Comparing components for seismic risk modelling using data from the 2019 Le Teil (France) earthquake

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Abstract. Probabilistic seismic hazard and risk models are essential to improving our awareness of seismic risk, to its management, and to increasing our resilience against earthquake disasters. These models consist of a series of components, which may be evaluated and validated individually, although evaluating and validating these types of models as a whole is challenging due to the lack of recognised procedures. Estimations made with other models, as well as observations of damages from past earthquakes lend themselves to evaluating the components used to estimate the severity of damage to buildings. Here, we are using a dataset based on emergency post-seismic diagnoses made after the Le Teil 2019 earthquake, third-party estimations of macroseismic intensity for this seismic event, shake-map analyses, and scenario damage analyses to compare estimations under different modelling assumptions. First we select a rupture model using estimations of ground motion intensity measures and macroseismic intensity. Subsequently, we use scenario damage analyses based on different exposure models, including the aggregated exposure model in the 2020 European Seismic Risk Model (ESRM20), as well as different site models. Moreover, a "building-by-building" exposure model is used in scenario analyses, which models individually the buildings in the dataset. Lastly, we compare the results of a semi-empirical approach to the estimations made with the scenario analyses. The post-seismic diagnoses are converted to EMS-98 damage grades and then used to estimate the damages for the entirety of the building stock in Le Teil. In general, the scenario analyses estimate lower probabilities for damage grades 3-4 than the estimations made using the emergency post-seismic diagnoses. An exposure and fragility model assembled herein leads to lower probabilities for damage grades 3-5 than the ESRM20 exposure and fragility model, while the semi-empirical approach leads to even lower probabilities.

1   Introduction

Earthquakes are among the disasters with most severe consequences, which include loss of human life, disruption of critical infrastructures, insured and uninsured direct economic losses, as well as socio-technical impacts in multi-risk safety contexts. Assessments based on probabilistic seismic hazard and risk analysis (PSHA, PSRA) are key elements of efforts to improve awareness of seismic risk, response, and resilience to earthquakes. As far as seismic hazard and risk in Europe is concerned,

the 2020 European Seismic Hazard and Risk Models (ESHM20, ESRM20 - Crowley et al., 2021a; Danciu et al., 2021) are the state of the art models, which were created by the European Facilities for Earthquake Hazard and Risk consortium. The predictive accuracy of the multi-component ESHM20 and ESRM20 models, as that of all seismic hazard and risk models, and as that of all statistical and probabilistic models, needs to be evaluated, despite the fact that the individual components consisting them have already undergone evaluation.

In the nuclear industry, testing and evaluation of PSHA models and their components have been formalized in the form of Senior Seismic Hazard Analysis Committee (SSHAC) Hazard Studies (Ake et al., 2018). SSHAC projects aim to produce "technically defensible" distributions and probabilities of exceedance of ground motion intensity measures. Bommer et al. (2013) tested ground motion models and their logic tree by comparing their implementations by three independent teams of modellers. As far as the evaluation of PSHA logic trees is concerned, Marzocchi et al. (2015) argue that the hazard should be considered to be an ensemble of models, which do not need to be mutually exclusive and collectively exhaustive. Rood et al. (2020) used observations of geomechanical failures, i.e., rock toppling, to estimate upper limits of ground motion intensity measures and constrain hazard estimations for long return periods. Their procedure always leads to a reduction of the seismic hazard estimation, which depends on the model for the seismic fragility, i.e., the model estimating the probability of geomechanical failure conditioned on a ground motion intensity measure. Moreover, they proposed a procedure for dropping branches of the PSHA logic tree and reweighting the remaining. Gerstenberger et al. (2020) note that tests of national or regional hazard models are only meaningful at the level of the site, and that resorting to conversions of macroseismic intensity to ground motion intensity, when ground motion records are lacking, may introduce errors. Nevertheless, Mak and Schorlemmer (2016) did use such a conversion after testing the conversion equation itself.

In this study, to evaluate components used in seismic risk modelling, we use observations of damage in buildings in the municipality of Le Teil, France, caused by the 2019 Le Teil earthquake. First, in Section 2, we introduce a rule for converting post-seismic emergency diagnoses, which are based on a 3-level scale (i.e., a scale using the green, yellow, and red colour tags), into EMS-98 damage grades. Then, in Section 3, we detail the various assumptions and modelling choices that we aim to investigate as of components of the damage analysis chain, namely various source rupture models, building exposure models and ground motion models (GMMs) along with their site amplification models.

Subsequently, in Section 4, we do a series of comparisons based on ground motion intensity, macroseismic intensity, and damage distribution. For the comparison based on ground motion intensity, we generate samples for a set of ground motion intensity measures (IMs) estimated by scenario computations or shake-map methods (Wald et al., 2022), for rupture parameters reported by different sources. Shake-map analyses are employed due to their capability to take into account any available ground motion records or macroseismic observations in the interpolation of the estimated shaking. Subsequently, we convert the IMs to macroseismic intensities using different ground-motion intensity conversion equations (GMICEs). A third-party

65 macroseismic intensity estimation for the municipality of Le Teil, provided by detailed on-site investigations (Schlupp et al., 2022), is then used to select the rupture parameters that lead to the most compatible macroseismic intentensities, and which are used in the scenario damage calculations.

Finally, in Section 5, we perform three types of comparisons based on the damage to exposed buildings: (i) a comparison using
70 different risk analysis tools (Armagedom (Sedan et al. 2013), and the OpenQuake Engine (Pagani et al., 2014; Silva et al., 2014)), (ii) comparisons based on a "building-by-building model", and (iii) comparisons based on aggregated exposure models. In the last two types, we use alternative V_{S30} (the time-averaged shear-wave velocity up to a depth of 30 m) models to compare their effects on the estimated damages. The V_{S30} models used are the ESRM20 topography-based model, and a geology-based model specific to France (Roullé & Monfort, 2016). In addition to the V_{S30}, the slope and the geology are used to account for
75 ground motion amplification due to local site effects based on the KothaEtAl2020ESHM20SlopeGeology ground motion model. In the comparisons using aggregated exposure models, the exposure models used are the ESRM20 exposure, an aggregated exposure model based on French statistical data, and a "building-by-building" exposure model based on the field damage observations. The probabilities of the damages estimated based on the analyses are compared to the corresponding probabilities based on damage observations and expert judgement. The steps leading up to these comparisons are summarized
80 in Figure 1.
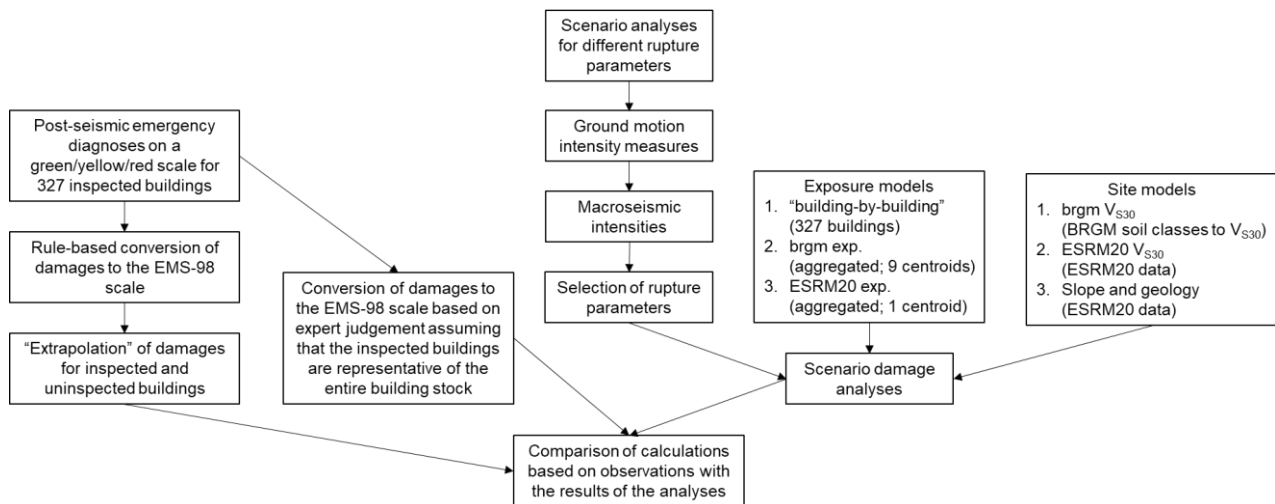
Figure 1 Overview of the steps leading up to the comparison of the different estimations of the damages

2 Seismological and damage data

2.1 Seismic hazard and information for 2019 Le Teil earthquake

The municipality of Le Teil is located in southeastern mainland France, a region that corresponds to low and moderate risk categories, according to the French Seismic Zonation. For Le Teil in particular, the ESHM20 estimates a mean Peak Ground Acceleration (PGA) of 0.04 g with a 0.21 % probability of exceedance in 1 year (475 years mean return period) on rock site conditions ($V_{s,30}$ = 800 m/s).

The Le Teil earthquake took place on the 11th of November 2019, and its epicentre is located at 44.518° N 4.671° E (Ritz et al., 2020), with a focal depth of 1 km and a magnitude Mw 4.9 (Ritz et al., 2020), in close proximity to the municipality of Le Teil and the town of Montélimar in the Lower Rhône valley in France. A private power plant accelerometer, located 15 km north-northeast of the epicentre, recorded PGA of 0.045 g (Schlupp et al., 2022), as the closest seismic station to the earthquake. Three stations of the French seismological and geodetic network (Résif / EPOS-FR) at 24-44 km from the epicentre recorded PGAs in the range of 0.004-0.007 g. These four stations are at such a distance from the epicentre and the municipality of Le Teil, so that they cannot accurately constrain the predicted IMs. Causse et al. (2021) used numerical modelling, including physics-based rupture modelling and modelling of near-fault wave propagation, and estimated near-fault PGAs with a 68 % confidence interval of 0.3-1.9 g in the fault projection on ground surface. They argued that their estimations are compatible with displacements of rigid block objects such as rocks and ledger stones. Moreover, they suggested that existing ground motion models may not be useful in the case of earthquakes such as this one, with a rarely observed shallow hypocentral depth, and with rupture parameters such as stress drop that are usually associated with earthquakes not only at larger depths, but of larger magnitudes too. However, it should be noted that some branches in the ESHM20 ground motion models logic tree should be able to account for the possibility of having extreme stress parameter values, by treating uncertainty in the stress drop as a source of epistemic uncertainty (Kotha et al., 2020; Weatherill et al., 2020). As far as the attenuation of the intensity of the PGA is concerned, the recorded value at 15 km was 0.04, which indicates a high attenuation probably due to the very shallow rupture: the Le Teil earthquake is a specific event, which generated very high large intensities right next to the epicentre, however the ground motion attenuated very quickly.

Schlupp et al. (2022) reported an EMS-98 macroseismic intensity of 7-8 for the municipality of Le Teil. This conclusion was the product of expert judgement considering the EMS-98 definitions of the intensity degrees and damage grades, the field observations from the Macroseismic Response Group, and the EMS-98 vulnerability classes of the buildings based on land registration data. Based on this procedure, Schlupp et al. (2022) determined 765 macroseismic intensities covering the area affected by the earthquake. The isoseist line of the map by Schlupp et al. (2022) for intensity 7 includes the built area of the Le Teil: given the limited spatial extent of this area, there is practically no spatial variation of the macroseismic intensity within this isoseist line, and the maximum is at the Le Teil (7.5).

2.2 Post-seismic emergency diagnoses dataset

We produced the dataset used here by processing post-seismic emergency inspection forms, and by completing and editing an existing dataset (Perez, 2020) for 501 inspected buildings. The inspection forms were filled in by the French Association of Earthquake Engineering (AFPS) during on-site inspections (Taillefer et al., 2021), which took place from the 3rd to the 5th of February 2020. Out of the 501 buildings, the produced dataset contains 327 entries with information about the coordinates of each inspected building, the number of storeys, the date of construction, and the description of damage for the entirety of each inspected building as well as for its structural and non-structural elements. The colour tags assigned by the post-seismic emergency diagnoses are on a three-level scale, i.e. green-yellow-red, which we converted to EMS-98 damage grades. The 174 entries that were not included in the produced dataset were left out due to the fact that, although they included the colour tag for the building, they lacked information with respect to the damage to the structural elements and the non-structural components.

For the conversion of the post-seismic emergency diagnoses in EMS-98 damage grades, we used the rules in Table 2-1. We defined these rules based on expert judgement, and they are based on the observed structural and non-structural damage, which are the criteria for classification according to the EMS-98 damage scale (Grünthal, 1998). Therefore, for this specific purpose, the essential data in the forms are the entries in the fields for the structural elements bearing vertical and horizontal loads (which were considered separately), and for the non-structural elements as well. The rest of the fields on the forms are related to procedures for life safety, e.g. evacuation, and they were not required for classifying damage according to the EMS-98. In this way, we used the raw information from the inspection forms to classify buildings according to structural damage and not whether a building was usable or not. In the cases where a given parameter is red, the damage grade is assigned irrespective of the other parameters. As far as the column "Types of elements" in Table 2-1 is concerned, the four components are ordered hierarchically. If both vertical and horizontal structural elements are red, then damage grade 5 is assigned, but if the horizontal structural elements are red and the vertical are yellow or green, then grade 4 is assigned. In the cases where everything is green, damage grade 1 is assigned (damage grade 1 corresponds to no structural damage and slight non-structural damage). This assignment is done based on our judgement. The dataset that we used contains only damage observations, which were made during inspections on request by the building owners. We consider that at least slight non-structural damage was the cause that led the owners to request an inspection of their building. The results of this reclassification (which involves the distribution of EMS-98 damage levels in the green, yellow, red tags) are presented in Table 2-2 for the entire dataset, independently of building typology.

Table 2-1 Proposed classification of the observed damage in the EMS-98 damage grades as a function of the colour tags assigned by the inspectors.

Type of elements	Colour tag: G (green), Y (yellow), R (red)																
Vertical load-bearing structural elements	R				Y	Y	Y	Y	G	G	Y	Y	G	G	G	G	G
Horizontal load-bearing structural elements		R			Y	Y	Y	Y	Y	Y	G	G	G	G	G	G	G
Internal non-structural elements			R		R	Y	R	Y	R	Y	R	Y	R	Y	Y	G	G
External non-structural elements				R	R	R	Y	Y	R	R	R	R	Y	R	Y	Y	G
EMS-98 damage grade	5	4	2	2	4	4	3	3	3	3	4	3	2	2	2	2	1

155 **Table 2-2 Percentage of buildings in each damage grade as a function of the building's final tag for the entire dataset**

Building tag	Damage grade	Count	Percentage (%)
Green	1	91	61
Green	2	22	15
Green	3	35	24
Yellow	3	95	90
Yellow	4	8	8
Yellow	5	2	2
Red	4	47	64
Red	5	27	36

In the following sections, we will compare results of analyses to three different sets of damage distribution that are based on the post-seismic emergency diagnoses. An overview of the estimation of the three different sets is given in Table 2-3. The first

160 set, labelled "DD1", consists of EMS-98 damage grades resulting from the conversion based on the post-seismic emergency diagnoses (with respect to the 327 inspected buildings), by applying the rules from Table 2-1. The damage distributions in "DD2" and "DD3" are estimated for the entirety of the 2778 buildings in Le Teil (according to the national statistics database): to this end, an "extrapolation" of DD1 is performed in order to account for the fact that only a part of the buildings in Le Teil have been inspected, by applying probabilities of damage grades given the inspection or not of the building.

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Table 2-3 Description of the different estimations of damage based on the observations

Observed Damage Data ID	Exposure resolution	Exposure data	Damage estimation method	Damage conversion method
DD1	Building-by-building (327 buildings)	AFPS emergency survey	Observations on 327 buildings (Green / Yellow/ Red tags)	Conversion to EMS-98 damage grades (Tab. 2-1)
DD2	Infra-municipality districts (2778 buildings)	National statistics database	Observations on 327 buildings (Green / Yellow / Red tags) + "Extrapolation"	Conversion to EMS-98 damage grades (Tab. 2-1) + Bias adjustment on total number of 2778 buildings (accounting for non-surveyed buildings)
DD3	Infra-municipality districts (2778 buildings)	National statistics database	Observations on 327 buildings (Green / Yellow / Red tags) + "Extrapolation"	Conversion to EMS-98 damage grades with expert judgment (Tab. 2-8)

170 The calculation of the probabilities of the damage grades for DD2 are given in Table 2-4 to Table 2-7. Table 2-4 includes the probabilities of the colour tags in the original dataset for 501 buildings. Table 2-4 also includes the probabilities of the damage

grades conditioned on the colour tags, which result from the conversion of the post-seismic emergency diagnoses. In Table 2-5, the total probabilities of the damage grades are calculated assuming that the probabilities of the damage grades conditioned on the colour tags are representative of the 501 buildings in the original dataset. Table 2-6 gives the damage grade probabilities conditioned on whether a building has been inspected. The first line of Table 2-6 includes the probabilities based on the damage observations, while the second line includes probabilities of the damage grades for the uninspected buildings, which were selected based on our judgement and our assumption that the damage grade probabilities for the buildings that have not been inspected are different, because the inspections were made upon owner request. The calculation of the total probabilities of the damage grades for inspected and uninspected buildings is given in Table 2-7. Given that these probabilities are practically the probabilities in Table 2-7 weighted by the probability of a building to have been inspected ($P(Insp. = False)$), they depend to a large degree on the probabilities for the uninspected buildings, because most of the buildings were not inspected ($P(Insp. = True) \gg P(Insp. = False)$).

As far as the probabilities for DD3 are concerned, they are calculated using the Table 2-8 in combination with the probabilities of the green/yellow/red tags (P(tag) in Table 2-4). In specific, they result if we take a 1-row vector of the values in P(tag) in Table 2-4, and do a matrix multiplication with the values in Table 2-8. This calculation differs from the calculation of the probabilities in DD2 in that it implies that the damage observations are representative of the damage over the entire town of Le Teil. This is implied by the fact that there is no conditioning on whether a building has been inspected.

Table 2-4 Probabilities of the damage grades conditioned on the colour tag assigned to a building that has been inspected during post-seismic emergency diagnoses

tag	n_buildings	P(tag)	P(DG1|tag)	P(DG2|tag)	P(DG3|tag)	P(DG4|tag)	P(DG5|tag)
Green	238	0.475	0.610	0.150	0.240	0.000	0.000
Yellow	157	0.313	0.000	0.000	0.900	0.080	0.020
Red	106	0.212	0.000	0.000	0.000	0.640	0.360

Table 2-5 Calculation of the total probability of the damage grades for buildings inspected during the post-seismic emergency diagnoses

tag	P(DG1|tag)·P(tag)	P(DG2|tag)·P(tag)	P(DG3|tag)·P(tag)	P(DG4|tag)·P(tag)	P(DG5|tag)·P(tag)
Green	0.290	0.071	0.114	0.000	0.000
Yellow	0.000	0.000	0.282	0.025	0.006
Red	0.000	0.000	0.000	0.135	0.076
Sum:	0.290	0.071	0.396	0.160	0.082

Table 2-6 Probabilities of the damage grades conditioned on whether a building has been inspected (the probabilities for inspected buildings are based on the damage observations, the probabilities for the uninspected buildings are based on expert judgement)

Inspected	P(Insp.)	P(DG1|Insp.)	P(DG2|Insp.)	P(DG3|Insp.)	P(DG4|Insp.)	P(DG5|Insp.)
TRUE	0.180	0.290	0.071	0.396	0.160	0.082
FALSE	0.820	0.500	0.300	0.100	0.050	0.050

Table 2-7 Calculation of the total probabilities of the damage grades accounting for both inspected and uninspected buildings

Inspected	P(DG1|Insp.)·P(Insp.)	P(DG2|Insp.)·P(Insp.)	P(DG3|Insp.)·P(Insp.)	P(DG4|Insp.)·P(Insp.)	P(DG5|Insp.)·P(Insp.)
TRUE	0.052	0.013	0.071	0.029	0.015
FALSE	0.410	0.246	0.082	0.041	0.041
Sum:	0.462	0.259	0.153	0.070	0.056

Table 2-8 Probabilities of EMS-98 damage grades conditioned on the building colour tag according to expert judgement

tag	P(DG1|tag)	P(DG2|tag)	P(DG3|tag)	P(DG4|tag)	P(DG5|tag)
Green	0.80	0.20	0	0	0
Yellow	0	0.40	0.60	0	0
Red	0	0	0.55	0.40	0.05

3 Modelling assumptions

3.1 Rupture models

Various ground-motion scenarios are generated for different assumptions of rupture models, which are detailed in Table 3-1. The scenarios are named after the source of the data for the magnitude and the hypocentre location, i.e., "CEA" (CEA/LDG, 2011; Duverger et al., 2021), "EMSC" (EMSC, 2019), "RENASS" (Schlupp et al., 2022), "Ritz et al." (Ritz et al., 2020) and "USGS" (USGS, 2019). The strike, dip, and rake angles of the focal mechanism solutions reported by "CEA" and "Ritz et al." are arbitrarily assigned to the scenarios "EMSC" and "RENASS", respectively. The surface of the rupture is estimated using the Wells and Coppersmith (1994) scaling relation, and the coordinates of the points defining the rupture geometry are calculated in order to be used in the OpenQuake Engine simulations and in the conversion of ground motion IMs to macroseismic intensity. In the case of the rupture model according to the parameters based on Ritz et al. (2020), the area of the rupture model is equal to 6.49 km^2. To calculate the coordinates of the corners of the rupture geometry, we assume that its geometric centroid is located at the hypocentre. This assumption leads in some cases to an upper rupture edge above ground

surface. This is amended by translating the rupture geometry on its plane so that its upper edge coincides with the fault trace on ground surface. The depths of the upper and lower edges of the rupture geometry are used to define in the Simple Fault model the upper and lower seismogenic depths, respectively. The coordinates of the ends of the trace of the fault on the ground surface required by the Simple Fault model are calculated by projecting the rupture geometry on the ground surface in the

225 direction of the dip. Moreover, a maximum rupture mesh spacing of 0.5 km is used, which leads to a 6 by 6 grid in all scenario analyses, which we consider sufficient.

Table 3-1 Rupture parameters associated with the five source models

Rupture model	M_W	Hypocentre longitude (°E)	Hypocentre latitude (°N)	Hypocentre depth (km)	Strike (°)	Dip (°)	Rake (°)
"CEA"	4.9	4.65	44.53	2.0	47	65	93
"EMSC"	4.9	4.62	44.57	10.0	47	65	93
"RENASS"	4.8	4.64	44.53	2.0	50	45	89
"Ritz et al."	4.9	4.671	44.518	1.0	50	45	89
"USGS"	4.84	4.638	44.612	11.5	53	57	99

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3.2 Exposure and fragility models

In the components, three different exposure models are considered in order to characterise the built area of Le Teil. A main distinction is made between aggregated models (i.e., distribution of building classes within a geographical unit) and models at

235 the level of single buildings.

The first aggregated exposure model ("ESRM20 exp."), which is based on the ESRM20 exposure (Crowley et al., 2019, 2020, 2021b), consists of a single area containing a total of 1679 residential buildings. This exposure model results from the simplification of the ESRM20 exposure model, by fusing similar building types with a small portion of the overall number of

240 buildings in the original ESRM20 exposure (Table A1) into 7 building classes (Table A2). Given that the original ESRM20 exposure includes classes with a small percentage of the total number of buildings, which could be grouped with similar classes, we opted for such mergers in order to reduce the total number of classes and simplifying the comparisons. For example, we decided to group in Class 1 (revised Table A1) buildings categories with 6 or more storeys, which have a small number of buildings, together with buildings with 3-5 storeys on the basis of the similarity of their load-bearing systems. The effect of

245 the simplification of the ESRM20 model is checked with an additional analysis using the original ESRM20 exposure and the corresponding fragility models.

The second aggregated exposure model ("brgm exp.") is based on national statistical data, and it includes 9 distinct areas (**Erreur ! Source du renvoi introuvable.**) with 2778 residential buildings. In this exposure model, the buildings are categorized in 12 ESRM20 classes (Table A3), which we selected based on the exposure model in Sedan et al. (2013).
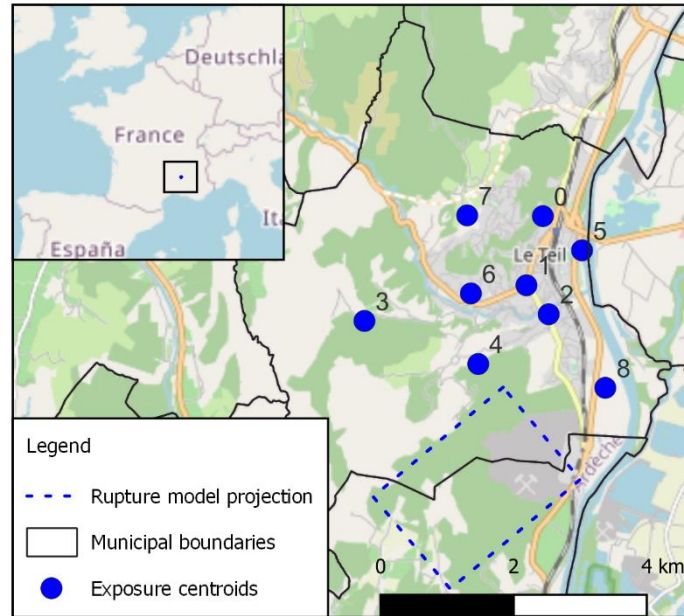
Figure 2 Location of the 9 exposure centroids in the BRGM exposure model and surface projection of the "Ritz et al." rupture model.

Finally, the "building-by-building" exposure model includes 327 buildings located at the coordinates of the buildings in the damage dataset DD1, for which the information in the dataset is sufficient for determining the building class and damage grade. In the simulations, the fragility model consists of fragility curves from the ESRM20, which we selected according to the information in the damage dataset. Initially, we defined building classes in terms of the GEM Building Taxonomy v3.0 (Silva et al., 2022) building class based on the building materials and the number of storeys (Table 3-2). Moreover, we assigned an EMS-98 vulnerability class according to the building material, the year of construction, as well as the building types in Le Teil and their vulnerability class reported by Schlupp et al. (2022). Based on the building type and the vulnerability class, we then selected fragility models from the ESRM20. It is noted that the lateral force coefficient in the selected building classes may not be selected based on the damage dataset. Moreover, it was not considered during the selection of the fragility models. An EMS-98 vulnerability class was assigned based on the year of construction, and then we selected fragility models, which we considered to be in agreement with the construction material and the EMS-98 vulnerability classes.

Table 3-2 Assigned GEM Building Taxonomy v3.0, ESM-98 vulnerability, and ESRM20 building classes for the buildings in the post-seismic emergency diagnoses dataset. The fragility curves in ESRM for the selected classes are function of the listed intensity measure types (IMT)

GEM Building Taxonomy v3.0 class	EMS-98 vuln. class	ERSM20 class	IMT	Number of buildings
MUR+STDRE/LWAL+DNO/HAPP:2	A	MUR-STDRE_LWAL-DNO_H2	$S_a(0.3s)$	124
MUR+STDRE/LWAL+DNO/HAPP:2	B-D	MCF_LWAL-DUL_H2	PGA	20
MUR+STDRE/LWAL+DNO/HAPP:4	A	MUR-STDRE_LWAL-DNO_H3	$S_a(0.6s)$	122
MUR+STDRE/LWAL+DNO/HAPP:4	B,D	MCF_LWAL-DUL_H3	$S_a(0.3s)$	6
CR/HAPP:2	C	CR_LFINF-CDL-10_H2	$S_a(0.6s)$	23
CR/HAPP:2	E-D	CR_LFINF-CDM-0_H1	$S_a(0.3s)$	2
CR/HAPP:4	C	CR_LFINF-CDL-15_H4	$S_a(1.0s)$	29
CR/HAPP:4	E	CR_LFINF-CDM-10_H1	$S_a(0.3s)$	1

3.3 Ground-motion models and site amplification

In order to generate the ground motion fields in the scenario analyses, we use two GMMs: "KothaEtAl2020Site", and "KothaEtAl2020ESHM20SlopeGeology", which were developed in the context of the development of the GMM "KothaEtAl2020ESHM20" (Weatherhill et al. 2020, GEM Foundation 2023a-b), which is used in the ESHM20. The GMMs KothaEtAl2020Site and KothaEtAl2020ESHM20SlopeGeology are based on site amplification modelling as a function of V_{S30} and as a function of slope and geology, respectively. The effect of the V_{S30} mapping on the estimated probabilities of the damage grade is investigated by using two different site models, which are described below.

The first site model ("brgm V_{S30}") uses a map of site classes assembled at BRGM for the French territory (Monfort and Roullé, 2016). This map of soil classes has then been converted into a V_{S30} map by taking the median value of each EC8 soil class. The resolution in the "brgm V_{S30}" model is based on a geological map at the 1/50000 scale. Vs30 values are then directly extract at the coordinates of the entries in the exposure model, i.e., the 9 centroids in the BRGM exposure model, the one centroid in the ESRM20 exposure model, or the 327 points in the "building-by-building" exposure model.

The second site model ("ESRM V_{S30}") uses the values of the V_{S30} at the coordinates of the exposure centroids, which are given by the "exposure" workflow in the "exposure to site tool" in the ESRM20. Based on this workflow, the value of V_{S30} is calculated at the coordinates of the exposure centroids by averaging over the polygon of the municipality of Le Teil (République Française, 2022). In the case of the "building-by-building" scenario analyses, the V_{S30} values for the "ESRM V_{S30}" model are obtained by using the "exposure to site tool" in the ESRM20, in which the "point" workflow is applied, which returns the V_{S30} value associated with the 30-arcsec cell, where the query points are found. It should be noted that these two

295 different ways to collect V_{S30} values at the centroids (weighted mean of V_{S30} values across the area versus punctual value at the centroid) may constitute an additional source of discrepancy, in addition to the initial differences between the two V_{S30} models.

The V_{S30} values from the two site models at the coordinates of the centroids in the "brgm exp." exposure model are compared
300 in Table 3-3. Both site models ("brgm V_{S30}" and "ESRM V_{S30}"), when used in combination with the exposure model "brgm exp.", consider one point for each exposure centroid, which has identical coordinates with its corresponding exposure centroid. The "brgm V_{S30}" model includes V_{S30} values corresponding to soft soils, while the lowest V_{S30} values in the "ESRM V_{S30}" model are typical of hard soil sites. The same applies to the V_{S30} values for the two site models, when they are used in combination with the ESRM20 exposure model (Table 3-4).

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Table 3-3 Site parameters in the site models "ESRM V_{S30}" and "brgm V_{S30}" used in combination with the BRGM exposure model (9 centroids)

Centroid	Latitude	Longitude	Region	brgm V_{S30} (m·s^{-1})	ESRM20 V_{S30} (m·s^{-1})	V_{S30} Type	Geology	Slope
0	44.5546	4.6835	1	800	807	inferred	CRETACEOUS	0.0823
1	44.5453	4.6804	1	270	831	inferred	CRETACEOUS	0.0645
2	44.5414	4.6846	1	270	730	inferred	HOLOCENE	0.0487
3	44.5405	4.6498	1	800	726	inferred	CRETACEOUS	0.0768
4	44.5347	4.6713	1	800	831	inferred	CRETACEOUS	0.0467
5	44.5500	4.6909	1	270	699	inferred	HOLOCENE	0.0160
6	44.5442	4.6699	1	800	830	inferred	CRETACEOUS	0.0522
7	44.5547	4.6692	1	580	840	inferred	CRETACEOUS	0.0503
8	44.5315	4.6953	1	270	644	inferred	HOLOCENE	0.0439

310 **Table 3-4 Site parameters in the site models "ESRM V_{S30}" and "brgm V_{S30}" used in combination with the ESRM20 exposure model (one centroid)**

Site ID	Latitude	Longitude	Region	brgm V_{S30} (m·s^{-1})	ESRM20 V_{S30} (m·s^{-1})	V_{S30} Type	Geology	Slope
0	44.54307	4.66441	1	270	834	inferred	CRETACEOUS	0.0304

4   Comparisons of estimated intensities

4.1   Comparison based on ground-motion parameters

Here we compare intensity measures of the seismic ground motion resulting from scenario analyses and one shake-map derivation. The scenario analyses are conducted for five different rupture models using the OpenQuake Engine (Pagani et al., 2014; Silva et al., 2014) and the ground motion model (GMM) "KothaEtAl2020Site", a version of the GMM by Kotha et al. (2020) with a polynomial site amplification as a function of the V_{S30}, which is available in the OpenQuake Engine. The geometries of the ruptures in the shake-map as well as in the scenario analyses are all modelled as "Simple Faults" of flat square geometry, each defined by the set of parameters in Table 3-1. As far as the shake-map for this scenario is concerned, it was re-calculated with the USGS ShakeMap v4 engine (Wald et al. 2022), using the rupture parameters according to Ritz et al. (2020) (i.e., "Ritz et al." model in Table 3-1), and it was constrained with ground motion measurements only (no "Did You Feel It" reports were used). However, the closest stations are over 15 km from the epicentre, which leads to practically no constraint.

To account for the uncertainty in the intensity of the ground motion, 1000 ground motion fields are generated, i.e. samples of IMs at the location of the 9 centroids of the aggregated exposure model (**Erreur ! Source du renvoi introuvable.**). The ground motion fields are generated for the IMs peak ground acceleration (PGA), spectral pseudo-acceleration at 0.3, 0.6, 1.0 and 3.0 s. Furthermore, the spatial correlation of the IMs is taken into account in the generation of the IM samples by using the Jayaram and Baker (2009) model in the OpenQuake Engine, assuming no clustering of the V_{S30} values in the study area. Moreover, the On the other hand, the shake-map estimates updated parameters of the lognormal distributions of the IMs (PGA, spectral acceleration at 0.3, 0.6, and 1.0 s) at the 9 centroids, which are then used to generate ground motion fields, i.e. 1,000 samples for each IM at each centroid, using R (R Core Team, 2023). For the generation of the samples, we use correlation models for the spatial correlation and the correlation between spectral accelerations at different periods (Baker and Jayaram, 2008; Jayaram and Baker, 2009), as in the analyses with the OpenQuake Engine, as well as the Nearest Positive Definite Matrix using the approach by Higham (2002) as implemented in the R package "Matrix" (Matrix package authors and Oehlschlägel, 2023).

Figure 3 shows box plots for the samples of the considered IMs, which were generated at the locations of the exposure centroids. For a specific IM, the median and the mean of the entirety of the samples for all centroids are represented by the line at the middle of the box and the point marker, respectively. The boundaries of a box mark the 1st and 3rd quartile, while the whiskers approximate the 95 % confidence interval. If we consider only the boxplots corresponding to the five scenario analyses, the dispersions of the samples are equivalent, as expected due to the use of the same GMM. However, the differences with respect to the means of these five IM samples have to be attributed to the differences between the epicentre locations, the depth of the hypocentre, and the focal solution, because these are the parameters affecting the distance between the exposure

centroids and the geometry of the rupture. Moreover, the means for the scenarios "EMSC" and "USGS" are consistently the lowest. We attribute this primarily to the hypocentral depths in these two scenarios (10.0 and 11.5 km), which are significantly larger those in the other three scenarios, leading to distances from the rupture between 10.0 and 25.0 km, when the corresponding distances in the other three scenarios are less than 5.0 km. Regarding the samples based on the shake-map derivation, the boxplot whiskers are relatively shorter than those for the five scenarios, signifying smaller dispersions of the IM logarithms. This difference should primarily originate from the differences between the GMMs in the shake-map configuration and in the scenario analyses.

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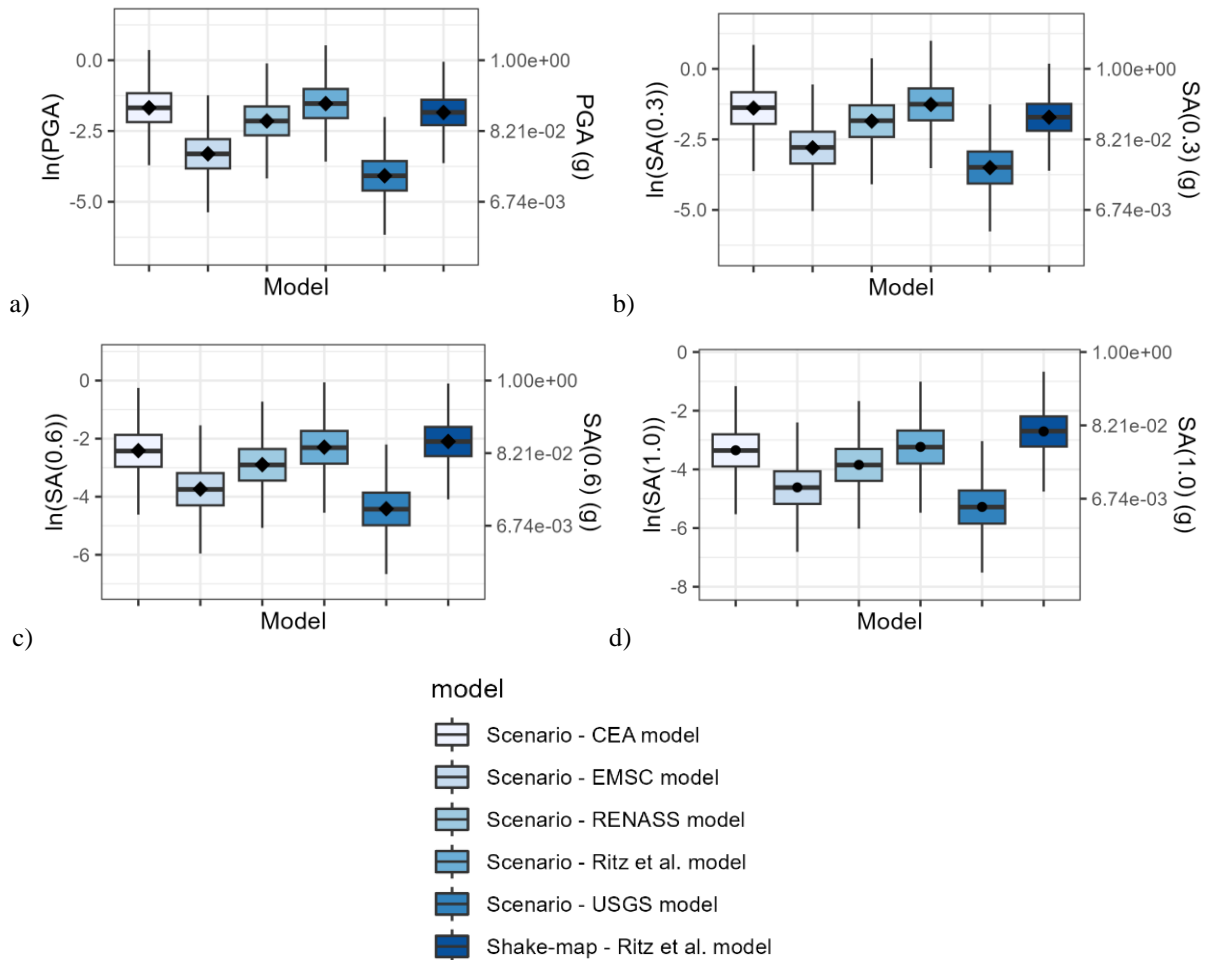


Figure 3 Ground motion intensity measures aggregated from all exposure centroids (the edges of the box are located at the first and third quartile, respectively, the line at the middle of the box is located at the median, the point marker is located at the mean of the sample, the whiskers extend up to 1.5 times the distance between the first and third quartile approximating the 95 % confidence interval).

4.2 Comparisons based on macroseismic intensity

365 The generated IM samples are subsequently converted to macroseismic intensities using GMICEs and they are compared with the macroseismic intensity reported by Schlupp et al. (2022). The aim of this comparison is to identify the rupture models leading to macroseismic intensities closest to the reported ones. Moreover, another motive for this comparison is the fact that it is difficult to compare the models with measured observations (i.e., recordings of seismic stations), since such measures are very sparse (the nearest station is around 15km from the epicentre). Therefore, in the absence of measures in the epicentral

370 area, it is difficult to compare the effects of different rupture distances in this area to measured ground-motions (this is where the relative differences in rupture distance are the largest, as they are greatly reduced further away from the epicentre). This is why we use macroseismic intensity (precise estimates obtained from field surveys) for the comparison. Two GMICEs are used for this comparison, which we consider compatible with the study area. These are the GMICEs by Faenza and Michelini (2010) ("FM2010", Equation 1) and by Caprio et al. (2015) ("CA2015", Equation 2).

375

$$MCS = a + b \cdot logIM + \sigma_{MCS}$$

Where MCS is the Mercalli-Cancani-Sieberg intensity, IM is PGA (in cm·s^{-2}) or PGV (in cm·s^{-1}), and σ_{MMI} is the model's standard deviation.

380

$$INT = a + b \cdot logIM + \sigma_{singleline}$$

Where INT is a combination of the Modified Mercali Intensity (MMI) and the Mercalli-Cancani-Sieberg intensity (MCS), IM is the ground motion intensity measure, i.e., PGA (in cm·s^{-2}) or PGV (in cm·s^{-1}), a and b are the model's parameters, and

385 $\sigma_{singleline}$ is the model's standard deviation. The CA2015 model is bilinear and its parameters are found in Table 4-1, while the FM2010 is the single line model in Faenza and Michelini (2010), whose parameters are found in Table 4-2. To account for model uncertainty during the conversions with Eq. 1-2, random residuals were generated from zero-centred normal distributions with the corresponding standard deviation and added to the means given by the equations.

390 **Table 4-1 Parameters used in the implementation of the CA2015 model**

IM type	IM range	a	b	σ_x
PGA (cm·s^{-2})	$\log_{10}(IM) < 1.6$	2.270	1.589	0.6
	$\log_{10}(IM) \geq 1.6$	-1.361	2.671	0.5
PGV (cm·s^{-1})	$\log_{10}(IM) < 0.3$	4.424	2.270	0.4
	$\log_{10}(IM) \geq 0.3$	4.018	3.82	0.4

Table 4-2 Parameters used in the implementation of the FM2010 model

IM type	a	b	$\sigma_{singleline}$
PGA (cm·s^{-2})	1.68	2.58	0.35
PGV (cm·s^{-1})	5.11	2.35	0.26

395   Figure 4 shows the boxplots for the MCS and the INT, respectively, which result from the conversion of the IM samples. Despite the fact that the MMI and MCS have differences, we adopt here the guidelines by Musson et al. (2010), which take the two scales as equivalent (to each other and to the EMS-98 scale) up to intensity 10. We make this assumption to distinguish the effects of the employed GMICEs on the distributions of the generated samples of macroseismic intensities in Figure 4 from the differences due to the underlying hazard model components.

400

In order to assess the usefulness of the distribution for each scenario in Figure 4, we are using the 7.5 EMS-98 intensity estimated by Schlupp et al. (2022) for the municipality of Le Teil. The MCS distributions resulting from the FM2010 model, whose median is closer to the 7.5 observation-based estimation, are those for the "CEA", "RENASS", and "Ritz et al." scenarios, and the shake-map derivation. As far as the application of the CA2015 model (Figure 4b) is concerned, it leads to

405   macroseismic intensity distributions with larger dispersions and lower medians with respect to the FM2010 (Figure 4a) in the cases considered. In the cases examined here, the distributions whose median is closest to the 7.5 observation-based estimation, are those from the scenarios "CEA", "RENASS", and "Ritz et al." and from the shake-map.

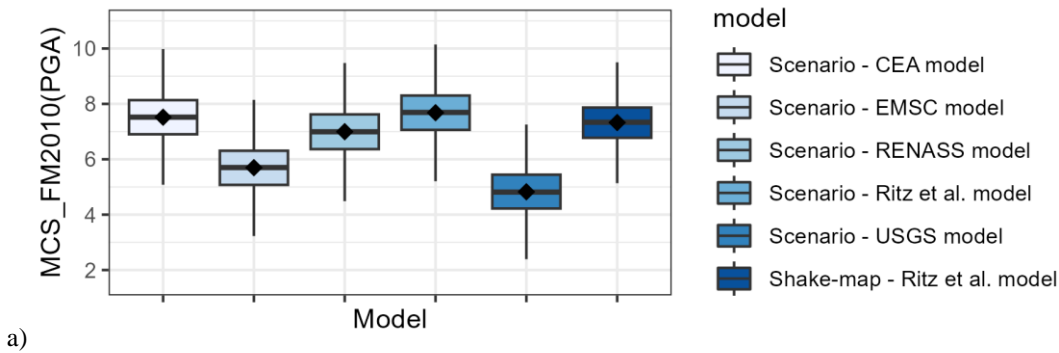
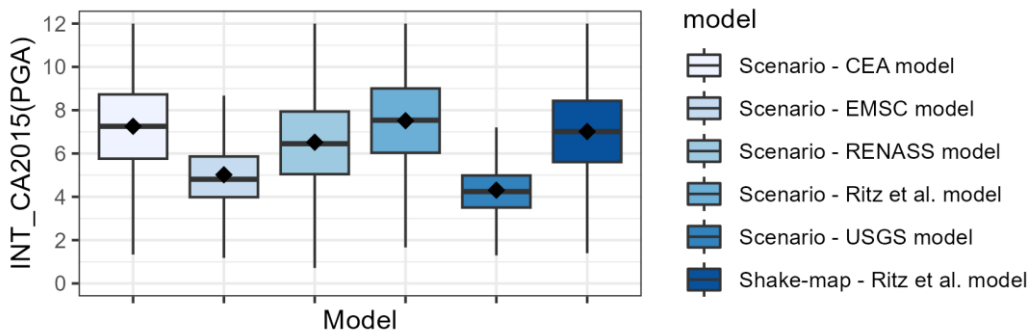
a)

410 b)

Figure 4 Boxplots for a) the Mercalli-Cancani-Sieberg (MCS) macroseismic intensity as a function of the PGA given by the ground motion-to-intensity conversion equation by Faenza and Michelini (2010) (FM2010), and b) the macroseismic intensity (INT) as a function of the PGA given by the ground motion-to-intensity conversion equation by Caprio et al. (2015) (CA2015) (the edges of the box are located at the first and third quartile, respectively, the line at the middle of the box is located at the median, the point marker is located at the mean of the sample, the whiskers extend up to 1.5 times the distance between the first and third quartile approximating the 95 % confidence interval).

5 Comparisons of estimated damages

5.1 Estimated damage based on a "building-by-building" exposure model

420 First, we perform "scenario damage" analyses using the OpenQuake Engine and the "building-by-building" exposure model, which includes 327 buildings. The ground motion fields in the analyses are generated using four different configurations (Table 5-1), which include the two different GMMs, i.e. "KothaEtAl2020Site", and "KothaEtAl2020ESHM20SlopeGeology", and two different site models, i.e. "brgm V_{S30}" and "ESRM V_{S30}". In all cases, the rupture is modelled according to the "Ritz et al." scenario (Table 3-1). A scenario analysis is also performed using as input ground motion fields generated from the shake-

425 map procedure described in Section 4.1 ("GM4" in Table 5-1).

Table 5-1 The configurations (GM Map IDs) used to generate the ground motion fields in the scenario damage analyses based on a building-by-building exposure model

GM Map ID	Type	GMM	Site model	Rupture model	Observations
GM1	ground-motion field	KothaEtAl2020Site	BRGM soil classes to Vs30	Ritz et al.	No
GM2	ground-motion field	KothaEtAl2020ESHM20 SlopeGeology	Slope & Geology (ESRM20 data)	Ritz et al.	No
GM3	ground-motion field	KothaEtAl2020Site	ESRM20 Vs30 data	Ritz et al.	No
GM4	shake-map	KothaEtAl2020Site	BRGM soil class to Vs30	Ritz et al.	Seismic stations

430

Figure 5 gives the distribution of the damage grades and the corresponding number of buildings based on the analyses. First, it is worth noting that the GM4 simulation leads to similar, but somewhat lower probabilities for the damage grades 3-5 than the GM1 simulation. GM1 and GM4 use the same GMM and site model, the difference lies in the fact that the GM4 uses ground motion fields based on a shake-map analysis. The main drivers of the probabilities of the damage grades are the buildings in the classes MUR-STDRE_LWAL-DNO_H2 and MUR-STDRE_LWAL-DNO_H3, which include 38 % and 37 %, respectively, of the total number of buildings in the model. These two classes are also the most vulnerable among the classes in the model, as indicated by the fact that they were classified in the EMS-98 vulnerability class A. The fragility curves of these two building classes are functions of $S_a(0.3s)$ and $S_a(0.6s)$, respectively. Based on the results in Figure 3, we consider that the $S_a(0.3s)$ is on average higher in the analysis "Scenario – Ritz et al. model" (GM1) than in "Shake-map – Ritz et al. model" (GM4), and that there are no significant differences between the two with respect to the $S_a(0.6s)$. This is the factor to which we attribute the differences in the probabilities of the damage grades based on the simulations GM1 and GM4.

The GM3 analysis leads to the lowest probabilities for the damage grades 3-5 amongst all computations in Figure 5. In this simulation, 68 % of the buildings are located on sites with $V_{S30} \geq 800\ m \cdot s^{-1}$, while in GM1 72 % of the buildings are on sites with $V_{S30} \leq 360\ m \cdot s^{-1}$, which is expected to lead to higher ground motion intensities due to site amplification. It interesting to note that the GM2 analysis, which uses the KothaEtAl2020ESHM20SlopeGeology GMM, gives results which are practically halfway between the results of the analyses GM1 and GM3.

Figure 5 also includes the estimation DD1 of actual damages, which is based on our conversion of the damage observation (Table 2-3). For damage grades 4 and 5, there are significant differences between the probabilities based on DD1 and the probabilities based on the scenario analyses and the shake-map analysis (GM1-4), however, they are not as important as the differences in the case of the damage grades 2 and 3. We presume that the rule that we used for the translation of the damage observations to damage grades (Table 2-1) is the source of these discrepancies. Moreover, DD1 leads to a distribution in Figure 5 that has an unusual "valley" for damage grade 2. The proposed mapping of damage observations assigns damage grade 3, when the vertical or the horizontal structural elements have a yellow tag. We believe that a yellow tag with respect to the structural elements signifies moderate structural damage, hence damage grade 3. The fact that in these cases a green tag was assigned, perhaps indicates that a further inspection took place, which either reclassified the damage as green structural damage, or as yellow non-structural damage. Such cases could be taken into account by a future refinement of the proposed mapping scheme.

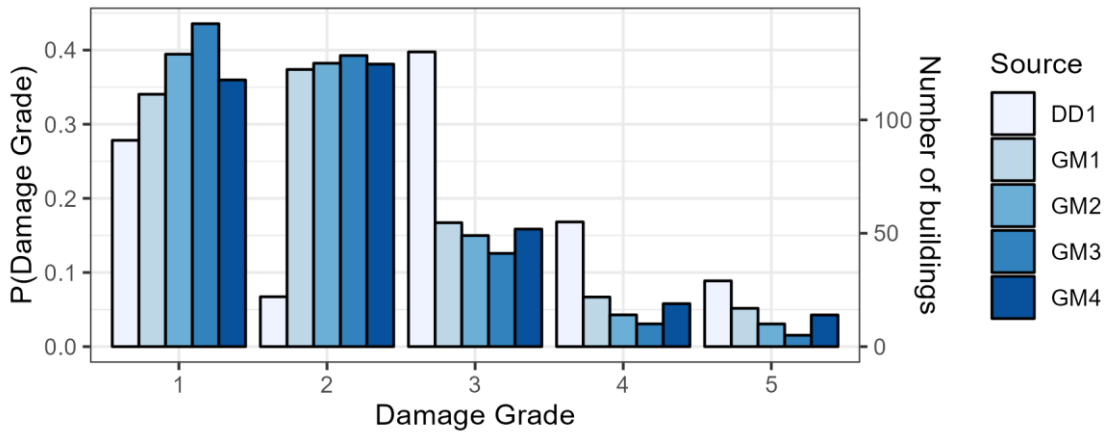



Figure 5 Distribution of the damage grades based on the calculations with the building-by-building exposure model compared with the DD1 estimation of actual damages

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5.2 Estimated damage based on aggregated exposure models

In addition to the "building-by building" analyses, we perform a series of "scenario damage" analyses with the two aggregated exposure models that include the total number of residential buildings in the municipality of Le Teil. In the analyses with the

470 aggregated exposure models, the ground motion intensity measures are modelled with nine different combinations of GMMs, site models and exposure models, as shown in Table 5-2. The damages based on the "scenario damage" analyses are firstly calculated on the damage scale of the ESRM20 fragility models, and then they are converted to the ESM-98 damage scale using as criterion the structural damage according to Table A4.

475

Table 5-2 Combinations of ground motion map IDs with the exposure models for each damage scenario ID

Damage scenario ID	GM Map ID	Exposure model
DS1	GM1	BRGM exposure
DS2	GM1	ESRM20 exposure
DS3	GM2	BRGM exposure
DS4	GM2	ESRM20 exposure
DS5	GM3	BRGM exposure
DS6	GM3	ESRM20 exposure
DS7	GM4	BRGM exposure
DS8	GM4	ESRM20 exposure
DS9	GM3	Original ESRM20 exposure

In Figure 6, we may see the effect of the different exposure models on the distribution of the damage grades and on the corresponding number of buildings. **Erreur ! Source du renvoi introuvable.**a includes the distributions of the damage grades for the damage scenarios DS5, DS6, and DS9, which all use the same rupture model, GMM, and site model. Compared to DS5, the DS6 calculation for the ESRM20 exposure leads to somewhat higher probabilities for damage grades 3-5. The differences between DS5 and DS6 are due to the use of the BRGM or ESRM20 exposure model, respectively. Moreover, Figure 6a includes the results of damage scenario DS9, which uses the original ESRM20 exposure and fragility model to check the effect of the simplification of the ESRM20 exposure and fragility models. By comparing the results between DS6 and DS9, we conclude that the simplification has a minor effect on the results. Figure 6a also includes our estimations DD2 and DD3, which utilize the damage observations and expert judgement, respectively (as explained in Section 2.2). The probabilities of the damage grades 3 and 4 calculated by the damage scenario analyses DS5, DS6, and DS9 are lower than the DD2 and DD3 estimations. However, for damage grade 5, the results of the damage scenarios are found in the range between the DD2 and DD3 estimations.

The numbers of buildings in Figure 6b are calculated by multiplying the total number of buildings in the exposure model by the probabilities in Figure 6a. In the case of the DD2 and DD3 estimations, we chose to calculate the number of buildings by multiplying with the number of buildings in the BRGM exposure model. Despite the difference in the total number of buildings in the BRGM and in the ESRM20 exposures (2778 versus 1679), the results of the damage scenarios for damage grades 3-5 present minor differences.

a)

b)

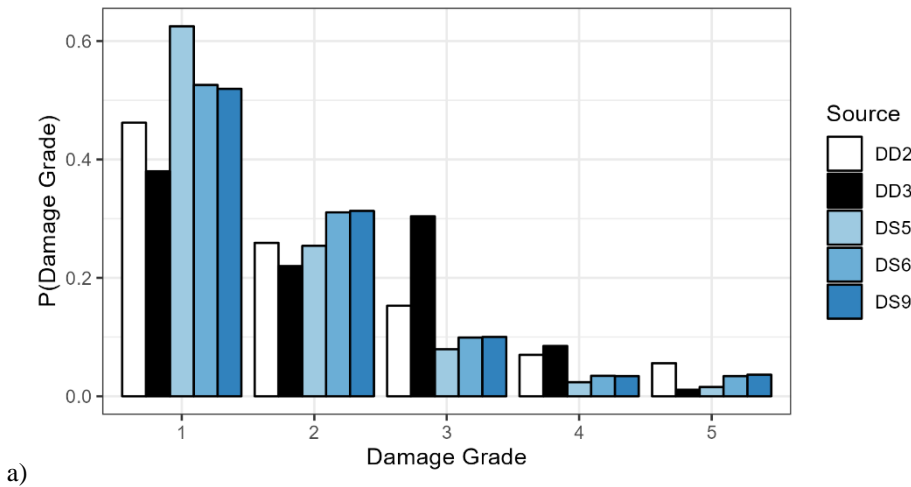
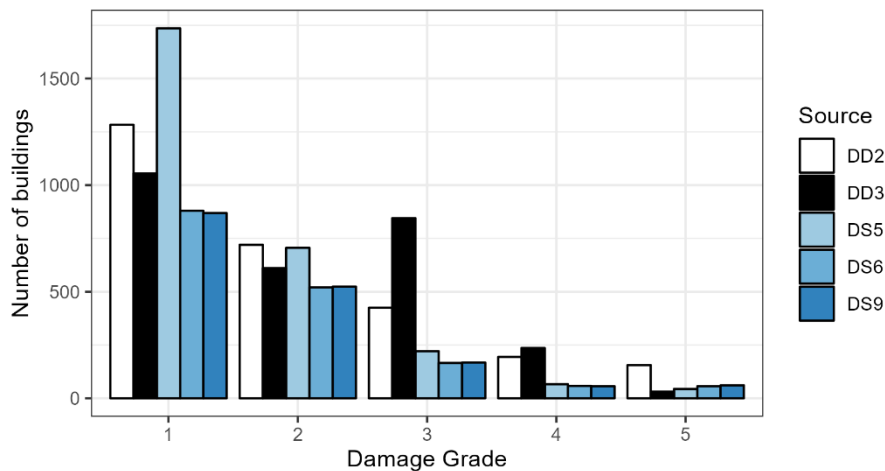
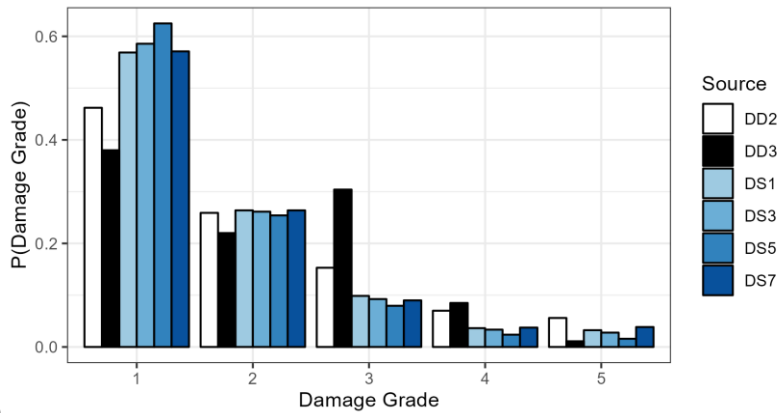
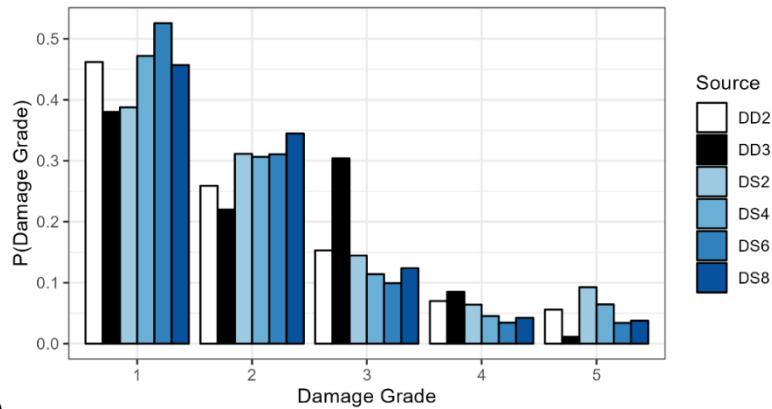



Figure 6 Effect of the exposure model on the a) probabilities and b) number of buildings per EMS-98 damage grade for the analyses with an aggregated exposure including the total number of buildings in Le Teil

The comparison with respect to the site amplification models is done using the damage scenario analyses DS1, DS3, DS5, DS7 (Figure 7a), where the same exposure model is used, i.e., the BRGM exposure model, but each time we use one of the four different GM Maps (GM1 to GM4 in Table 5-1 and Table 5-2). The effect of using the BRGM V_{S30} model instead of the ESRM20 V_{S30} model may be seen by comparing DS1 with DS5. The probabilities of the damage grades 2-5 are slightly lower in the scenario DS5. This may be explained by the fact that the V_{S30} values are higher in GM3 than in GM1, however we would expect more important differences considering the differences in the V_{S30} values shown Table 3-3. The damage grade probabilities in the scenario DS5, which uses the KothaEtAl2020ESHM20SlopeGeology GMM, are between the results for DS1 and DS3 for all damage grades. As far as the results based on DS7, which uses a shake-map analysis, they present small differences from those from DS1, which is reasonable considering that they use the same ground motion and site model, and that the updating based on ground motion observations in the shake-map analysis is minor. The results based on the ESRM20 exposure and fragility model (Figure 7b) show a similar image with the exception of the difference between the DS2 and DS8. Again, DS2 and DS8 use the same ground motion and site model, so the origin of this difference may be the consideration of observations in the shake-map analysis used by DS8.

a)

b)

520 **Figure 7 Effect of the GMM and site model on the probabilities of EMS-98 damage grade for the analyses with a) the BRGM and b) the ESRM20 aggregated exposure models including the total number of buildings in Le Teil**

5.3   Estimated damage based on a semi-empirical vulnerability approach

525   For the comparison with respect to the distribution of damages according to different calculation methodologies, we compare the estimated damages using the seismic risk analysis tool *Armagedom* (Sedan et al. 2013), running on the VIGIRISKS platform (Negulescu et al. 2023), with an estimation made with the DS1 scenario analysis with the OpenQuake Engine.

The Armagedom tool implements the semi-empirical macroseismic method developed by the RISK-UE project (Lagomarsino
530   and Giovinazzi, 2006). In contrast to the scenario calculations with the OpenQuake Engine, where 1,000 ground motion realizations are used to account for ground motion uncertainty, the analysis with Armagedom uses a third-party pre-calculated map of macroseismic intensity (Schlupp et al., 2022). The semi-empirical macroseismic method applied by Armagedom calculates the mean EMS-98 damage grade as a function of the macroseismic intensity and two parameters, i.e. the vulnerability

and the ductility index. These indices have been assigned to building classes in the exposure model used for the calculation using Armagedom based on criteria such as the material and the year of construction (Sedan et al. 2013). Subsequently, the semi-empirical macroseismic method applied in Armagedom assumes a binomial distribution to calculate the probabilities of exceeding the EMS-98 damage grades as a function of macroseismic intensity. On the other hand, the OpenQuake Engine uses ground motion realizations in combination with fragility curves to generate realizations of damages. Despite the differences between the two methods, it is worth comparing their results in the case of an earthquake scenario, if one considers that damage assessment is an objective they have in common.

The estimated distribution of buildings in each damage grade based on the two analyses is given in Figure 8, along with the distribution from the damage datasets DD2 and DD3. The percentage of buildings with Heavy and Very Heavy damage is 1.1 % and 0.0 % with Armagedom, and 3.7 % and 3.3 % with the OpenQuake Engine, respectively. Both the DS1 and the Armagedom calculation lead to estimations for damage grades 3 and 4, which are lower than the estimations DD2 and DD3. As far as damage grade 5 is concerned, the DS1 calculation estimates a probability of 3.3 % which lies between the DD2 and DD3 estimations, i.e., 5.6 % and 1.1% respectively. On the other hand, the Armagedom analysis globally underestimates damages when compared to the DS1 analysis. It should be noted that DS1 is based on the GM1 map, which corresponds to macroseismic intensity ranges (see Figure 4) that are well in line with the estimates by Schlupp et al. (2022), i.e. intensity around 7.5. Therefore, differences between DS1 and Armagedom may be mostly attributed to the different methods of damage estimation, i.e. the conversion between building vulnerability classes and corresponding fragility functions.

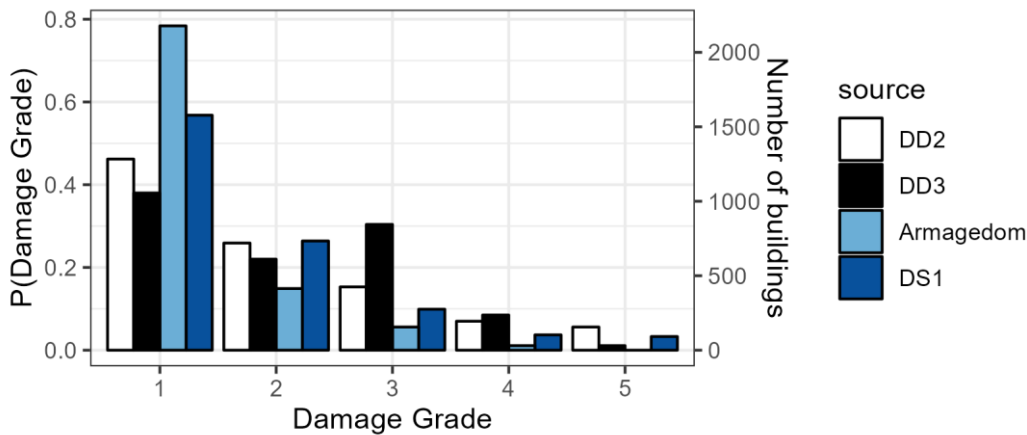



Figure 8 Estimation of damage grades using the Armagedom tool compared to the estimations DD2-DD3 and the results of the DS1 analysis.

6 Conclusion

Using simulations of earthquake scenarios and shake-map analyses, we conducted comparisons based on ground motion intensity, and comparisons based on the estimated number of damages based on components of the ESRM20. Moreover, we produced a dataset of 327 entries containing damage on the EMS-98 scale based on emergency post-seismic diagnoses on a 3-level (red/orange/green) scale, which were made after the Le Teil 2019 earthquake. The damage on the EMS-98 scale in the dataset is the result of a conversion based on a proposed rule, which considers structural and non-structural damage. The produced dataset was also used to make estimations for the entirety of the residential building stock in Le Teil.

Based on scenario analyses using the OpenQuake engine, as well as shake-map analyses, we calculated the ground motion intensity at a series of points of interest on the town of Le Teil, and then we converted the ground motion intensity to macroseismic intensity. This was done for different models of the earthquake rupture to select the model to be used in subsequent damage scenarios.

The damage scenarios used different ground motion models, site models, and exposure and fragility models to study the effect of these modelling assumptions. The GMMs used are KothaEtAl2020Site and KothaEtAl2020ESHM20SlopeGeology, while the site models include a site model based on V_{S30} values based on a map of site classes produced by the BRGM, and a site model based on the ESRM20. As far as the exposure models are concerned, they include the BRGM exposure for Le Teil, a model based on French national statistical data, and the ESRM20 exposure.

The scenario damage analyses leads to higher probabilities for damage grades 3-5 based on the ESRM20 than based on the BRGM exposure and fragility model. The number of buildings is also higher based on the ESRM20 model despite the smaller total number of buildings. Furthermore, the damage scenarios using the ESRM20 exposure and fragility model are overall in better agreement with the estimations (DD2, DD3) for the entire building stock using the dataset based on the emergency post-seismic diagnoses. In general, the scenario damage analyses estimate lower probabilities for damage grades 3-4 than the DD2 and DD3 estimations using the damage dataset, while they are in better agreement in the case of damage grade 5. The estimation based on the Armagedom tool, it gives probabilities of damage grades 3-5 which are even lower than those based on the damage scenario using the BRGM exposure and fragility model. As far as the ground motion and site models are concerned, the damage grade probabilities based on the KothaEtAl2020ESHM20SlopeGeology model lead in general to results between those obtained with KothaEtAl2020Site in combination with the BRGM and the ESRM20 site model. This is observed in the scenario analyses with the "building-by-building" and the aggregated exposure models.

At this point it is worth referring to the difficulties, limitations, and challenges related to the presented comparisons. A first and obvious one is the conversion of emergency post-seismic diagnoses to ESM-98 damage grades. The proposed rule that

uses the red/orange/green tags for structural and non-structural elements may have a significant effect on the damage grades resulting from the conversion, although we did not study the effect of possible alternative conversion rules. We acknowledge that the proposed rule can be refined, especially if we consider the valley in damage grade 2 in our conversion. One refinement could be a probabilistic rule which would return damage grade probabilities instead of a single value for the damage grade as a function of the colour tags for structural and non-structural elements. Moreover, the fact that there is a need for this conversion leads us to recommend to future post-seismic surveys to record damage observations on the EMS-98 scale instead or in addition to the typical 3-level scale.

A challenge in comparing the results of damage scenario analyses with damage observations is the extrapolation from observed damage over a sample of buildings to the entirety of the building stock. Moreover, we believe that the buildings that were included in the emergency post-seismic inspections in Le Teil are not a representative sample of the entire building stock. We presume that this could be true in other cases too. Not only buildings in Le Teil were inspected upon request, but we believe that undamaged or completely destroyed buildings were not inspected, because that would be meaningless in emergency post-seismic diagnoses, which aim to inform about the risk associated with the use of impacted buildings. Therefore, in order to extrapolate from the sample of inspected buildings to the entire stock one may consider resorting to other means, e.g. remote sensing, or envision future solutions such as rapid damage assessments based on numerous pre-installed low-cost sensors.

7   Author contribution

KT: Conceptualization, Data curation, Formal analysis, Investigation, Methodology, Software, Validation, Visualization, Writing – original draft preparation, Writing – review & editing

PG: Conceptualization, Data curation, Formal analysis, Investigation, Methodology, Software, Supervision, Validation, Writing – original draft preparation, Writing – review & editing

CN: Conceptualization, Data curation, Funding acquisition, Methodology, Project administration, Supervision, Writing – original draft preparation, Writing – review & editing

HC, LD: Writing – review & editing

8   Competing interests

The authors declare that they have no competing interests.

9 Code/Data availability

Code and data are available upon request.

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Table A1 Selected ESRM20 fragility classes based on the building types in Le Teil according to the ESRM20

Original ESRM20 type	N. buildings	Selected ESRM20 frag. class	# class
CR+PC/LWAL+CDN/HBET:3-5	53	CR_LDUAL-DUL_H4	1
CR/LDUAL+CDL+LFC:4.0/HBET:3-5	7	CR_LDUAL-DUL_H4	1
CR/LDUAL+CDM+LFC:4.0/HBET:3-5	3	CR_LDUAL-DUL_H4	1
CR/LDUAL+CDL+LFC:4.0/HBET:6-	3	CR_LDUAL-DUL_H4	1
CR/LDUAL+CDN/HBET:6-	2	CR_LDUAL-DUL_H4	1
CR+PC/LWAL+CDN/HBET:6-	1	CR_LDUAL-DUL_H4	1
CR/LDUAL+CDM+LFC:4.0/HBET:6-	1	CR_LDUAL-DUL_H4	1
CR/LFINF+CDL+LFC:4.0/H:1	76	CR_LFINF-CDL-10_H2	2
CR/LFINF+CDL+LFC:4.0/H:2	67	CR_LFINF-CDL-10_H2	2
CR/LFINF+CDM+LFC:4.0/H:1	42	CR_LFINF-CDM-10_H2	3
CR/LFINF+CDM+LFC:4.0/H:2	37	CR_LFINF-CDM-10_H2	3
CR/LFINF+CDN/HBET:3-5	38	CR_LFINF-CDL-15_H4	4
CR/LFLS+CDN/HBET:6-	9	CR_LFINF-CDL-15_H4	4
MUR+CL/LWAL+CDN/H:2	378	MUR-CL99_LWAL-DNO_H2	5
MUR+ST/LWAL+CDN/H:2	130	MUR-CL99_LWAL-DNO_H2	5
MUR+CL/LWAL+CDN/H:1	690	MUR-CL99_LWAL-DNO_H1	6
W/LWAL+CDN/H:1	100	W_LFM-DUL_H2	7
W/LWAL+CDN/H:2	43	W_LFM-DUL_H2	7

735

Table A2 Summary of the exposure based on the European Exposure model for the municipality of Le Teil

#	Selected ESRM20 class	N. of buildings
1	CR_LDUAL-DUL_H4	70
2	CR_LFINF-CDL-10_H2	143
3	CR_LFINF-CDM-10_H2	78
4	CR_LFINF-CDL-15_H4	46
5	MUR-CL99_LWAL-DNO_H2	508
6	MUR-CL99_LWAL-DNO_H1	690
7	W_LFM-DUL_H2	143

Table A3 Summary of the BRGM exposure model for the municipality of Le Teil

#	Selected ESRM20 class	Number of buildings
1	CR_LFINF-CDL-10_H1	296
2	CR_LFINF-CDL-10_H2	138
3	CR_LFINF-CDL-15_H2	348
4	CR_LFINF-CDL-15_H3	631
5	CR_LFINF-CDL-15_H4	12
6	CR_LFINF-CDM-0_H1	27
7	CR_LFINF-CDM-10_H1	8
8	MCF_LWAL-DUL_H2	127
9	MCF_LWAL-DUL_H3	278
10	MUR-STDRE_LWAL-DNO_H1	130
11	MUR-STDRE_LWAL-DNO_H2	483
12	MUR-STDRE_LWAL-DNO_H3	300

740

Table A4 Conversion of the damage scale of the ESRM20 fragility models to the EMS-98 damage scale used for the comparisons

ESRM20	EMS-98
D0 no damage (combined structural and non-structural damage) [implied damage state]	Grade 1: Negligible to slight damage (no structural damage, slight non-structural damage
D1 slight (combined structural and non-structural damage)	Grade 2: Moderate damage (slight structural damage, moderate non-structural damage
D2 moderate (combined structural and non-structural damage)	Grade 3: Substantial to heavy damage (moderate structural damage, heavy non-structural damage)
D3 extensive (combined structural and non-structural damage)	Grade 4: Very heavy damage (heavy structural damage, very heavy non-structural damage)
D4 complete (combined structural and non-structural damage)	Grade 5: Destruction (very heavy structural damage)