk management, as they offer different scales and timeframes for prevention, preparedness, response, and recovery.

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49 Forecasting agencies such as the National Hurricane Center (NHC) have significantly improved operational meteorological 50 risk analysis, credited to gains made in numerical weather prediction models (McAdie et al., 2000, Cangialosi et al., 2020). 51 Despite advancements, operational forecast errors remain significant enough to necessitate considering the inherent 52 uncertainties in these forecasts for informed preparedness decision-making (Lamers et al., 2023). A common probabilistic 53 approach is to represent the resulting uncertainty in track prediction by a cone envelope as a graphical representation that 54 illustrates the possible track variation of the TC center (NHC, 2023). The shape of the cone can be derived from the historical error data of the forecast and typically represents a 66.7% probability that the track will be within the cone (i.e., 33.3% chance 55 56 the track falls outside the cone). The cone increases in size with lead time as the errors in the prediction accumulate. While the 57 cone gives valuable insight into the potential range of TC variability of the core, it can be easily misinterpreted as the 58 corresponding impacted area, which can be substantially larger. Quantification of the uncertainty in track prediction can be 59 computed with several methods. For example, De Maria et al. (2009) introduced a Monte Carlo method to generate 1,000 60 realizations by randomly sampling from historical error distribution functions from the past 5 yr for both the track and intensity. 61 De Maria et al. (2013) improved their method so that the track uncertainty is estimated on a case-by-case basis using the Goerss 62 predicted consensus error (GPCE; Goerss, 2007), where the uncertainty is estimated based on the spread of a dynamical model 63 ensemble instead of historical averages. Other methods exist. For example, Chen et al. (2023) introduced a deep-learning 64 ensemble approach for predicting tropical cyclone rapid intensification. However, these methods were all derived to provide





insight, before landfall, into the uncertainty of the wind speeds and not designed to force hydrodynamic or wave models, and
 can thus result in too erratic forcing conditions.

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Early Warning Systems (EWS) for coastal compound flooding are sensitive to uncertainties in the TC, including nonlinear interactions between the TC size, forward speed, location of landfall, tides, rainfall, and infiltration. However, often EWS for coastal flooding use physics-based and, due to computational constraints, deterministic approaches in which the best track is used to force a hydrological & hydrodynamic model that computes the storm surge and the complex interactions between coastal, fluvial, and pluvial processes (e.g., Global Storm Surge Information System; GLOSSIS based on Delft3D Flexible Mesh; Kernkamp et al., 2011 and Coastal Emergency Risks Assessment; CERA based on ADCIRC; Luettich et al., 1992).

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75 Several examples of probabilistic coastal flood methods capture uncertainty in forcing. For example, the Global Flood Awareness System (GloFAS; Alfieri et al., 2013) is a modeling chain run by the European Centre for Medium-Range Weather 76 77 Forecasts (ECMWF) based on the LISFLOOD hydrological model forced by 51 ensemble members. While GloFAS is an 78 excellent resource for communities worldwide, it operates at a large scale with a relatively coarse resolution of 0.1 degrees 79 (~10 km), and is thus not designed explicitly for TCs that require high spatial resolutions (Roberts et al., 2020), and neither 80 account for relevant coastal processes such as tides. Higher resolution and the inclusion of coastal processes can be found in several regional applications. For example, the Stevens Flood Advisory System (SFAS; Ayyad et al., 2022) is an ensemble-81 82 based probabilistic forecasting of tide, surge, and riverine flow across the US Mid-Atlantic and Northeast coastline and runs 83 for 96 different atmospheric forcing datasets. Other examples include forecasting systems from the UK Met Office (Flowerdew et al., 2010) and the Royal Netherlands Meteorological Institute (de Vries, 2009). All these systems rely on coarser numerical 84 85 forecasting products, focus on mid-latitude regions, and are thus not explicitly designed to forecasts hazards related to TCs.

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87 Probabilistic modeling systems for TC-induced coastal flooding for operational risk analyses in the US and Japan include P-Surge (Taylor and Glahn, 2008; Gonzalez and Taylor, 2021), which uses data from the NHC to create a set of synthetic storms 88 89 by perturbing the storm's position, size, and intensity based on past errors of the advisories. Subsequently, the Sea, Lake, 90 Overland, Surge from Hurricanes model (SLOSH; Jelesnianski 1992) is run and forecasts storm surge in real-time when a 91 hurricane is threatening. However, SLOSH does not account for several relevant (coastal) processes (e.g., tides, waves, rainfall, 92 infiltration) and thus lack their interactions. The Japan Meteorological Agency (JMA) does use a dynamic tide and storm surge 93 model (Higaki et al., 2009) but only accounts for a limited number of 11 ensemble members (Hasegawa et al., 2015). Moreover, 94 both methods are created with a specific region in mind and are not easily transferable to other locations.

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Besides probabilistic physics-based techniques, statistical machine-learning techniques (e.g., Lecacheux et al., 2021 or Nguyen
& Chen, 2020) are becoming increasingly popular in reducing the computational expense of forecasting compound flooding.





98 However, these machine learning downscaling methods lack nonlinear interactions between relevant coastal processes driving 99 compound flooding. Hybrid methods focus on reducing the number of tracks simulated and proved capable of accurately 100 representing a larger set of scenarios (Bakker et al., 2022).

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As introduced by Suh et al. (2015), the constraints in real-time forecasting for operational risk analysis are around both 'accuracy' and 'promptness'. In other words, the time constraints associated with forecasting dictate that some modeling systems use a purely deterministic approach or a limited number of ensemble members to perform more detailed compound flooding predictions and thus simplify the meteorological uncertainty (e.g., GLOSSIS, CERA, JMA). On the other hand, probabilistic approaches for meteorology with a large number of ensemble members use simplified hydrodynamics or have an insufficient resolution for TCs and thus do not account for the processes needed to forecast TC-induced coastal compound flooding (e.g., GloFAS, SFAS, NHC).

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110 To address the limitations listed, we propose a method to generate probabilistic wind and compound flood hazard maps by using, for the first time, ensembling techniques via statistical emulation of TCs combined with physics-driven modeling for 111 coastal compound flooding. The workflow emulates the TC evolution using an autoregressive technique in combination with 112 113 reported mean errors in track and intensity, similar to DeMaria et al. (2009) but without the need for historical error distribution 114 functions. Next, this emulator produces an ensemble of several (herein thousands) TC members. Then, for each ensemble member, a time- and spatially-varying wind field is generated and used to force a computationally efficient compound flood 115 116 model SFINCS (Leijnse et al. 2021). The output consists of probabilistic wind and flood hazard maps that can be forecast on time with limited computational resources anywhere in the world. This paper refers to the TC forecasting framework as the 117 118 Tropical Cyclone Forecasting Framework, TC-FF.

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120 The paper is structured as follows. Section 2 introduces the Monte Carlo forecasting methodology. Section 3 describes the 121 case study site and historical event of interest. The materials and methods used in this paper are described in Section 4. 122 Validation in terms of tides and storms and application of the forecasting methodology are presented in Section 5. Finally, 123 Sections 6 and 7 discuss and summarize the main conclusions of the study.

124 2 Tropical Cyclone Forecasting Framework

In this paper, we introduce the probabilistic Tropical Cyclone Forecasting Framework, TC-FF, to compute TC-induced compound flooding for operational risk analysis. Our approach integrates a TC emulator using a Monte Carlo-based ensemble sampling generation with an autoregressive technique, which is a simplified adaptation of DeMaria et al. (2009). The ensemble members are generated around the forecasted official track, considering the average historical errors in intensity, cross-track, and along-track. Tthe ensemble members are provided as input for the fast compound flood model called SFINCS.





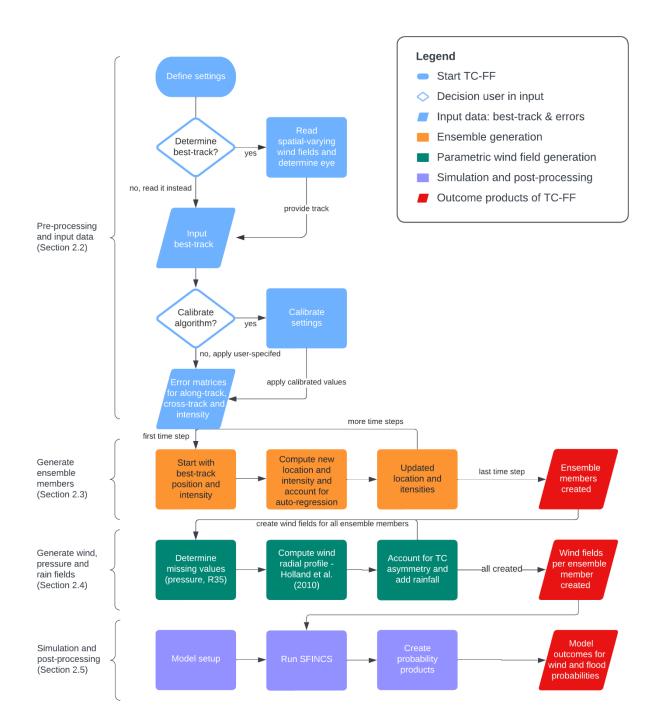
- 130 Additionally, TC-FF considers tidal movements, storm surge, precipitation, and infiltration. The outcomes are consolidated
- 131 into a unified probability product. By choice, each member has an equal likelihood of occurrence. The Python code for this
- 132 method is accessible on GitHub via the following link: <u>https://github.com/Deltares/CoastalHazardsToolkit</u>.

133 2.1 TC-FF flowchart

- A compact flowchart of TC-FF used to generate the ensemble member is shown in Figure 1. The steps of this process are asfollows:
- Define settings: The user specifies the data source, period, time step of the ensemble generation, and the number of
 ensemble members requested.
- Input best track: The code either determines the best track based on gridded time and spatial-varying wind and
 pressure fields (e.g., COAMPS-TC; Doyle et al., 2014) or reads in the forecasted track by one of the forecasting
 centers (e.g., NHC or other agencies).
- Error matrices for along-track, cross-track, and intensity: The tool first computes random realizations based on
 the along-track, cross-track, and intensity standard deviations imposed for the time steps requested. The imposed
 mean absolute error is scaled with the timestep to overcome any time step dependency.
- 4. Generate ensemble members: Following the approach of DeMaria et al. (2009), a Monte Carlo method generates
 numerous ensemble members based on error matrices of the previous step in combination with an autoregressive
 technique for the along-track, cross-track, and intensity error.
- 5. Generate wind, pressure, and rain fields: Generate meteorological forcing conditions, i.e., the surface wind and
 pressure fields per time step per ensemble member, based on parametric methods (e.g., Holland et al., 2010) for
 subsequent analysis and application within numerical models. Rainfall can be included as well via intensity
 relationships.
- 6. **Simulation and post-processing:** In this study, the compound flood model SFINCS is applied, but in principle, other hydrodynamic models can also be applied, albeit typically at a higher computational expense. Data from the different ensembles are combined into several probabilistic outputs ranging from the probability of gale force winds (wind speed > 35 knots or >18 m/s), compound flooding (water depth> 15 cm) to quantile estimates (e.g., 1% exceedance water level).
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- 158 Figure 1. Flowchart of the Tropical Cyclone Forecasting Framework (TC-FF). Pre-processing stages are represented in light blue, 159 the computational core of ensemble generation is denoted in orange, the parametric wind field generation is portrayed in green, the
- 160 hydrodynamic simulation and analysis of winds are marked in purple and outcomes in red.





161 In the subsequent paragraphs, we describe in more detail the pre-processing, the computation of the ensemble members (track 162 and intensity variations), and the determination of time- and spatially-varying wind fields.

163 2.2 Pre-processing and input data

164 The pre-processing of TC-FF comprises three components.

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First, one specifies the period they would like to simulate, including the total time period over which wind fields need to be generated and the time period over which the ensembles need to be generated. In addition, a timestep for ensemble generation (default 3 hours) needs to be specified. At this stage, one also specifies the mean absolute error and auto-regression coefficients for the along-track, cross-track, and intensity. When these values are unknown, calibration needs to be performed to determine them by comparing them with the reported errors of the forecast center (see calibration in Section 5.2.1). At this stage, one also specifies the number of ensemble members requested. The influence of the number of ensemble members is discussed in Section 5.3.2.

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Second, since TC-FF creates random realizations around the best track, an input track is needed. Depending on the application, TC-FF reads a forecast bulletin that generates the track or determine the best track from the output of a high-resolution regional meteorological model. The determination of a track from a meteorological model is based on an algorithm that finds the minimum pressure in an area of interest. It takes in grid values, u and v wind components, pressure, minimum distance for clustering, and returns lists of x and y coordinates of cyclone eyes, the maximum wind speed plus pressure around each eye.

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Third, before the generation of the ensemble members, TC-FF creates random errors with a normal distribution based on the provided average errors. Matrices are two-dimensional, with one dimension being the number of time stamps and the other the number of ensemble members. The imposed mean absolute error is scaled with the timestep to overcome any time step dependency and converted into a standard deviation.

184 2.3 Ensemble members

185 2.3.1 Track realizations and calibration

An important component in TC-FF is the generation of track realizations (or ensemble members) from the official track forecast. The official positions are interpolated with a spline function to include values at all requested times. Our approach for the track realization largely follows DeMaria et al. (2009). We decompose the track error into the along-track (AT) and cross-track (CT) components and account for the track error serial correlation via autoregressive regression (Equations 1 and 2).

$$AT_t = a_t AT_{t-i} + B_{rnd}$$
 Equation 1





	$CT_t = c_t CT_{t-i} + D_{rnd}$ Equation 2	
191	in which AT _t and CT _t are the AT and CT error at time steps t, a _t and c _t are constants, AT _{t-3} and CT _{t-3} are errors of the	e prev
192	time step (typically i=3 hours), and B and D are random numbers that are normal (Gaussian) distributed, scaled with	h the m
193	absolute error but are limited to $+/-2\sigma$.	
194		
195	Unlike DeMaria et al. (2009), we do not access the probability distributions of historical errors. Instead, we cal	librate
196	parameters (a ₁ , c ₁ , and mean absolute errors for B and D) based on the reported historical errors from the agency re	respons
197	for the issued forecast (see Section 5.2.1). This is a simpler methodology and requires substantially less data (whi	nich is
198	typically not accessible outside the forecast centers). These historical errors are routinely reported by the forecast cert	enters (
199	see Section 4.1.2 for information on the data sources used in this paper). Note that errors in our implementation (n	neither
200	error nor the auto-regressive coefficient) vary with lead time. We calibrate a constant mean absolute error in combination of the constant mean absolute error in combination error in combination error	nation v
201	a single auto-regression coefficient (see Section 5.2.1 for calibration and Section 5.2.2 for the influence of simplify	ificatio
202	Moreover, the mean absolute error is converted into a standard deviation using a fixed relationship assuming	g a nor
203	distribution of the error and scaled with the applied time step to allow the user flexibility in the applied time step.	

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The determination of the ensemble members is subsequently based on the sum of the forecast and random error components. In other words, we add the along-track and cross-track error to forecasted along- and cross-track. An example of the first 20 ensemble members is presented in Figure 2B. Using this procedure, 10,000 ensembles are generated for each forecast case within this study; however, it is possible to use fewer ensemble members to reduce the computational cost but at larger statistical uncertainty (see Section 5.3.2 for trade-offs).

210 2.3.2 Intensity realizations and calibration

Similar to the track realization, the maximum wind speed (intensity) at a specific interval is determined using a random sampling approach. The starting point is the official forecast of intensity that is interpolated to include values at all requested times, and a random error component (VE_t) is added.

$$VE_t = e_t VT_{t-3} + F_{rnd}$$
 Equation 3

in which VE_t at time steps t, e_t is a constant, VE_{t-3} are errors of the previous time step (typically 3 hours) and F is random numbers that are normally distributed, scaled with the mean absolute error and is limited to +/- 2σ .

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The inland wind decay model adjusts the maximum intensity as a function of the distance inland, is directly based on DeMaria et al. (2009) and is computed with Equation 4. If the intensity of any inland ensemble member exceeds this predetermined value at any forecast time, the intensity is adjusted to match this value. Subsequently, the intensity errors are recalculated based

220 on the adjusted intensity. Additionally, if the intensity of an inland ensemble member falls <7.7 m/s (15 knots) at any point in





time, the TC intensity is reset to zero for all subsequent periods to overcome any unrealistic reintensifying TCs. All these criteria follow DeMaria et al. (2009).

$V_i = 20 + 120e^{0.0035D}$ Equation 4

in which the maximum wind speed (Vi) in knots and the distance to land (D) in kilometers (with negative values indicating inland cyclones) are given, the intensity of an inland cyclone can be determined.

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The intensity implementation differs from DeMaria et al. (2009) in the following ways. We remove the dependency that the error scales with wind intensity and bias correction. Again, the determination of the ensemble members is based on the sum of the forecasted and random components computed with Gaussian mean absolute errors and an auto-regressive constant over lead time. Similar to the track realization, intensity errors are scaled with the time step to overcome any time step dependency. The influence of the simplifications and the difference compared to NOAA operational code based on the original DeMaria et al. (2009) and DeMaria et al. (2013) implementation are discussed in Section 5.2.2.

232 2.4 Parametric wind fields

233 After the determination of the ensemble members, the time and spatial varying wind fields are constructed and written in a polar coordinate system. Several (horizontal) parametric wind profiles have been presented in the literature (e.g., Fujita, 1952; 234 Chavas et al., 2015), with the original Holland wind profile (Holland, 1980) being the most widely used due to its relative 235 236 simplicity. Several codes have been developed for storm surge models to provide time and spatial wind and pressure fields 237 (e.g., Hu et al., 2012 for ADCIRC). Deltares has developed the Wind Enhance Scheme (WES; Deltares, 2018) to generate TC 238 wind and pressure field around the specified location of a tropical cyclone center and given a number of TC parameters. In its current implementation, information on wind radii (radius of gale force winds) can be considered in the Holland et al., (2010) 239 formulation using information either from best track-data or from the proposed relationships of Nederhoff et al. (2019), which 240 241 increases the accuracy of the method. Furthermore, the asymmetry of the wind field in a TC is also implemented, as delineated by Schwerdt et al. (1979). Additionally, tropical cyclone-induced precipitation can be incorporated using empirical 242 243 relationships such as IPET (2006).

244 2.5 SFINCS simulation and post-processing

After the determination of the wind fields for all the requested ensemble members, TC-FF runs a hydrodynamic model. In this study, we apply the compound flood model SFINCS (Leijnse et al., 2021), which lends itself well to a large number of simulations in a reasonable amount of time due to its reduced complexity. SFINCS reads the tidal boundary conditions and wind, pressure, and rainfall conditions from the wind fields. Once all the ensemble member simulations have finished, probability products regarding wind and flood hazards are created. These products are created by sorting the results for each





250 grid cell and providing estimates for either specific intervals (e.g. wind speeds > 35 knots or water depth> 15 cm) or quantile 251 estimates (e.g., 1% exceedance water level).

252 3 Case study

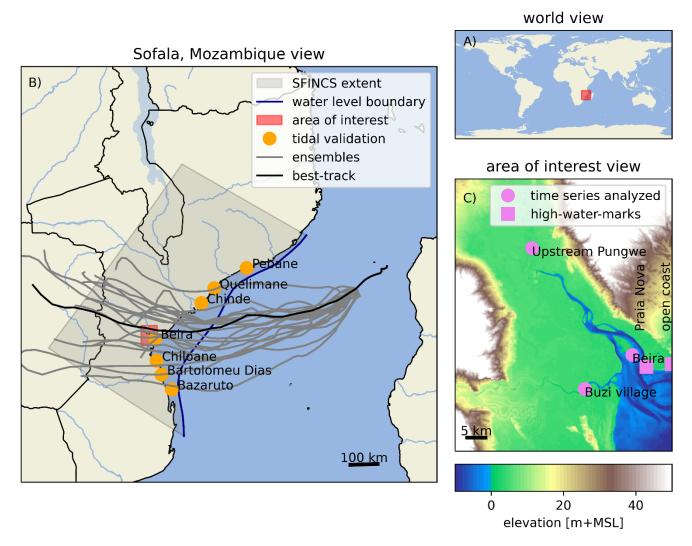
253 The TC forecasting framework is applied to a historical event that took place in Mozambigue's Sofala province: Cyclone Idai, in March 2019. Mozambique is a country located in southeastern Africa (Figure 2A). The country has a diverse population of 254 255 over 31 million people, of which 2 million live in the Sofala province in central Mozambique (National Institute of Statistics 256 of Mozambique, 2017). Sofala is primarily rural, with small communities along the Pungwe and Buzi river deltas (Emerton et 257 al., 2020). Beira is the province's largest city, home to over 500,000 people, and an important port linking the hinterland to the Indian Ocean. The city is prone to flooding, particularly during the rainy season, which generally extends from October to 258 259 April or May. This period coincides with the cyclone season, as cyclones often bring intense rainfall to the region. The 260 vulnerability of Beira to flooding is exacerbated by factors such as climate change, rapid urbanization, and limited infrastructure. 261

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263 Cyclone Idai was an example of a compound flood event that affected large parts of the coastal delta of Sofala (Eilander et al., 264 2022). The storm began as a tropical depression in the Mozambique Channel, causing extensive flooding after its first landfall 265 in early March. It later intensified as it moved back over the sea, developing into a tropical cyclone with 10-minute sustained wind speeds of 165 km/h. Idai made landfall near the port city of Beira, bringing powerful winds, resulting storm and heavy 266 267 rains that caused widespread flooding and destruction. Large areas were flooded, first around the coast and a few days later, 268 more inland in the Buzi and Pungwe floodplains. The total rainfall across the five days from March 13-18 ranged from 250-269 660 mm (NASA GPM, 2019). Over 112,000 houses were destroyed, and an estimated 1.85 million people were affected (UN 270 OCHA, 2019).







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Figure 2. View of the study site: (A) Mozambique's Sofala Province is situated in the southeastern region of Africa in the Southern Hemisphere. (B) Geographical and hydrodynamic representation of the study area. The SFINCS model extent, highlighted in Panel B, encompasses a portion of the Sofala region, forced offshore with a water level boundary, and is validated at seven tidal stations (indicated by orange circles; see Section 8.1). The best track is represented by a solid dark line, with the first 20 ensemble members 5 days before landfall demonstrated as gray lines. (C) The area of interest is the Pungwe estuary, situated near the city of Beira. Model validation also takes place at two high-water-marks close to the city (signified by a purple box), with model outcomes depicted

278 at three diverse locations across the estuary (marked by circles).





280 4 Material and methods

281 4.1 Materials

282 4.1.1 Elevation datasets

Several topographic and bathymetric datasets were collected and combined to develop a merged DEM. Data includes field survey data points collected during three campaigns in November-December 2020 across Beira, locally-collected LiDAR, bathymetric charts, MERIT (Yamazaki et al., 2017) and GEBCO19 (IOC, IHO and BODC, 2003). Careful consideration was given to prioritize specific datasets in space to ensure the most detailed, recent, and accurate datasets were used in a given area. For example, survey and LiDAR data is bare earth and prioritized over the usage of MERIT and GEBCO19. The merged DEM was produced on medium-resolution (50 m) regional DEM, and a fine-resolution (5 m) local DEM in Beira. For more information on merging the data, one is referred to Deltares (2021).

290 4.1.2 Forcing conditions

Tidal boundary conditions were based on harmonic constituents provided by TPXO 8.0 (Egbert and Erofeeva, 2002), and tidal amplitudes and phases for all available 13 components were applied. The best track data (BTD) by the Joint Typhoon Warning Center (JTWC) is used throughout this study for meteorological forcing conditions (JTWC, 2022). Reported error statistics by the JTWC for the 5-year average from 2016-2020 were used to inform the ensemble generation (JTWC, 2021). Ensemble members from TC-FF were compared to 1,000 members produced with the code from NOAA, NHC, and JTWC based DeMaria et al. (2009) and DeMaria et al. (2013) that is used operationally (Buck Sampson, personal communication; June 5, 2023).

298 4.1.3 Validation data

Observed tidal coefficients near the city of Beira were used for the calibration and validation of the model (van Ormondt, 2020; see Figure 2 for locations). The validation of the event Cyclone Idai (2019) consisted of comparing both, observed and modeled flood extent in deltas of the Pungwe and Buzi rivers and high-water marks in the city of Beira. The observed flood extent was derived from Sentinel-1 synthetic aperture radar data (Eilander et al., 2022); and two observed high water marks (Deltares, 2021) were used, one at Praia Nova, in the western side of the city, and another one at the open coast beach in the southeast (see Figure 2 for locations). Correspondingly, values of modeled flood extent and high water marks were output at the same locations.





306 4.2 Methods

307 4.2.1 Area schematization

For this study, we employed the Super-Fast INundation of CoastS (SFINCS) model, which solves the simplified equations of mass and momentum for overland flow in two dimensions (Leijnse et al. 2021). The goal was to create one continuous compound flood area model that computes tidal propagation, storm surge, pluvial and fluvial flooding.

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312 The area schematization builds upon Eilander et al. (2022) but varied in three ways. First, we extended the model alongshore 313 and in deeper water to alleviate the need to nest in a large-scale regional coastal circulation model and generate tidal 314 propagation and storm surge within the domain. Using a quadtree implementation (e.g., Liang et al., 2008), we applied a variable model resolution ranging from 8000 to 500 meters. A quadtree is a technique in which the refinement from one level 315 to another is based on the original cell but divided into 4 smaller cells with 2 times smaller grid size and allows extending the 316 317 model setup into deeper water without having time step restrictions in deeper water based on the explicit numerical scheme of SFINCS. Second, high-resolution topo-bathymetry and land roughness were included in the native resolution utilizing sub-318 319 grid lookup tables (Leijnse et al., 2020). However, the hydrodynamic computations were performed on a coarser resolution to save computational time. Up to 10-meter DEM information was included in the 500-meter grid cells (i.e., factor 50 refinement). 320 Lastly, sub-grid bathymetry features were included to account for maximum dune height based on the DEM to control overflow 321 322 during storm conditions around Beira.

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324 A spatially-varying roughness and infiltration was used based on land elevation. All points above mean sea level (MSL) have a high Manning friction coefficient of 0.06 s/m^{1/3}, and an infiltration rate of 1.9 mm/hr (typical values from HSGs Group C; 325 United States Department of Agriculture, 2009), and all other points have lower friction of 0.02 s/m^{1/3} to represent water and 326 327 do not have any infiltration. The SFINCS model was forced with tidal boundary conditions and time- and spatially-varying 328 winds, pressure, and rainfall fields. We refer to Section 8.1 for calibration of the tides, in which we show that the area model 329 reproduces tides with a median MAE of 21 cm. Wind and pressure fields were created with the Holland wind profile (Holland 330 et al. 2010) based on the BTD (see Section 2.4 for details). Rainfall for TCs was based on the Interagency Performance 331 Evaluation Task Force Rainfall Analysis (IPET, 2006) method. Comparison with the reported rainfall total revealed a 332 significant underestimation of cumulative rainfall during Idai based on IPET. Based on the magnitude of the underestimation, 333 rainfall estimates by IPET were tripled, resulting in a cumulative rainfall in the area of interest of 495 mm for the best track, 334 which is in a similar order of magnitude as observed (see Section 3).

335 4.2.2 Simulations periods

The validation of the area schematization focused on two time periods. First, 3 spring-neap cycles (January 13, 2022, until February 26, 2022) were used for the tidal calibration and validation in the area of interest (see Appendix 8.1). Second, Idai





- was hindcasted forced with the JTWC BTD and compared to observational data for flood extent and high-water levels (Section
 5.1). After validation of the area schematization, the new forecasting methodology introduced in Section 2 was applied. Various
 lead times ranging from 1 to 5 days before the second landfall for 10,000 ensemble members were computed (Section 5.3).
- Model runs were performed on the Deltares Netherlands Linux-based High-Performance Computing platform using 10 Intel
 Xeon CPU E3-1276 v3. On average, a 7-day Idai simulation took about 4 minutes on a single core. Running all 50,000 events
 took ~15 days.

345 4.2.3 xModel skill

Several accuracy metrics were calculated throughout this study: model bias, mean-absolute-error (MAE; Equation 5), rootmean-square-error (RMSE; Equation 6), unbiased RMSE (uRMSE; RMSE with bias removed from the predicted value). These error metrics are used for comparison in water levels, wind speed and track errors.

$$MAE = \frac{1}{N} \sum (|y_i - x_i|)$$
Equation 5
$$RMSE = \sqrt{\frac{1}{N} \sum (y_i - x_i)^2}$$
Equation 6

349 where N is the number of data points, y_i is the i-th prediction (modeled) value, x_i is the i-th measurement.

350

Moreover, skill is quantified by binary flood metrics (Wing et al. 2017). The model output (M) is converted to one of two states: wet (1) or dry (0), using a commonly used threshold of 15 cm (e.g., Wing et al. 2017) and compared to the Sentinel benchmark data (B). The Critical Success Index (C; Equation 7) accounts for both overprediction and underprediction and can range from 0 (no match between modeled and benchmark data) to 1 (perfect match between modeled and benchmark data).

$$C = \frac{M_1 B_1}{M_1 B_1 + M_0 B_{1+} M_1 B_0}$$
 Equation 7

355

For the comparison of cumulative distribution functions (CDF) of cross-track, along-track and intensity, we also applied the Continuous Ranked Probability Score (CRPS; Matheson & Winkler, 1976). CRPS measures how good forecasts are in matching observed outcomes; where CRPS = 0, the forecast is wholly accurate, and CRPS = 1, the forecast is wholly inaccurate.





$$CRPS(F,x) = \int_{-\infty}^{\infty} [F(y) - F_0(y)]^2 dy$$

Equation 8

360 where F(y) is the CDF is associated with an empirical probabilistic reference and prediction.

361 4.2.4 Analysis method

362 The analysis of forecasting results was undertaken using several methods. Initially, extreme wind speeds and water levels were

363 assessed by charting them as time-series data, inclusive of quantile estimates such as the 95% confidence interval (CI).

364 Following this, the maximum values registered during the simulation were organized into cumulative distribution functions

365 (CDFs). This process offered insights into their exceedance probability. Finally, the mean probability of flooding was

366 computed. The method to derive this value entailed counting the instances where computational cells registered a minimum of

367 15 cm of water. Only cells positioned above mean sea level (MSL) were incorporated into the area estimates.

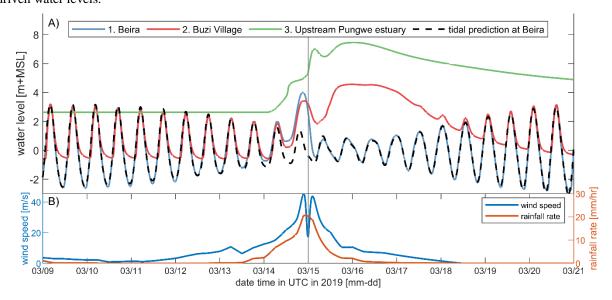




368 5 Results

369 5.1 Verification of the numerical model for Cyclone Idai

Computed water levels near Beira show the strong tidal modulation and the wind-induced storm surge during the landfall of the cyclone (Figure 3 – panel A; blue line for water level and vertical line for moment of landfall). Based on the difference between the predicted astronomic tide and the total modeled water level, we estimate a storm surge of >3.5m due to the ~45 m/s wind speeds (Figure 3 – panel B). The storm surge at Beira is driven by wind setup as well as pluvial and fluvial drivers. Deeper in the estuary, in the Pungwe flood plains, water levels peaked several days after landfall due to intense upstream rainfall and subsequent runoff. Water levels near Buzi Village seem to be a combined result of first marine and second riverinedriven water levels.



377

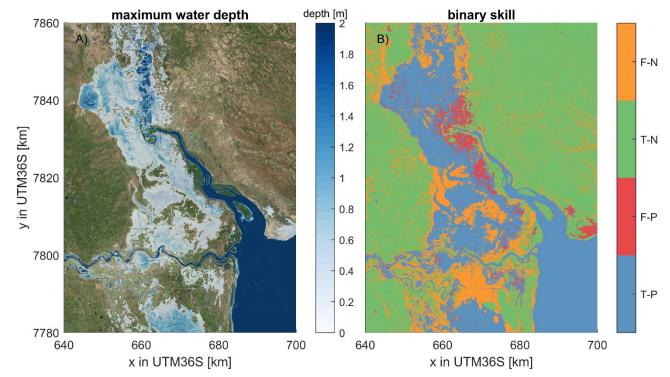
Figure 3. Time series of water levels, wind speed, and precipitation within the study area. (A) Computed water levels at various locations (blue for Beira, red for Buzi village, and green for upstream in the Pungwe estuary (see Figure 2C for their location) and the black dashed line representing the astronomical prediction at Beira. (B) Simulated wind speed (blue) and rainfall rate (red) over the same period. Idai made landfall on March 15, and its powerful winds and rainfall resulted in marine flooding at Beira and riverine-driven flooding upstream in the estuary. The vertical line represents the moment of landfall.

383 Validation of the SFINCS model for the observed extent (blue colors in Figure 4A) gives confidence in the ability to simulate 384 the compound flooding (Figure 4). The model can reproduce the Sentinel-1 derived extent with a Critical Success Index of 385 0.59. This skill score is comparable to previous work by Eilander et al. (2022), albeit somewhat lower. Based on the differences 386 between the modeled and satellite-derived extent, it becomes apparent that the model underestimates the flooding around the 387 Buzi River (false negative; orange colors in Figure 4B around 660-7800 km). We hypothesize this is due to the lack of river inflow related to an underestimation of rainfall further upstream and/or overestimation of infiltration due to soil saturation 388 389 which is not considered. Moreover, the comparison with satellite-derived flood extent indicates an overestimation of the 390 flooding at Beira (false positive; red colors in Figure 4). Here, we suspect that the benchmark data might be off, and the coastal





- flooding already receded before the Sentinel data recorded the extent. The observed high-water marks near Beira ranged from 3.6m within the estuary to 2.9 m + MSL at the open coast and are reproduced by SFINCS with respectively 3.8 and 3 m +
- 393 MSL. This difference suggests a positive bias of the model results at the coast of ~10-20 cm, similar to the tidal validation (see
- Appendix 8.1), which revealed a median MAE of 21 cm.



395

396 Figure 4. Maximum computed water depth (Panel A) and binary skill of flood extents for Idai (Panel B). Water depths are 397 downscaled from the model resolution to the 10x10 meter resolution of the topo-bathymetry. The binary skill evaluation (Panel B) 398 assists in determining the model's accuracy and dependability, and the Sentinel-1 radar data is used as a reference to determine 399 skill. A true-positive (T-P) outcome denotes a correct flood prediction by the model compared to Sentinel-1 derived extent, whereas 400 a false-positive (F-P) occurs when the model forecasts a non-existent flood. In contrast, a false-negative (F-N) indicates where the 401 model overlooks an actual flood, and a true-negative (T-N) result occurs when the model accurately predicts the lack of a flood event. 402 The model produces large-scale flooding, which is largely also observed in the data, but local differences of over- and 403 underestimation exist. The coordinate system of this figure is WGS 84 / UTM 36 S (EPSG 32736). © Microsoft.

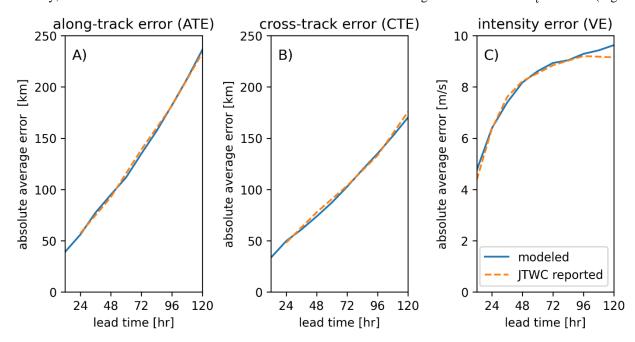




405 5.2 Calibration and influence of simplifications of TC-FF

406 5.2.1 Calibration of TC-FF: mean absolute error and auto-regression

This study used JTWC-reported errors for the along-track, cross-track, and intensity for the Southern Hemisphere to calibrate our methodology (JTWC, 2021). For other case studies, for example, based on different forecasting agencies or in other ocean basics), these reported errors can be used instead. Calibration is performed by minimizing the square-root difference between computed and reported mean absolute values for various lead times using the Nelder-Mead method. This effort resulted in mean absolute errors for B and D of 68.5 and 55.3 km and autoregression coefficients a_t , c_t , of 1.214 and 1.181 (Figure 5A and B) for the along-track and cross-track. Moreover, we calibrated the mean absolute error and regression coefficients for the intensity, which resulted in mean absolute errors for F of 9.28 m/s and autoregression coefficient e_t of 0.624 (Figure 5C).



414

Figure 5. Comparison of calibration results for the probabilistic forecasting method TC-FF (solid blue line) and the Joint Typhoon Warning Center (JTWC) reported error statistics based on the 5-year average (2016-2020) in the Southern Hemisphere (dashed orange line). Panel A represents the along-track error, Panel B demonstrates the cross-track error, and Panel C exhibits the wind speed or intensity error. Modeled errors are based on 1,000 ensemble members. Modeled absolute average errors are similar to JTWC.

420 **5.2.2** Comparison of TC-FF with operational forecast products

421 Errors produced by TC-FF are compared to the implementation from NOAA, NHC and JTWC that are used operationally.

422 Minor differences between the TC-FF and full implementation based on DeMaria et al. (2009) and De Maria et al. (2013) exist

- 423 and are attributed to the simplifications used in the error distribution (including the lack of GPCE) and lack of bias conditions.
- 424 The distribution in along-track, cross-track, and intensity error is typically in the same order (Figure 6), which is confirmed by

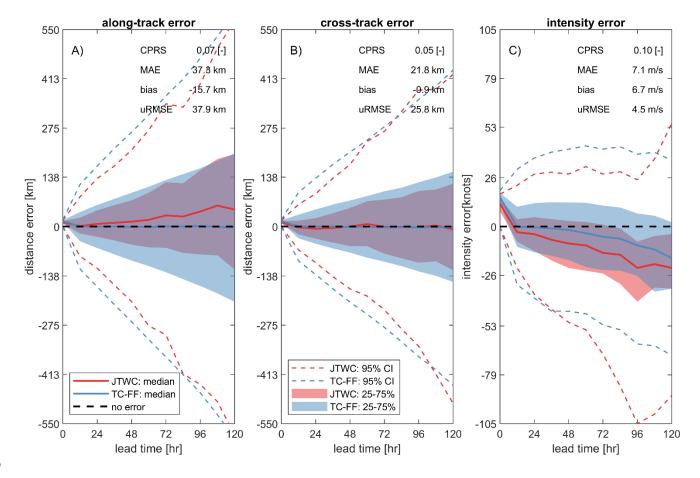




425 a median CPRS over various lead times from 0 to 120 hours of 0.07, 0.05, 0.10 and median MAE of 37 km, 21 km, and 7 m/s 426 of for respectively the along-track, cross-track, and intensity. At the same time, TC-FF has by design no bias corrections, 427 whereas the operational system does, leading to the positive median along-track error in red compared to the blue line in Figure 428 6A and a median bias of -16 km. Besides the median estimates, the interquartile range (25-75%) and 95% CI match relatively 429 well for the along-track and cross-track errors. Larger differences are found for the intensity error. In general, the wind intensity 430 error looks visually erratic and doesn't start at zero for no lead time, which is the result of the inland wind decay model. Both JTWC and TC-FF have a negative bias due to the effect of land, but TC-FF does have a median bias of +6.7 m/s compared to 431 432 JTWC, suggesting that TC-FF overestimates. However, more substantial differences are found for the interquartile range and 433 95% CI. These findings for the along-track, cross-track, and intensity are supported by a more detailed analysis of the CDF 434 for the different parameters as a function of lead time (Figure 12, Figure 13, Figure 14). For the along-track and cross-track, 435 we observe an increase in the MAE and uRMSE as a function of lead time but a decrease in the CPRS. The increasingly larger error distribution influences this pattern. Moreover, TC-FF produces Gaussian-distributed errors while the JTWC error 436 437 distribution differs since it is based on historical error distribution and adjusted based on the GPCE. Similar to Figure 6, larger 438 differences are found for the intensity error, which is influenced by the bias correction that increases with lead times.







439

Figure 6. Comparison of validation results for the probabilistic forecasting method TC-FF (blue line) and the Joint Typhoon Warning Center (JTWC) operational product (red line). Panel A represents the along-track error, Panel B demonstrates the crosstrack error, and Panel C exhibits the wind speed or intensity error. Errors are computation for both the TC-FF and JTWC are based on 1,000 ensemble members. Solid lines are median estimats, shaded areas the interquantile range (25-75% CI) and dashed line the 95% CI. TC-FF and JTWC produce broadly similar error distributions for different lead times.

445 5.3 Forecasting of Idai using the TC-FF

446 This section presents the application of forecasting Idai using the TC-FF.

447 5.3.1 Uncertainty three days before landfall

- 448 The TC-FF method with 10,000 ensemble members is applied to the case of Cyclone Idai. The results reveal that accounting
- 449 for the uncertainty of the TC track and intensity of eye three days before landfall results in considerable uncertainty regarding
- 450 wind speeds and water levels near Beira (Figure 7) or the region (Figure 8). In particular, the wind speeds show a 95% CI of
- 451 about 7-40 m/s at the moment of landfall (Figure 7A) versus ~45 m/s or a Saffir-Simpson Hurricane Wind Scale (SSHWS) of
- 452 2 of the best track. Moreover, TC category 1 wind speeds could occur as early as March 15 at 21:30 UTC or as late as March





453 16 at 19:50 UTC. This spread of possible maximum wind speeds at Beira results from the large uncertainties in intensity and a difference in landfall location and time. Based on the same model simulations, the empirical cumulative distribution function 454 455 (CDF) of the maximum wind speed at Beira ranges from 8.8 to 59.2 with a median wind speed of 25.5 m/s, while the best track has a 5.9% exceedance probability (Figure 7B). Consequently, water levels vary greatly (Figure 7C). For example, 456 457 ensemble members can exhibit a sizeable wind-driven setup due to TC wind blowing from offshore into the estuary, pushing 458 water up in the estuary and at Beira. For landfall locations west of the estuary, the wind blows offshore, resulting in a large set-down. Note that Beira is in the Southern Hemisphere, and due to the Coriolis effect, TCs spin clockwise. The highest water 459 levels occur when high tide and wind-driven setup coincide, which explains the three peaks in the 95% CI water level given 460 the semi-diurnal tide and the highest possible wind speed for ~1.5 days (Figure 7C). The maximum water levels are dominated 461 462 by the tide except in the situation of cyclone impact (see the CDF in Figure 7D and the minimum value of ~3.5m+MSL around 463 90%, which is influenced by the tide and time window over which it is determined). The specific track of Idai resulted in relatively extreme conditions compared to other possible combinations (both for winds and water levels). A similar pattern 464 465 can be observed in the spatial maps shown in Figure 8. The average probability of flooding in the area is 26%, with higher probabilities of flooding found in the lower-lying portions of the estuary (note we are excluding points below MSL; Figure 466 8A). The 1% exceedance flood depth threshold shows a large extent and is quite similar to the computed extent due to Idai 467 468 (see Figure 4A for comparison with Figure 8B). The main difference is that there is more flooding near the city of Beira and somewhat less near Buzi Village. The match between the 1% exceedance flood depth and the best track with Idai suggests that 469 the event was relatively severe and implies that even though many other potential scenarios could have unfolded, they likely 470 471 would not have resulted in the same extensive flooding caused by Idai.





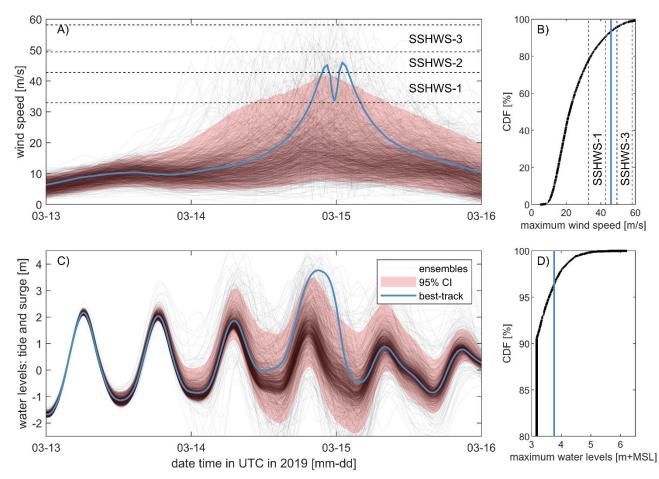
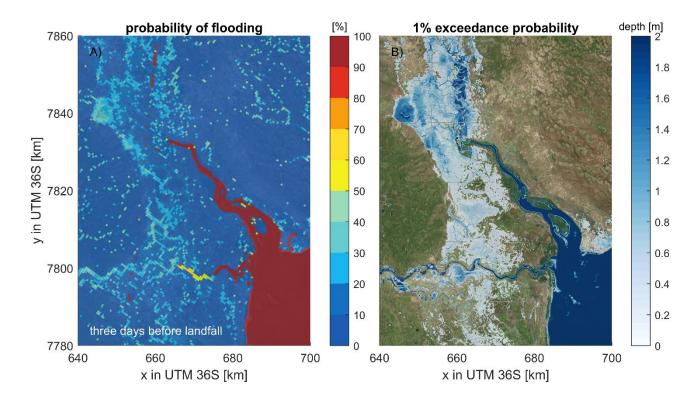


Figure 7. Multi-panel Analysis of wind and water Levels three days before landfall: (A) time series of wind speeds, (B) maximum wind speeds, (C) time series of water levels near Beira, and (D) maximum water levels. Data is derived from 10,000 ensemble members (black transparent line; every 10th plotted) with red shading representing the 95% CI. The best track (blue line) and the Saffir-Simpson Hurricane Wind Scale are included for comparison (panels A and B only). There is substantial uncertainty in wind speeds and water levels near Beira three days prior to landfall.

- 478
- 479
- 480







481 482

Figure 8. Probabilistic flood analysis for Cyclone Idai three days before landfall: (A) Spatial distribution of flooding probability; (B) Corresponding 1% exceedance water depth estimates, highlighting areas at most significant hazard. Results in panel A are determined from 10,000 ensemble members on the original 500-meter model resolution, while water depth in panel B is downscaled to the original 10x10-meter bathymetry resolution. Higher probabilities of flooding are found in the lower-lying portions of the estuary. The coordinate system of this figure is WGS 84 / UTM 36 S (EPSG 32736). © Microsoft.

488 **5.3.2** Influence of sampling size

As described by Cashwell and Everett (1959) and DeMaria et al. (2009), the precision of Monte Carlo techniques is proportional to the number of ensemble members (N). The convergence rate typically shows a slower progression than 1/N, constituting a limitation intrinsic to all Monte Carlo methods. To investigate the convergence rate and the error induced by employing a finite number of ensemble members, the Idai forecasting case three days prior to landfall is used, analogous to the preceding section, albeit with a variable number of ensemble members. Additionally, bootstrapping is employed to approximate convergence rates and the accompanying uncertainty.

- 495
- 496 The estimation of the 95% exceedance maximum water levels in proximity to Beira exhibits convergence with the number of
- 497 ensemble members, albeit with considerable deviations compared to a fully converged solution with 10,000 members when
- 498 implementing a low number of ensemble members (Figure 9A). For instance, employing merely 50 ensemble members results
- 499 in an interquartile range (25-75%) of -0.28 to +0.10 m. Increasing the number of ensemble members reduces this sampling
- 500 uncertainty to a range of -0.09 to +0.06 m for 200 ensemble members.





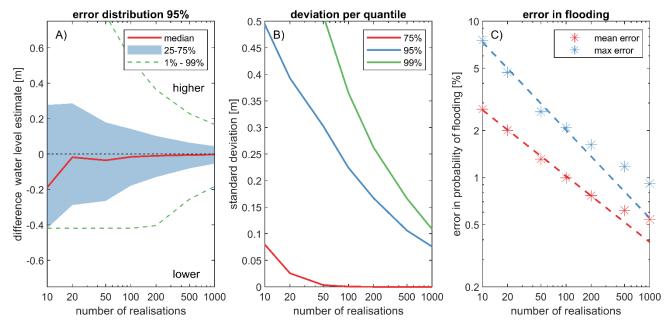
501 Similarly, the standard deviation for several quantiles of maximum water level estimates at Beira reduces with more ensemble 502 members. It exhibits a similar pattern from higher to lower quantiles (Figure 9B). In essence, estimating rare events necessitates 503 executing more ensemble members to attain comparable convergence. This study found that the 95% exceedance maximum 504 water level at Beira when utilizing 200 ensemble members has a standard deviation of 21 cm (blue line Figure 9B). This level 505 of convergence seems acceptable since it is in a similar order as the skill of the hydrodynamic model (see Section 5.1).

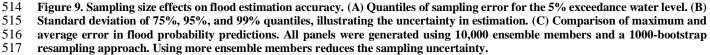
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513

507 The probability of error in flood potential is expressed as a function of N on a log-log plot (Figure 9C). Compared to a fully 508 converged solution with 10,000 members, for N=200, the mean error constitutes 0.95%, and the maximum error amounts to 509 1.53%. Note that this estimate is without considering the model error. In the log-log diagram, the errors exhibit near-linear 510 correlations with N and could serve as a basis for determining the number of ensemble members needed for a specified 511 confidence level. For instance, to achieve a maximum error of 1% in flood probability, it would be necessary to utilize 500

512 ensemble members.





518 5.3.3 Importance of lead time

519 Thus far, the probabilistic TC forecasting framework has been implemented three days prior to the landfall of Idai. 520 Nevertheless, the forecast's results fluctuate with lead times, consequently influencing the associated evaluations of water 521 levels (Figure 10) and flood probabilities (Figure 11).





522

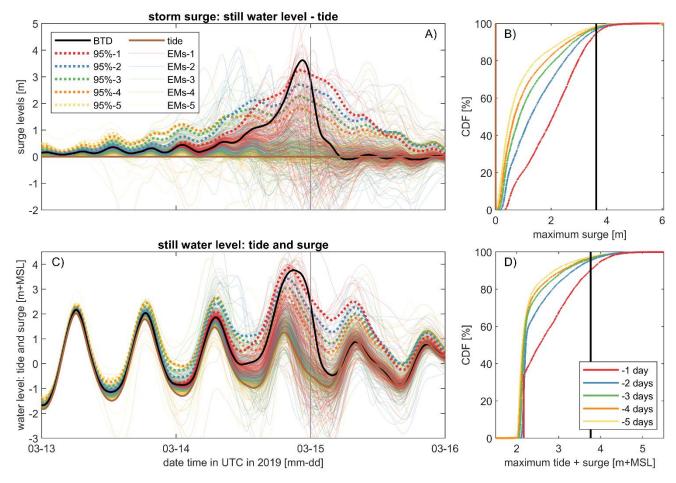
The predicted water levels (tide + surge) vary with lead times (Figure 10A and C). Specifically, at a lead time of five days 523 524 before landfall, an (unsurprisingly) larger spread between the ensemble members is observed compared to lead times of, for 525 example, one or three days. Moreover, as landfall approaches, the time series converges since increasing ensemble members 526 produce highly similar predictions. For example, notice how individual ensemble members 1 day before landfall show similar 527 storm surges and still water levels (i.e., the concentration of lines which becomes more apparent in Figure 10). Moreover, the 5% and 95% exceedance values become less spread out and more peaked around landfall (dashed lines in Figure 10). This 528 529 convergence is more apparent for the storm surge. The CDF of the maximum storm surge levels increases with reducing lead 530 time (Figure 10B). For example, the median storm surge increases from 0.5m five days before landfall to 0.9 and 2.0m for lead 531 times of three days and one day, respectively (notice the increasing median estimate in the CDF plot from 5 to 1 day in Figure 532 10B). This increase in maximum storm surge shows the increasing certainty that the TC will land near Beira. However, for 533 other locations, the opposite may occur as the landfall shifts away from it. The still water levels are influenced by both tidal 534 motions and the influence of the TC (Figure 11C). This strongly influences the maximum computed still water level (Figure 535 11D). For instance, the lowest maximum water level for all simulations is around ~ 2 m above MSL, resulting from the maximum tidal range rather than the TC itself. The 95th quantile of the maximum still water level is 3.4 m + MSL five days 536 537 prior to landfall, which increases to 3.6 and 4.0 m+MSL for lead times of three days and one day, respectively. The best track 538 of Idai is included as a reference and estimated to have a 9% probability of exceedance 1 day before landfall.

539

A large portion of the Sofala province faces a minor flood risk five days before the actual landfall. The flood probability for the estuary near Beira increases as lead times reduce (Figure 11B). In particular, the average probability of flooding five days before landfall is 15%, increasing to 17 and 24% for lead times of three and one day, respectively. Conversely, for the entire model domain, a probability of greater than 1% flooding declines from 97 to 94 and 64 km² for lead times of five, three, and one day (Figure 10A). In other words, five days before landfall, less confidence in predictions translates into more spatial variability on flooding probability tied to a larger impact area. Closer to the actual landfall, there is more certainty over which area will be affected.





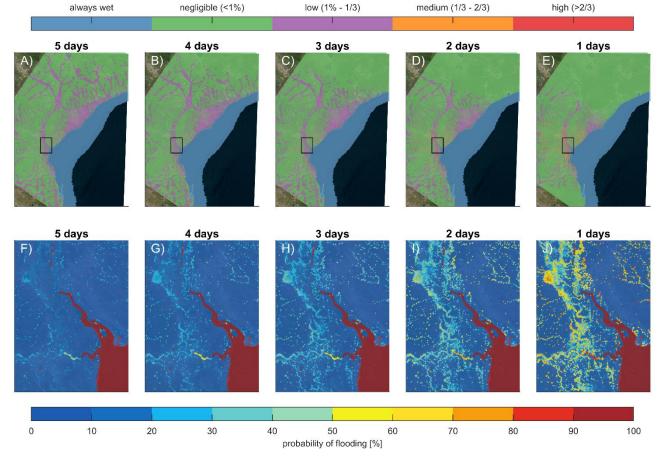


547

548 Figure 10. Forecasted water levels in Beira for 1-5 day lead times: temporal evaluation and cumulative distribution. Panel A and C: 549 Time series illustrating the forecasted water levels in proximity to Beira with lead times ranging from 1 to 5 days prior to landfall 550 showcasing both individual ensemble members (solid transparent lines; every 100th plotted), tide-only (brown), best-track (black) 551 and quantile estimates (95% dashed lines). Panels A and C use the same colors and line styles. Panel B and D: Cumulative 552 distribution function (CDF) showing the maximum water levels in ascending order for all ensemble members, providing insights 553 into the probability of occurrence for various water level thresholds. Panels B and D use the same colors. Panels A and B show the 554 storm surge levels (computed still water levels minus predicted tidal levels), while Panels C and D present the still water level (tide 555 and surge).







556

557 Figure 11. Evolution of the flood probability prior to landfall: Panels A-E depict the spatial distribution of flooding probabilities at 558 5, 4, 3, 2, and 1 day(s) before landfall, respectively. Color gradients represent the varying probability. The top panels focus on the entire area simulated and the bottom panels on the Pungwe and Buzi river deltas. With decreasing lead time, the area that could be affected decreases while there is an increased probability of flooding near Beira. The coordinate system of this figure is WGS 84 /

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561 UTM 36 S (EPSG 32736). © Microsoft.
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563 6 Discussion

This paper describes a new probabilistic method to forecast TC-induced coastal compound flooding by tide, surge, and rainfall 564 using Monte Carlo sampling. Due to the limited number of observations on TC evolution, for short-term operational analyses, 565 an autoregressive technique that imposes potential errors on top of the forecasted track is preferred over those parametric 566 567 sampling techniques used for long-term and strategic risk assessments based on historical records (e.g., Nederhoff et al., 2021). 568 In addition, for the same scarcity of observation, there is limited knowledge of the underlying joint distribution between TC and ocean characteristics, which makes Monte Carlo sampling preferred compared to sampling techniques that are highly 569 efficient for complex multivariate patterns such as cluster analysis (e.g., Choi et al., 2009) and MDA methods (e.g., Bakker et 570 571 al., 2022). However, exploring the possibility of increasing efficiency via the aforementioned methods is important, especially 572 since the error space increases as a function of lead time, and estimating these events requires increasing amounts of ensemble members (Figure 9B). However, this is a topic that requires an in-depth analysis and is beyond the scope of the present study. 573

574

575 Compared to the implementation of DeMaria et al. (2009) and DeMaria et al. (2013), the ensemble generation is simplified by removing bias corrections, applying a single normal error distribution calibrated on historical errors (Figure 5), and does not 576 577 account for GPCE. While we acknowledge these simplifications, this method does make it possible to account for TC forecasting errors for any ocean basin based on reported average historical errors alone. Nevertheless, the behavior of a specific 578 tropical cyclone (TC) does not necessarily conform to the "average" pattern, and differences between the operational JTWC 579 580 model were found (Figure 6). The implications of these assumptions on the precision and predictive proficiency of our 581 approach for coastal compound flooding remain undetermined. Our implementation has been recently integrated into an 582 operational system tailored for the contiguous United States. Verification of the reliability of this operational system is 583 currently pending. Regardless, TC-FF compares well with the predictions provided by ECMWF of Idai that showed a 584 probability of 50 to 90% of severe flooding four to one day before landfall (Figure 10).

585

In the introduced methodology, we apply the compound flooding model SFINCS. While the validation gave confidence that 586 587 the hydrodynamic model reproduces the main tidal motions and flooding during Idai, differences did exist compared to the data (Figure 4). The model skill could be improved by including additional wind radii information in the parametric wind 588 589 model (e.g. radius of gale force winds along different quadrants) and more accurately resolving on-land winds, rainfall, and infiltration processes. For example, Done et al. (2020) present a methodology to account for terrain effects by adjusting winds 590 591 from a parametric wind field model by using a numerical boundary layer model. Here, we applied the IPET empirical 592 relationship that relates pressure drop to rainfall intensity. Initially, deployment showed the necessity to triple the rainfall rate 593 due to severe underestimation of the total rainfall and associated flooding. Improvement (stochastic parametrization) of TC 594 rainfall could overcome this limitation. Moreover, SFINCS was run with a constant infiltration rate and does not account for 595 drainage systems and flood protection measures besides the frontal levee. It is also unknown how the topo-bathymetry that





was collected before Idai influenced results. Lastly, the effects of waves (e.g., setup, runup, overtopping) and morphological change were not considered. All these limitations affect the model skill and could explain some mismatches observed compared to Sentinel-1 data and high-water marks at Beira. However, the computational efficiency of SFINCS allowed us to run thousands of ensemble members on limited computational resources. We accept the loss of some model accuracy with this gain of speed.

601

The focus of the development of TC-FF has been geared to the computation of overland flooding. However, TCs pose significant hazards through both water *and* wind. A study by Rappaport (2014) indicated that from 1963 to 2012 in the United States, approximately 90% of fatalities associated with tropical cyclones were due to water-related incidents. The wind-related fatalities were about 8%. This does not provide insight into the cause of damage associated with landfalling TCs, nor does it provide insight into how these ratios vary across the globe. Regardless, TC-FF does provide the possibility to estimate extreme wind speeds and link this to potential damage as an additional data product. Including wind damage as part of our framework is something we are planning to work on in the future.

609 7 Conclusions

A new method and highly flexible open-source tool was developed to perform probabilistic forecasting of tropical cyclone-610 611 induced coastal compound flooding. The Tropical Cyclone Forecasting Framework, TC-FF, computes a set of ensemble 612 members based on a simplified DeMaria et al. (2009) method. In particular, TC-FF uses gridded time- and spatially-varying wind and pressure fields or forecasted tracks and combines this with historical observed error on the along-track, cross-track, 613 614 and intensity. Subsequently, the tool creates a temporally and spatially varying wind field, including rainfall, to force a 615 computationally efficient compound flood model. This approach allows for the inference of probabilistic wind and flood hazard 616 maps calibrated to any ocean basin in the world with limited computational resources. In contrast to the current practice, TC-FF allows uncertainty analysis using large ensembles produced with physics-based models, narrowing down confidence bands 617 618 on forecasting coastal compound flooding focused on operational TC risk analyses.

619

The validation of the quadtree SFINCS model for Mozambique's Sofala province showed reliable skill in terms of tidal propagation in the area of interest (median MAE of 21 cm), including good skill in reproducing the observed flood extent for the case of the flooding caused by Cyclone Idai (2021). The model was able to reproduce the storm surge generation during landfall and flooding near the city of Beira, including the subsequent compound flooding resulting from rainfall runoff in the Pungwe estuary (critical success index of 0.59). Moreover, the model runs efficiently with a wall clock time of 4 minutes for a 7-day event allowing it to be deployed in probabilistic operational assessments when using multiple cores.





627 TC-FF was calibrated with the average reported errors for the southern hemisphere via the Nelder-Mead method to determine the mean absolute errors and autoregression coefficients. A comparison between TC-FF and JTWC (based on the complete 628 629 implementation of DeMaria et al., 2009) and DeMaria et al., 2013) revealed minor differences. In particular, for various lead times from 0 to 120 hours, a median Continuous Ranked Probability Score (CRPS) of 0.07, 0.05, 0.10 and median MAE of 37 630 631 km, 21 km, and 7 m/s for respectively the along-track, cross-track, and intensity error were found. These findings give 632 confidence that the TC-FF, including the simplified DeMaria et al. (2009) implementation, can be used for more generalized 633 applications in data-scarce environments. 634 635 TC-FF provides valuable insights into the uncertainty of wind speeds, water levels, and potential flooding due to Idai, revealing 636 the impacts of track and intensity uncertainties. This is demonstrated in the wide array of possible maximum wind speeds and 637 significant fluctuations in water levels, which are primarily affected by tidal influences and the cyclone. For instance, even just three days prior to landfall, there's a broad spread in the predicted flood areas. This suggests that there is still a significant 638

639 chance that Idai may not hit the anticipated area or may not generate a substantial storm surge.

640

The precision of forecasts is directly related to the number of ensemble members used. A mean error in flood probability of less than 1% and <20 cm sampling errors for the 1% exceedance water level at Beira required 200 members. Based on that, we determine that at least 200 ensemble members are needed to get reliable water levels and flood results three days before landfall. A higher number of ensemble members reduces sampling uncertainty and increases the accuracy of water level and flood potential estimates.

646

647 The lead time before landfall has a considerable impact on the forecast's precision. As the lead time decreases, the variability 648 of forecasts diminishes, and the forecasts converge to similar predictions. Similarly, the probability of flooding in certain areas, 649 such as the estuary near Beira, increases as the lead time shortens, providing more certainty over the areas that will be affected 650 by the event.

651

TC-FF offers a significant advancement compared to the current status quo of a single deterministic simulation when forecasting tropical cyclone compound flooding hazards. This approach facilitates a comprehensive understanding of complex interdependencies and uncertainties. By quantifying the likelihood of various outcomes, probabilistic methods enable stakeholders to make more informed decisions, allocate resources better, and enhance preparedness and resilience in the face of these catastrophic natural phenomena.

657

658 Code and data availability.





- The code and data are freely available to other researchers and consultants. The Python code for this method is accessible on
- 660 GitHub via the following link: <u>https://github.com/Deltares/CoastalHazardsToolkit</u>.

661

- 662 Author contributions.
- 663 KN and MvO developed the method and the outline for the manuscript. KN wrote the initial manuscript, with editorial 664 comments by JV, AvD, JA, T and DR.

665

- 666 *Competing interests.*
- 667 The authors declare that they have no conflict of interest.
- 668
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674 8 Appendices

675 8.1 Tidal calibration and validation

A tidal calibration was performed on the SFINCS computed tidal constituents compared to the tidal constituents at Beira. 676 677 Constituents with an amplitude of more than 5 cm (M2, S2, N2, K2, and K1) were adjusted in terms of amplitude (multiplication) and phase (addition). Amplitude changes varied between 0.84 and 1.07 while phase difference changed on 678 679 average by 40° . These calibration steps of adjusting the tidal constituents substantially reduced tidal errors at the Beira from a 680 MAE of 43 to 17 cm. Secondly, model skill in reproducing tidal amplitudes and phases is assessed at 7 tide stations across the 681 area of interest (including the calibration station of Beira). The SFINCS model reproduces tide with a median MAE of 21 cm, median RMSE of 25 cm, and median difference in M2 and S2 amplitude and phase of respectively -10 and -1 cm and -10 to -682 683 12° (median values computed over the different stations). Our hypothesis is that the reduction in tidal error observed at Beira throughout the calibration process might be due to a misalignment in the amplitudes and phases of the TPXO model which 684 685 were used to generate the tidal boundary conditions (see Section 3.1.2). Presumably, the bathymetry contributes to the error

686 observed in the validation process.

687

Table A1. Evaluation of model proficiency in replicating tides near the Sofala province. Stations are ordered south to north. Columns one and two present the Mean Absolute Error (MAE) and Root Mean Square Error (RMSE), respectively, as error metrics for the comparison between observed and simulated tidal time series. The final four columns display the discrepancy (Δ) in amplitude (A) and phase difference (ϕ) for the two most prominent tidal constituents in the area (M2 and S2), where Δ is calculated as the difference between observed and simulated values.

Name	MAE [m]	RMSE [m]	ΔM2 A [m]	ΔM2 φ [°]	ΔS2 A [m]	ΔS2 φ [°]
Bazaruto	0.13	0.15	-0.10	-7	0.01	-2
Bartolomeu Dias	0.12	0.15	-0.14	1	-0.11	-1
Chiloane	0.30	0.41	0.20	-10	0.08	-15
Beira	0.17	0.20	0.00	0	0.00	0
Chinde	0.21	0.25	-0.08	-13	-0.01	-12
Quelimane	0.26	0.32	-0.14	-15	-0.09	-21
Pebane	0.21	0.25	-0.14	-11	-0.09	-15
Median	0.21	0.25	-0.10	-10	-0.01	-12



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694 8.2 Additional figures for Section 5.2.2. 'Influence of simplification in TC-FF'

695 Figure 12, Figure 13 and Figure 14 provide additional information for Section 5.2.2. 'Influence of simplification in TC-FF'.

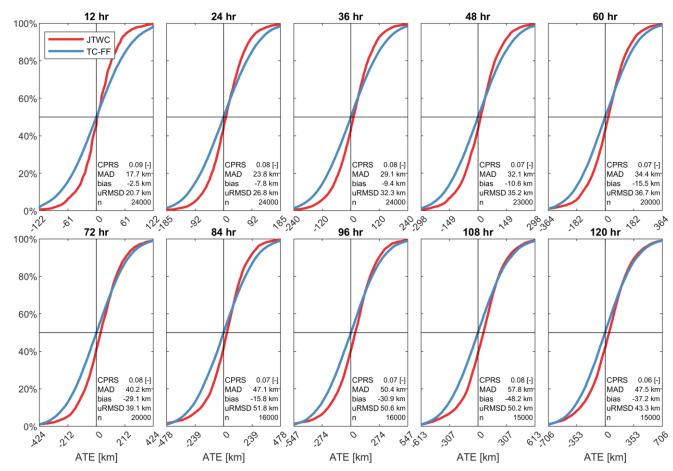
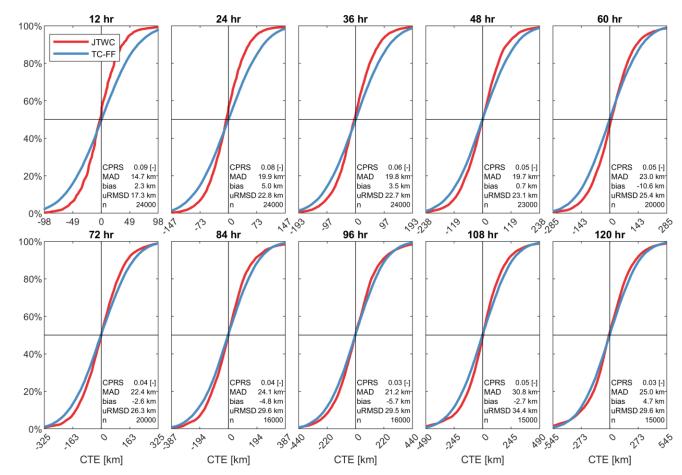


Figure 12. Comparison between the cumulative distribution function (CDF) of the along-track-error (ATE) for JTWC (red;
 reference) and TC-FF (blue; modeled). The different panels represent different lead times.





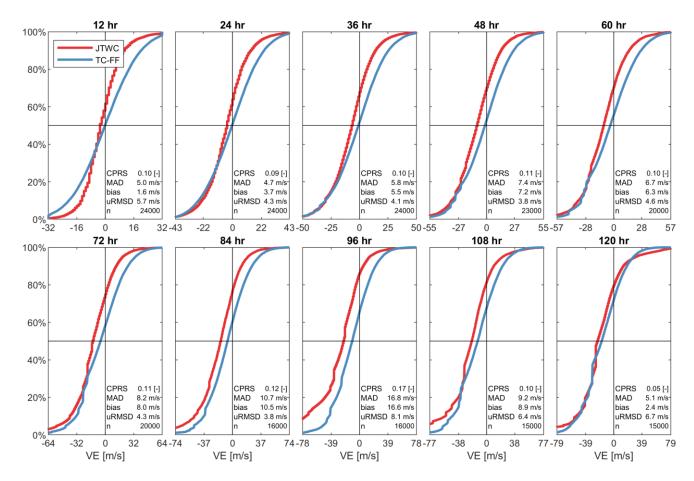


699

700 Figure 13. Comparison between the cumulative distribution function (CDF) of the cross-track-error (CTE) for JTWC (red; 701 reference) and TC-FF (blue; modeled). The different panels represent different lead times.







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Figure 14. Comparison between the cumulative distribution function (CDF) of the intensity error (VE) for JTWC (red; reference)
 and TC-FF (blue; modeled). The different panels represent different lead times.





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707 9 References

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