



1 A landslide runout model for sediment transport, landscape 2 evolution and hazard assessment applications

3 Jeffrey Keck^{1,2}, Erkan Istanbuluoglu², Benjamin Campforts³, Gregory Tucker^{4,5}, Alexander
4 Horner-Devine²

5 ¹ Washington Department of Natural Resources, Forest Resources Division, Olympia, WA, USA

6 ² University of Washington, Civil and Environmental Engineering, Seattle, WA, USA

7 ³ Institute of Arctic and Alpine Research, University of Colorado Boulder, Boulder, CO, USA

8 ⁴ Department of Geological Sciences, University of Colorado Boulder, Boulder, CO, USA

9 ⁵ Cooperative Institute for Research in Environmental Sciences (CIRES), University of Colorado Boulder, Boulder,
10 CO, USA

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12 *Correspondence to:* Jeffrey Keck (keckje@gmail.com)

13 Abstract

14 We developed a new rule-based, cellular-automaton algorithm for predicting the hazard extent, sediment transport and
15 topographic change associated with the runout of a landslide. This algorithm, which we call MassWastingRunout
16 (MWR), is coded in Python and implemented as a component for the package Landlab. Given the location and
17 geometry of an initial landslide body (i.e., landslide polygon), MWR models the downslope progression of the runout
18 process and evolves the underlying terrain. Runout behavior is controlled by mass continuity, topography, and rules
19 for erosion and deposition, which can be informed from field observations. MWR includes a calibration utility that
20 uses a Markov Chain Monte Carlo algorithm to sample model parameter space and tune the model to match observed
21 patterns of landslide runout extent, deposition and erosion. Output from the calibration utility informs probabilistic
22 implementation of MWR. Here we demonstrate calibrated model performance relative to a range of observed runout
23 phenomena and terrain, including debris flows in channelized, low-energy-dissipation terrains and debris avalanches
24 on open-slope, moderate, to high-energy-dissipation terrains. We test model ability to predict runout probability at a
25 case study site using parameters that were determined through calibration to a different site. Finally, we show how to
26 use a calibrated MWR model to determine runout-probability from an expert-defined, potentially unstable slope and
27 a landslide hazard map.

28 1. Introduction

29 Over geologic timescales, landslides (e.g., Hungr et al., 2013) and their runout shape the topographic expression of
30 mountain ranges and channel networks (Campforts et al., 2022; Korup, 2006; Larsen and Montgomery, 2012;
31 Montgomery and Deitrich, 1988). Over more pragmatic engineering and environmental risk management timescales,
32 landslides and their runout pose a primary risk to humans and infrastructure but also support numerous ecosystem
33 benefits, including carbon and nutrient transport from hillslopes to channels and the creation of ecological habitats
34 (Benda et al., 2003; Bigelow et al., 2007; Goode et al., 2012). Therefore, explicit representation of the landslide runout
35 should be an integral component of (1) landscape evolution models, with emphasis on topographic change prediction;



36 (2) landslide risk assessment tools, with emphasis on probabilistic impact and; (3) sediment budget models, with
37 emphasis on the mobilization and deposition of sediment carried by the landslide. Our proposed runout model is
38 developed as a component of the Landlab earth surface modeling toolkit (Hobley et al., 2017) and designed to serve
39 all three purposes.

40 Landslide runout processes can be generalized into three phases: initiation, erosion and deposition, briefly discussed
41 here. After a landslide initiates, it may break apart and become a granular flow such as a debris flow or dry debris
42 slide. Field observations show that erosion rates are sensitive to the moisture content of the channel bed (McCoy et
43 al. 2012) and, although highly variable, generally increase with flow depth (Schürch et al. 2011). At the same time,
44 laboratory experiments show that entrainment rates vary with granular stress (Capart et al. 2015) and increase with
45 flow grainsize (Egashira et al., 2001). Deposition occurs by layered accretion rather than emplacement of a single,
46 massive deposit (Major, 1997) and the length of the deposit is a function of resistance to the flow, the initial landslide
47 volume and the volume of material entrained during the runout (Iverson, 1997). The grain to water ratio and friction
48 angle of the slide material control the slope at which deposition begins (Takahashi, 2014; Major and Iverson, 1999;
49 Zhou et al., 2019) but the friction angle of the material varies as a function of the grains in the flow and fluidization
50 (fluctuation of interstitial fluid and the solid particles) of the flow material (Hutter et al. 1996). If the water content of
51 the runout material is high enough, as the solid fraction of the debris flow compresses, the water is squeezed out of
52 the debris flow and may continue to transport sediment as an immature debris flow (*sensu* Takahashi, 2014) or intense
53 bedload (*sensu* Capart & Fraccarolo, 2011), which also causes the deposition angle to decrease and extent of the runout
54 to increase.

55 Existing runout models attempt to represent the above processes with a variety of different numerical approaches and
56 level of detail. Most models are parameterized from measurements of frictional characteristics of the runout material
57 and model performance is validated against historic observations (e.g., measurements of an actual landslide and
58 runout). These models can broadly be grouped into four categories: (1) site-specific-empirical/statistical models
59 (Benda and Cundy, 1990; Fannin and Wise, 2001; Miller and Bennet, 2008; Reid et al. 2016); (2) detailed, continuum-
60 based mechanistic models that predict flow physics and granular dynamics (Takahashi, 1978; Iverson and Denlinger,
61 2001); (3) top-down, simplified rules-based abstractions of some of the key physical processes into reduced-
62 complexity flow-routing models (e.g., Murray, 2003; Clerici and Perego, 2000; Guthrie et al., 2008); and (4) hybrid
63 modeling approaches that combine detailed, mechanistic approaches with empirical models and top-down, rule-based
64 abstractions (Chen et al., 2023; D'Ambrosio et al., 2003; Iovine et al., 2005; Lancaster et al., 2003; McDougall and
65 Hungr 2004; Medina et al., 2008; Frank et al, 2015; Han et al., 2017, 2021).

66 While any of the above models can be implemented in a probabilistic manner for risk assessment purposes, simply by
67 repeatedly running the models for different initial landslide volumes or parameter values (e.g., Hurlimann et al., 2008),
68 these models generally either lack a built-in probabilistic implementation or are too computationally intensive for
69 Monte Carlo simulations. In addition, only a few models are designed for use in watershed sediment budget and
70 landscape evolution applications, which involves evolving both the topography and regolith during the runout event.
71 Finally, landslide hazard or risk maps typically use a hillslope stability model to delineate regional or watershed-scale



72 landslide risk at the initiation zone (e.g., Wu et al., 1995; Lee and Park, 2015; Lee et al., 2018; Strauch et al., 2018)
73 but exclude the subsequent runout hazard. To determine runout hazard, a landslide runout model is needed.
74 Of the landslide runout models used in landscape evolution applications, they generally include the removal and
75 redistribution of hillslope material via the initial landslide and subsequent runout, but ignore erosion and entrainment
76 of new material. Deposition of eroded landslide material is often calculated by redistributing it downstream either
77 based on a depositional slope threshold, or through a calculated transport distance formulation (Carretier et al., 2016;
78 Campforts et al., 2020). Fully mechanistic models can predict runout, their computational expense and heavily
79 parameterized entrainment rules can limit their application at the scale required for regional mapping and rapid
80 response assessments (Hurlimann et al., 2008; Reid et al., 2016; Gorr et al., 2022). Recently developed hybrid, top-
81 down and empirical-statistical methods provide opportunities for mapping downslope landslide hazard (Gorr et al.,
82 2022; Guthrie & Befus, 2021; Horton et al., 2013; Liu et al., 2022, Wallace et al., 2022), but the models are still
83 heavily parameterized and may require extensive regional observations to calibrate.

84 Our proposed model is a new rule-based cellular-automaton algorithm, suitable for predicting the hazard extent,
85 sediment transport and topographic change caused by the runout of a landslide and easily coupled with a landslide
86 hazard model. This algorithm is coded in Python and implemented as a set of components for the Landlab earth
87 surface modeling toolkit (Hobley et al., 2017; Barnhart et al., 2020) and is called MassWastingRunout (MWR). MWR
88 is designed to: (1) model topographic change caused by the erosion and deposition that occurs during the landslide
89 runout and; (2) map runout impact in space probabilistically as a function of uncertainties in the initial landslide
90 volume and model parameterization. Most MWR parameter values can be inferred from field or remote sensing
91 observations but two parameters (see Section 3) may rely on model calibration. For the purposes of rapidly calibrating
92 MWR to the observed runout extent and depositional patterns of a specific site, we developed a Markov Chain Monte
93 Carlo (MCMC) calibration utility. Additionally, MWR can track attributes of the regolith such as depth, organic
94 content and sediment size, represented as a numerical value on a model grid and transferred as a conserving and well-
95 mixed mass with the runout material.

96 In this paper, we present the MWR model (Section 2), describe the calibration utility and probabilistic implementation
97 of MWR (Section 3) and demonstrate calibrated model performance at four topographically and geologically unique
98 field sites (Section 4). The field sites include six different landslides and encompass the low-, moderate- and high-
99 energy-dissipation settings (Figure 1) described by Nicoletti and Sorriso-Valo (1991). The sites also span a range of
100 landslide-runout erosion rates. After demonstrating calibrated model performance, we then test the model using the
101 parameterization of one site to predict runout probability at a neighboring site. Finally, we show how to couple MWR
102 with a probabilistically modeled landslide hazard map or a field-mapped, potentially unstable slope to determine
103 runout probability (Section 5).



104

105 **Figure 1:** Runout at the four validation sites: (a) Cascade Mountains, WA: a large debris avalanche over steep, broadly convergent
106 terrain (photo credit: Stephen Slaughter). (b) Black Hills, WA: a large debris flow over a broadly convergent, gently sloped valley
107 (photo credit: Stephen Slaughter). (c) Rocky Mountains, CO: a moderate sized debris avalanche over steep, unconfined to divergent
108 hillslope. (d) Olympic Mountains, WA: small debris flows in steep, highly convergent channels.

109 **2. Description of the MassWastingRunout model**

110 **2.1 Overview of the cellular-automaton modeling approach**

111 MWR is coded as a discrete cellular automaton (CA) model. CA models iteratively apply a set of equations or rules
112 (deterministic or probabilistic) to individual cells of a grid (e.g., Codd, 1968) to change the numerical or categorical
113 value of a cell state or calculate fluxes of mass. In earth sciences, CA models have been used for modeling vegetation
114 dynamics (e.g., Nudurupati et al., 2023) and geomorphic transport, in which gravitationally directed erosion and
115 depositional processes modify a digital elevation model (DEM) representation of a landscape (e.g., Chase, 1992; Crave
116 & Davy, 2001; Murray & Paola, 1994; Tucker et al., 2018). Other cellular-automaton based landslide runout models



117 include DebrisFlow Predictor (Guthrie and Befus, 2021), SCIDDICA (D’Ambrosio et al., 2003) and a model by Han
118 et al. (2021). In the context of predicting the runout from a landslide, the CA approach often includes only the key
119 processes that control the runout at the DEM grid scale (See Section 4) and therefore may be faster than an equivalent
120 detailed, continuum-based mechanistic model that attempts to model small-scale processes such as the granular
121 physics of the runout. In MWR, computations only occur at the location of moving debris, in a manner analogous to
122 the “mobile” cellular automaton implementation of Chase (1992).

123 Chase (1992) modeled precipitation-driven fluvial erosion by randomly placing single packets of precipitation on a
124 DEM, which then moved downslope, eroding and transporting sediment as a function of slope. The individual packets
125 of precipitation were referred to as precipitons. After one iteration, both the precipiton and the landscape evolved and
126 the location of the precipiton for the next iteration was determined by the slope of the landscape and the state of the
127 precipiton. In our implementation, we route debris released from a specified landslide source area and we refer to the
128 packets of debris as “debritons”. The debritons represent debris flux. Each debriton occupies a single cell. Multiple
129 debritons can traverse the landscape at once. As they move, the debritons change volume, split to form new, smaller
130 debritons or combine to form larger debritons. In this model, debris flux is defined as the volume of debris per unit
131 area of the terrain transferred downslope in one model iteration [$L3/L2/iteration$]. Erosion and transport capacity of
132 the debritons are a function of slope as well as properties of the debritons.

133 The present implementation of the MWR algorithm uses the Landlab raster model grid. The raster grid consists of a
134 lattice of rectangular cells. Properties of each cell, such as topographic elevation, slope, and other spatially varying
135 attributes such as regolith depth or grain size, are recorded at nodes in the center of each cell (see Figure 5 of Hobbey
136 et al. 2017) and evolve as the model runs. In the next section, we describe the methodology used to determine flow
137 direction and how each debriton interacts with the terrain and regolith as it moves down slope.

138 **2.2 Mobilization of the initial landslide (Algorithm 1):**

139 Algorithm 1 determines the initial direction and flux of material over the landslide slip-surface (surface along which
140 the landslide body initially fails/moves). It operates on all nodes within the body, as represented by a user-specified
141 polygon and serves as the first iteration of MWR. All later iterations are implemented following Algorithm 2
142 (described below).

143 For each node n within the landslide polygon (Figure 2, iteration 0), Algorithm 1 determines which nodes will receive
144 debris (receiver nodes: R_n), the flux of debris sent to each of the node n ’s i -th receiver nodes (qs_{n,R_n}) and attributes
145 of the sent debris (ξ_n) based on topographic slope and attributes at node n (Figure 3). Movement is initiated at the
146 lowest debriton in the landslide body, where topographic slope is computed from the upper surface of the debriton. At
147 all other debritons, and for all other stages of modeled runout (Algorithm 2), topographic slope is determined from
148 the terrain underlying the debriton (See Figure 3a and 3b).

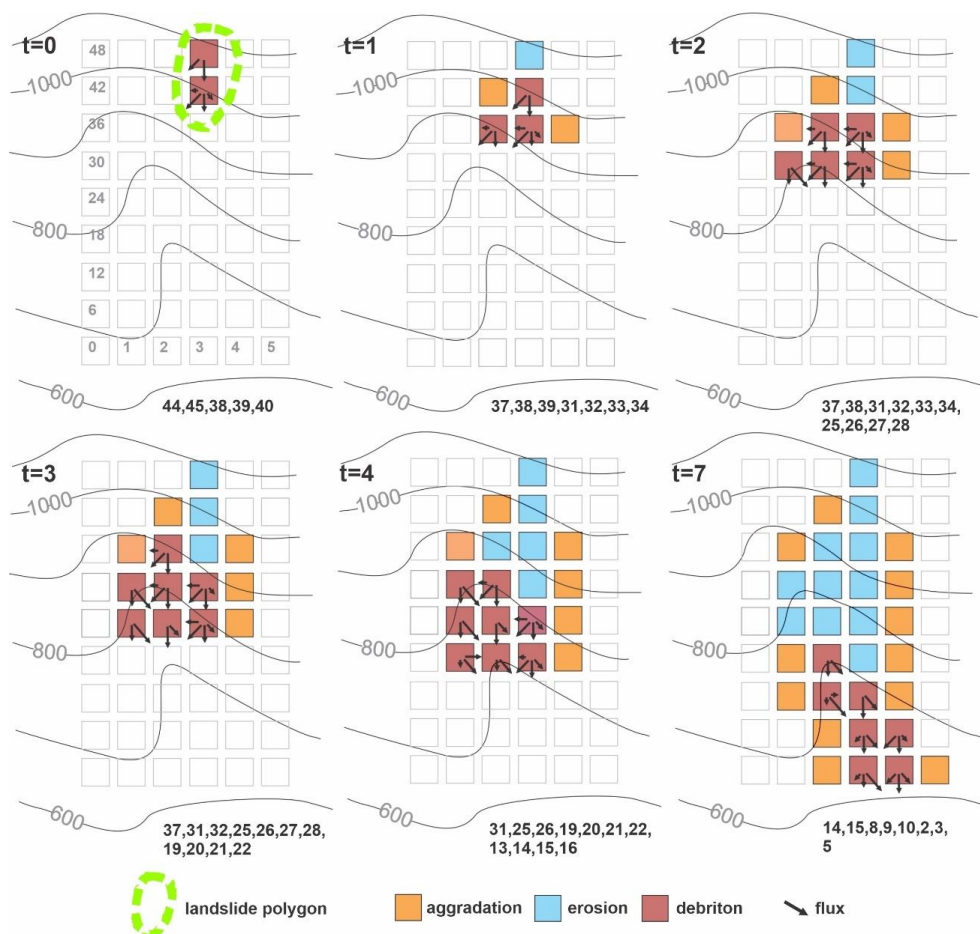
149 The depth of the slip surface is specified by the user, which can be inferred from field evidence at a specific landslide
150 (e.g., shallow failure along the fresh-bedrock to regolith contact) or determined from an external landslide model (See
151 Section 5). All nodes that send material are designated as donor nodes and appended to a list of donor nodes (D). All
152 R_n are appended to receiver node list R . Flux and attributes that will be passed from the donor nodes to the receiver



153 nodes are appended to lists qs and ξ . Lists D , R , qs and ξ are the input to Algorithm 2 and change with each iteration
 154 of Algorithm 2.
 155

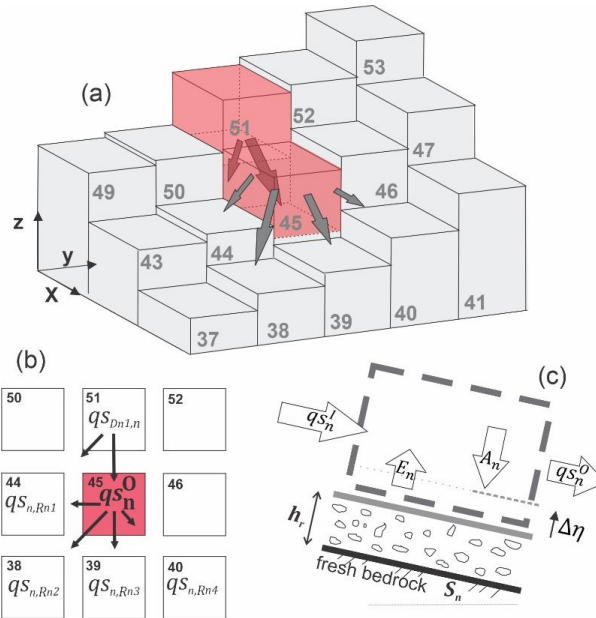
Algorithm 1 Determinin the initial D , R , qs and ξ lists
 sort initial landslide nodes from lowest elevation to highest
for each node n in the initial landslide nodes, beginning from the lowest node
 determine R_n , qs_{n,R_n} and ξ_n using (1)
 subtract landslide thickness from topographic elevation
 update the topographic slope using new topographic elevation
 append n , R_n , qs_{n,R_n} and ξ_n to lists D , R , qs and ξ
end for

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Figure 2. Illustration of initial mass wasting release ($t = 0$) and runoff. Notice how the receiver node list R , shown at the base of each iteration, changes with each iteration. The flow elongates and widens as the number of nodes in R increase and stops when the number of nodes in R is zero. If the incoming flux (sum of all incoming debris) to a node is less than qs_c , the material stops, causing aggradation.



163

164 **Figure 3.** (a) Three-dimensional illustration of iteration 0 in Figure 1, showing initial landslide nodes (initial debritons) and flux
 165 towards downslope nodes. Except for the first released debriton (lowest elevation node in the landslide), all debritons are directed
 166 downslope based on the underlying topographic slope (compare flow directions of node 51 to node 45); (b) Distribution of qs_n^O to
 167 downslope nodes and incoming material qs_n^I (here equal to $qs_{Dn1,n}^I$); (c) Aggradation (A_n) and erosion (E_n) are determined from
 168 field informed rules and qs_n^O is determined from mass continuity.

169

170 **2.3 Rules for debris flow erosion and deposition (Algorithm 2)**

171 Algorithm 2 is essentially the runout model. Algorithm 2 is repeatedly implemented while the length of R is greater
 172 than zero (while there are receiver nodes for the next model iteration). An illustration of several iterations of Algorithm
 173 2 is shown in Figure 2 and in the Validation section (Section 4, Figure 7). Algorithm 2 determines how material
 174 traverses and interacts with the terrain, beginning with the initial debritons, described by D, R, qs and ξ lists passed
 175 from Algorithm 1. While Algorithm 2 processes each debriton, it appends n (node index of the debriton), $R_n, qs_{n,Rn}$
 176 and ξ_n to temporary lists D', R', qs' and ξ' . At the end of each implementation of Algorithm 2, D', R', qs' and ξ'
 177 become the updated D, R, qs and ξ lists.

178 In addition to slope, debriton movement is controlled by mass continuity and a simple threshold flux constraint. At a
 179 node n , outgoing sediment flux, qs_n^O , is determined as (Figure 3c):

$$180 \quad qs_n^O = \begin{cases} qs_n^I - A_n + E_n, & qs_n^I \geq qs_c \\ 0, & qs_n^I < qs_c \end{cases}, \quad (1)$$

181 Where, qs_n^I is total incoming sediment flux [L/iteration] to the node (described in Equation 2); A_n and E_n are local
 182 deposition (aggradation) and erosion [L/iteration], both of which are calculated prior to (1); qs_c is the threshold flux
 183 for deposition. When $qs_n^I < qs_c$, qs_n^I deposits and qs_n^O becomes zero. Conceptually, qs_c represents the flow depth



184 below which flow resistance is large enough to cease the forward momentum of the flow, whether in the form of
 185 frictional resistance along the base of the flow or debris and vegetation in the path of the flow.

186 Summing the outgoing flux from each donor node towards node n , gives the total incoming flux, qs_n^I :

$$187 \quad qs_n^I = \sum_{j=1}^{Nd_n} qs_{D_{nj},n} \quad (2)$$

188 where Nd_n is the number of donors to node n , and $qs_{D_{nj},n}$ is the flux from node D_{nj} (the j -th donor to n) to node n .

189 The flux from node n to each of the node n 's i -th receiver nodes ($qs_{n,R_{ni}}$) is determined using the Freeman (1991)
 190 multiframe direction algorithm:

$$191 \quad qs_{n,R_{ni}} = qs_n^O \frac{S_i^a}{\sum_{i=1}^{Nr_n} S_i^a} \quad (3)$$

192 where Nr_n is the number of receiving nodes of node n , and S_i is the underlying topographic slope ($\tan \theta$) to each of
 193 node n 's i -th receiver and qs_n^O .

194 The multiframe direction algorithm is common to many models, and in this implementation it is handled by a pre-
 195 existing Landlab flow-routing component. The exponent a controls how material is distributed to downslope nodes
 196 as a function of slope. In a braided river cellular-automaton model, Murray and Paola (1994) used an approximation
 197 for turbulent shallow water flow to justify $a = 0.5$ (which is the exponent on the slope factor in channel friction laws).
 198 For our application, we found MWR was easier to calibrate to observed mass wasting runoff if $a = 1$.

199 Local aggradation (A_n) and erosion (E_n) in (1) are determined using a series of field-data-informed rules (Figure 3c).
 200 These rules are equivalent to the “transition functions” applied in SCIDDICA (D’Ambrosio et al. 2003) and the Han
 201 et al. (2021) models. If the local slope (S_n) is greater than a critical slope (S_c), then no material deposits, whereas if
 202 S_n is less than S_c , then a portion or all of qs_n^I deposits:

$$203 \quad A_n = \begin{cases} 0 & , \quad S_n \geq S_c \Delta x \\ \min(A_{p,n|N_a}, qs_n^I) & , \quad S_n < S_c \Delta x \end{cases} \quad (4)$$

204 Where, $A_{p,n|N_a}$ is the potential deposition depth at node n , which is the first node where the debris begins to deposit
 205 because the deposition criterion is met (described below, see Figure 4). To determine $A_{p,n|N_a}$, we initially used the
 206 nonlinear, nonlocal deposition scheme described by Carretier et al. (2016) and Campforts et al. (2020); however, when
 207 implemented with the flow routing method described above, unreasonably tall deposits resulted when qs_n^I was large
 208 and $S_n \ll S_c$. Alternatively, we experimented with limiting the deposition depth to $A_n \leq S_c \Delta x$, but when qs_n^I was
 209 large, this constraint resulted in long deposits that paralleled the underlying slope. Therefore, a different constraint
 210 was needed. We ultimately developed a new deposition rule that determines deposition at a node by back-computing
 211 the deposition amount necessary to form a deposit that spreads over multiple nodes whose surface slope is controlled
 212 by S_c , as described below.

213 When the slope criteria for deposition is met at a node n , sediment that enters the node is assumed to be
 214 deposited/distributed over a single row of downstream nodes following three rules: (1) the volume of the deposit will
 215 be equal to $qs_n^I \Delta x^2$ and spreads over N_a nodes; (2) the surface slope of the deposit will equal the critical slope S_c and;
 216 (3) the underlying topographic slope of the N_a nodes is uniform and equal to S_n . Assumption 3 is necessary because



217 the terrain evolves with each model iteration and the future downslope terrain is unknown. From this hypothetical
218 deposit, we can analytically define $A_{p,n|N_a}$ and N_a as a function of qs_n^I , S_c and S_n as follows (see Figure 4):

219 From assumption 1, qs_n^I can be expressed as the sum the N_a deposits that make up the overall deposit as:

$$220 \quad qs_n^I = \sum_{i=1}^{N_a} A_{p,i} \quad (5)$$

221 where $A_{p,i}$ is the i -th deposition amount in the deposit. Since we assume the deposit slope and underlying topographic
222 slope are uniform, the deposition amount at any of the N_a nodes can be determined from the last deposition amount
223 ($A_{p,1}$) as:

$$224 \quad A_{p,i} = A_{p,1} + (i - 1)\Delta x(S_c - S_n) \quad (6)$$

225 From (6) we can re-write (5) as a function of $A_{p,1}$ (write each of N_a deposits in (6) in terms of $A_{p,1}$) and rearrange to
226 define $A_{p,1}$ as a function of qs_n^I :

$$227 \quad A_{p,1} = \frac{1}{N_a} qs_n^I - \frac{N_a-1}{2} \Delta x(S_c - S_n) \quad (7)$$

228 Substituting (7) into (6) and solving for $i = N_a$ (i.e., A_{p,N_a}), we get an expression for $A_{p,n|N_a}$:

$$229 \quad A_{p,n|N_a} = \frac{1}{N_a} qs_n^I + \frac{N_a-1}{2} \Delta x(S_c - S_n) \quad (8)$$

230 Equation (7) can be rearranged into a quadratic equation and solved for N_a as:

$$231 \quad N_a = \frac{-A_{p,1} + \frac{1}{2} \Delta x(S_c - S_n) \pm \sqrt{\left(A_{p,1} - \frac{1}{2} \Delta x(S_c - S_n)\right)^2 + 2 \Delta x(S_c - S_n) qs_n^I}}{\Delta x(S_c - S_n)} \quad (9)$$

232 We use (8) to solve for $A_{p,n|N_a}$ and (9) to solve for N_a assuming $A_{p,1} = 1/2 \Delta x S_c$ and rounding the positive solution
233 to the nearest integer. When implemented using a single debriton released on a two-dimensional hillslope as illustrated
234 in Figure 4 (i.e., a single row of nodes), the debriton deposits over N_a nodes at a uniform slope equal to S_c . When
235 implemented on an actual terrain, the interaction between multiple debritons in multiple directions creates a complex
236 deposit whose slope changes with S_c .

237 The third variable in Equation 1 is debriton erosion rate, E_n [L/iteration]. We constrain E_n to the lesser of a potential
238 erosion rate, h_e , and local regolith depth, h_r :

$$239 \quad E_n = \min(h_r, h_e) \quad (10)$$

240 where h_e is computed as a function of the basal shear stress of the granular flow (τ , described below in Equations 12
241 and 13) and the critical shear stress (τ_c) of the regolith at the node:

$$242 \quad h_e = k(\tau - \tau_c)^m \quad (11)$$

243 Shen et al. (2020) used $\tau_c = 5$ Pa to represent a boulder-colluvium substrate. Stock and Deitrich (2006) used normal
244 rather than shear stresses and excluded τ_c ($\tau_c = 0$). The exponent m controls the non-linearity of h_e . Many authors
245 (Chen & Zhang 2015; Frank et al., 2015; Shen et al., 2020) use a value of 1 for m but field measurements by Schurch



246 et al. (2011) (see their Figure 3) suggest that m may be less than 1 if τ is assumed to vary linearly with flow depth,
247 particularly at high flow depths ($h > 3\text{m}$). We experimented with various values of m , ranging from 1 to 0.2 and
248 found that if τ is assumed to vary linearly with flow depth (see Equation 12, below), model performance improves
249 with lower values of m and if τ is assumed to vary as a function of grain-collision dependent shear stress (see Equation
250 13, below), as long as k is determined as a function of m (Section 4.3, Equation 36), the impact of the value of m on
251 model behavior is relatively minor.

252 The coefficient k in (11) is an erodibility parameter. Stock and Dietrich (2006) showed that k encapsulates rock
253 properties. If h_e is used to represent erosion over geomorphic time scales, with repeated debris flow occurrences in a
254 single model iteration, k becomes associated with debris flow length and frequency (Perron, 2017). In our application
255 we are modeling the erosion associated with a single runout event, as represented by the debritions. The coefficient k
256 therefore needs to scale h_e so that it is on the order of the average erosion depth caused by a single debrition.

257 As noted above, to define τ , MWR includes two options: (1) a quasi-static basal shear stress approximation or (2) a
258 grain-size-based shear stress approximation. The quasi-static basal shear stress approximation (e.g., Takahashi, 2014)
259 is defined as:

$$260 \quad \tau = \rho g h \sin \theta \quad (12)$$

261 where ρ is the density of mass wasting material (grain and water mixture), g is gravity and h is flow depth, which in
262 MWR is equivalent to qs_n^l . The grain-size-based shear stress approximation is defined as follows:

263 Bagnold (1954) defined τ as a function of grain-collision dependent normal stress (σ).

$$264 \quad \tau = \sigma \tan \varphi \quad (13)$$

265 where φ is the collision angle between grains, measured from the vertical axis (See Bagnold, 1954), with a value of
266 $\tan \varphi$ typically equal to 0.32. Following Bagnold (1954) and Iverson (1997), Stock and Dietrich (2006) defined σ as:

$$267 \quad \sigma = \cos \theta v_s \rho_s D_s^2 \left(\frac{du}{dz} \right)^2 \quad (14)$$

268 where θ is the topographic slope measured in degrees, v_s is the volumetric solids concentration, ρ_s is density of the
269 solids, u is flow velocity, z is depth below the flow surface, du/dz is strain and D_s is the representative grain size.
270 Stock and Dietrich (2006) suggested that D_s corresponds to a small percentile of the coarsest fraction of the runout
271 material (D_{88} to D_{96}) and they approximated du/dz as:

$$272 \quad \frac{du}{dz} = \frac{u}{h} \quad (15)$$

273 Solely for the purpose of computing du/dz , we approximate velocity at a node following Julien and Paris (2010) as:

$$274 \quad u = 5.75 u^* \log \left(\frac{h}{D_s} \right) \quad (16)$$

275 where u^* is shear velocity ($\sqrt{gh \tan \theta}$). This grain-size dependent approximation for τ is advantageous because it
276 permits landslide-driven erosion rates to vary by lithologic region, as represented by spatial variation in D_s generated



277 by each lithologic region (e.g., Roda-Boluda et al. 2018), Furthermore, D_s can be quickly approximated in the field
278 by measuring the coarser grain sizes in existing runout-deposits, road-cuts and tree-throw pits.
279 Once A_n and E_n are determined at a node, we assume uniform material density and determine the change in elevation
280 at the node ($\Delta\eta_n$) as:

$$281 \quad \Delta\eta_n = A_n - E_n \quad (17)$$

282 Finally, for each regolith attribute being tracked by the model (e.g., grain size), the attribute value at the node (ξ_n)
283 and the attribute value sent from the node to its receiver nodes (ξ_{Rn}) is determined using a volumetric-weighted
284 average as:

$$285 \quad \xi_n = \frac{\xi_n(h_r - E_n) + \xi_{Dn}(A_n)}{D + h_r - E_n} \quad (18)$$

$$286 \quad \xi_{Rn} = \frac{\xi_n E_n + \xi_{Dn}(qs_{in} - A_n)}{qs_{on}} \quad (19)$$

287 Where ξ_{Dn} is the weighted average attribute value delivered to the node from its donor nodes:

$$288 \quad \xi_{Dn} = \frac{\xi_D qs_{Dn}}{qs_{in}} \quad (20)$$

289 and qs_{Dn} is a vector containing all $qs_{D_{nj},n}$ sent to the node and ξ_i is a vector containing the incoming attribute values
290 for each $qs_{D_{nj},n}$.

291 For each model iteration t , as Algorithm 2 processes each unique node n in the receiver node list (R), it appends $\Delta\eta_n$
292 to a list $\Delta\eta$ and n to D' , R_n to R' , $qs_{n,Rn}$ to qs' and ξ_{Rn} to ξ' . After Algorithm 2 has cycled through all receiver nodes,
293 D' , R' , qs' and ξ' become D , R , qs and ξ and maps of regolith thickness ($h_{r,t}$), topographic elevation (η_t) and the
294 elevation of the runout flow surface (F_t) are updated for the entire model domain using vector operations as:

$$295 \quad \eta_t = \eta_{t-1} + \Delta\eta \quad (21)$$

$$296 \quad h_{r,t} = h_{r,t-1} + \Delta\eta \quad (22)$$

$$297 \quad F_t = \eta_t + qs \quad (23)$$

298 After η_t , $h_{r,t}$ and F_t are updated, the multi-direction slope of the DEM, which is used for routing the debritons the
299 next model iteration, is recomputed from η_t .

300 Note, the debritons are obstructed if they encounter a topographic pit or flat topography in the DEM. To allow a
301 debriton to pass an obstruction, we rely on a simple work-around: upon encountering the obstruction, the debriton is
302 directed to itself ($R_n = n$) and some portion of the debris is deposited based on (4). At the end of the model iteration,
303 the node elevation and slope are updated, and then during the next iteration, if the remaining mobile debris is no longer
304 obstructed, it moves to its downslope node(s). If the node is still obstructed, it is again sent to itself until either all
305 material has deposited or the elevation of the node exceeds that of its neighbor nodes, allowing the debriton to move
306 downslope.



307 Finally, as noted in Tucker and Hancock (2010), flow routing algorithms used in CA models, such as (3), can cause
 308 flow-depth dependence on the grid-size used to represent the terrain. Since aggradation and erosion rates determined
 309 by (8) and (11) are sensitive to flow depth, we thus constrain the flow depth used to determine A_n and E_n to no more
 310 than the maximum observed flow depth as:

$$311 \quad h' = \min(h_{max}, h) \tag{24}$$

312 Where h' is the flow depth used in (8) and (11) and h_{max} is the maximum observed flow depth, which can be inferred
 313 from field indicators (See Section 4). This constraint is not applied to mass continuity in (1).

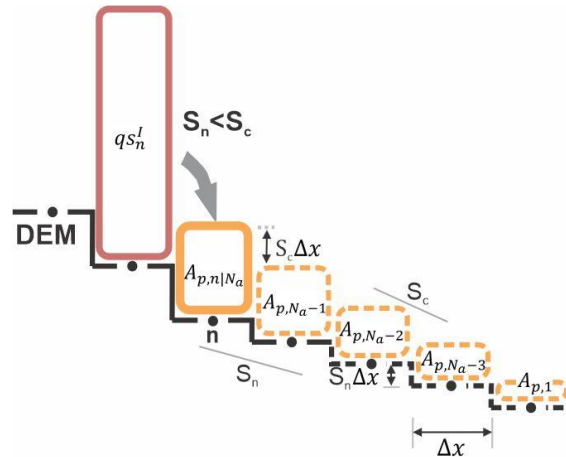
314

Algorithm 2 Runout algorithm

```

while length(R) > 0
  for node n in R
    determine flux and attributes of incoming material from lists qs and ξ
    compute erosion, deposition and outflow as a function of underlying topographic slope
    determine the new receiver nodes (Rn), receiving flux (qs_n,Rn), receiving properties (ξ_Rn)
    append n to D', Rn to R', qs_n,Rn to qs and ξ_Rn to ξ'.
  end for
  update the topographic elevation and slope for erosion and deposition
  lists D', R', qs' and ξ' become D, R, qs and ξ
end while
    
```

315



316

317 **Figure 4.** Conceptual model used to define deposition rule, here illustrated using $N_a = 5$. The rule relies on three
 318 assumptions about the depositional process: (1) deposition of the debris, which has a flux of qs_n^I at node n ,
 319 spreads over N_a nodes to form a hypothetical deposit as illustrated by yellow boxes above; (2) the underlying
 320 topographic slope of the N_a nodes is uniform and equal to S_n and; (3) the surface slope of the hypothetical deposit
 321 will be equal to S_c . Solid yellow box indicates deposition at node n . Dashed yellow boxes and lines indicate
 322 hypothetical deposition and underlying topography. Dots along DEM surface are nodes.



323 **3. Calibration and MWR probability**

324 **3.1 Calibration utility**

325 We developed a calibration program, written in Python as a utility for the package Landlab, to calibrate MWR. This
 326 utility uses a modified version of an adaptive Markov Chain Monte Carlo (MCMC) sampling algorithm from Coz et
 327 al. (2014), who used it to calibrate stage-discharge relationships to observed river flow in France. The MCMC
 328 algorithm automatically samples model parameter space, evaluates model performance relative to observed runoff,
 329 and determines the most likely set of parameter values that calibrate the model.

330 For each iteration of the calibration utility, a likelihood index, $L(\theta)$, for the given set of model parameters (θ) is
 331 determined as the product of the probability of the selected parameter values, based on a prior estimate of the
 332 probability density function (PDF) of each parameter ($p(\theta)$), and three other metrics as:

$$333 \quad L(\theta) = p(\theta) * \Omega_T * \frac{1}{\Delta\eta_E^2} * \frac{1}{Q_{sE}^2} \quad (25)$$

334 where Ω_T is the Lee-Salle index (Heiser et al., 2017) and indicates modeled planimetric fit to the observed debris flow
 335 extent, and $\Delta\eta_E^2$ and Q_{sE}^2 are new dimensionless indices, proposed for this study, that represent modelled distributed
 336 topographic change and cumulative flow errors, respectively. Larger values of Ω_T indicate that the modeled
 337 planimetric runoff extent more closely fits observed. Smaller values of $\Delta\eta_E^2$ and Q_{sE}^2 indicate better fit. Note that
 338 the reciprocal values of $\Delta\eta_E^2$ and Q_{sE}^2 are used in (25) so that improved fit causes $L(\theta)$ to increase. To ensure that
 339 all values of $L(\theta)$ are greater than or equal to zero, here we add a value of 1 to Ω_T so that it scales from 0 to 2 as:

$$340 \quad \Omega_T = \frac{\alpha - \beta - \gamma}{\alpha + \beta + \gamma} + 1 \quad (26)$$

341 where α , β and γ are the areas of matching, overestimated and underestimated runoff extent, respectively. The root
 342 values of $\Delta\eta_E^2$ and Q_{sE}^2 ($\Delta\eta_E$ and Q_{sE}) reflect the volumetric error of the modeled topographic change relative to the
 343 observed total mobilized volume (observed landslide + entrained material) and the mean-modeled-cumulative flow
 344 error along the runoff path relative to the observed mean cumulative flow. The index $\Delta\eta_E^2$ is determined by dividing
 345 the squared, cumulative, volumetric square error (SE) of modeled topographic change by the squared-observed-total-
 346 mobilized volume (V):

$$347 \quad \Delta\eta_E^2 = \frac{SE_\alpha + SE_\beta + SE_\gamma}{V^2} \quad (27)$$

348 Here values SE_α , SE_β and SE_γ are the SE of modeled $\Delta\eta$ in areas α , β and γ respectively, individually calculated as:

$$349 \quad SE = \sum_{i=0}^p [(\Delta\eta_{Oi} - \Delta\eta_{Mi}) \Delta x^2]^2 \quad (28)$$

350 where, p is the number of nodes within each of the α , β , and γ areas, and $(\Delta\eta_{Mi})$ and $\Delta\eta_{Oi}$ are the observed
 351 topographic change [L] at the i -th node (cell) in that erosion area.

352 Cumulative flow volume (Q_s) is determined along the channel profile, as the integrated, volumetric topographic
 353 change upslope of a point along the profile in a manner similar to Fannin and Wise (2001), who computed cumulative



354 debris flow volume at individual reaches along a channel profile (see their Figure 4). We compute cumulative flow at
 355 each node (grid cell) j along the channel profile as:

$$356 \quad Q_s = -\Delta x^2 \sum_{i=1}^{q_j} \Delta \eta_{i,j} \quad (29)$$

357 where $\Delta \eta_{ij}$ is the topographic change [L] at the i -th node located upstream of node j , q_j is the number of all nodes in
 358 the DEM located upstream of node j , and node j is a node located along the channel profile. The negative sign converts
 359 upslope erosion and deposition to a positive and negative change in cumulative flow respectively. For each node j
 360 along the profile, Q_s is computed for both the observed and modeled runout (Q_{sOj} and Q_{sMj}) and Q_{sE}^2 is the mean
 361 square error of the modeled-cumulative flow at all nodes along the profile, normalized by the observed-mean-squared-
 362 cumulative flow along the profile:

$$363 \quad Q_{sE}^2 = \frac{\frac{1}{r} \sum_{j=1}^r (Q_{sOj} - Q_{sMj})^2}{\frac{1}{r} \sum_{i=0}^r Q_{sOj}^2} \quad (30)$$

364 Where r is the number of nodes in the channel profile. As noted above, values of Q_{sE}^2 and $\Delta \eta_E^2$ (or Q_{sE} and $\Delta \eta_E$)
 365 close to zero indicate a good fit and large values ($\gg 0$) indicate a poor fit and in the final likelihood index, $L(\theta)$, we
 366 use the reciprocals of $\Delta \eta_E^2$ and Q_{sE}^2 , so that smaller values of Q_{sE}^2 and $\Delta \eta_E^2$ result in larger $L(\theta)$ values.

367 A minimalistic calibration of MWR requires a shape file, representing the boundaries of a landslide and its observed
 368 runout. In this application, (25) is determined as $L(\theta) = p(\theta) * \Omega_T$. A thorough calibration of MWR requires a DEM
 369 of difference (DoD), which is a map of the change in topographic elevation ($\Delta \eta$) caused by the observed runout, and
 370 is most easily obtained by subtracting the DEMs recorded before and after the landslide and subsequent runout but
 371 can also be created from a field survey (see Section 4). In case that a minimalistic calibration is used, because the
 372 likelihood index is limited to an evaluation of planimetric extent, the model tends to have high parameter equifinality
 373 (e.g., Beven 2006); multiple parameter sets result in an equally calibrated model as evaluated by Ω_T . When an
 374 observed DoD is used to calibrate MWR, a much narrower range of equally likely parameter values can be identified.
 375 The calibration utility is presently coded to modify S_c and qs_c , k , landslide thickness, and mean soil depth; however,
 376 any parameter of the MWR can potentially be calibrated using this approach.

377 As will be detailed in Section 4, most parameters in MWR can be inferred from field observations and a DoD map,
 378 but parameters S_c and qs_c are best determined through calibration. A range of S_c values can be inferred from
 379 deposition slopes measured at channel-filling deposits (reaches where deposition exceeds erosion and results in
 380 positive net deposition; Guthrie et al., 2010) of the observed runout but given the intermittency and variability of most
 381 debris flow deposits, identifying a single representative S_c value in the field can be difficult. As noted in Section 2,
 382 conceptually, qs_c represents a minimum flow thickness below which resistance due to friction or debris, such as
 383 vegetation or boulders in the path for the flow, causes the flow to cease. While it's possible that qs_c may be inferred
 384 from the minimum observed flow thicknesses along the margins and termini of the runout, this evidence, especially
 385 at the termini of debris flows, can be erased rapidly as streams erode deposits and may also vary along the runout path.
 386 It is through model calibration to the DoD of the observed runout, that a single value of S_c and qs_c that best represents
 387 the heterogeneity of observed depositional slope and flow thickness can be determined.



388 The MCMC algorithm is implemented as follows (see Algorithm 3): First, the user defines an initial prior PDF for
389 each parameter to be calibrated, currently only limited to normal and uniform distributions. The algorithm then
390 randomly selects a set of parameter values (θ) from the prior PDFs, evaluates the prior likelihood of the parameter set
391 ($p(\theta)$), and runs MWR using θ . Once the model has completed the run, the algorithm evaluates the posterior
392 likelihood of the parameter set ($L(\theta)$), based on how well modeled runout matches the planimetric extent, topographic
393 change and cumulative flow along the observed runout path.

394 After the first $L(\theta)$ has been determined, the algorithm selects a new set of parameters (θ_{t+1}) by jumping some
395 distance from each parameter in θ space. The jump distance is determined by randomly sampling a normal distribution
396 of possible jump distances, whose standard deviation is initially specified by the user, but changes as the algorithm
397 iteratively runs and evaluates model performance. The next iteration, $t + 1$, MWR is run using θ_{t+1} and once the
398 model has completed the run, L_{t+1} is determined. If L_{t+1} is larger than L , θ is updated to the value of θ_{t+1} and L is
399 updated to the value of L_{t+1} . If $L_{t+1} < L$, then θ and L have a probability of remaining unchanged equal to $1 - L_{t+1}/L$
400 where the fraction L_{t+1}/L is referred to as the acceptance ratio. This Markov process is repeated a user-specified N
401 times. The algorithm is adaptive because the jump distance changes depending on the average acceptance ratio, which
402 is evaluated every user-specified number of iterations (*Ncycles*). Two additional parameters control the jump size:
403 a_{min} and a_{max} . If the average acceptance ratio is below a_{min} , the variance of the jump-distance distribution is
404 decreased by 10 percent and if the average acceptance ratio is greater than a_{max} , the variance is increased by 10
405 percent. We initially tested the adaptive MCMC algorithm using values for *Ncycles*, a_{max} , a_{min} and the initial jump
406 size (*jump_size*) from Coz et al. (2014) but through a series of tests, found that values listed in Algorithm 3 worked
407 best for our application.

408 An advantage of defining $L(\theta)$ as the product rather than summation of the runout extent, topographic change and
409 cumulative flow errors metrics is that a change in any one metric has an equal impact on the relative value of $L(\theta)$
410 (e.g., a 20% reduction of any of the metrics causes a 20% reduction in $L(\theta)$). Furthermore, because we use the product
411 of the reciprocals of the squared topographic and flow error (e.g., error-based weighting, Foglia et al., 2009), this
412 formulation of $L(\theta)$ is very sensitive to changes in calibration performance and results in values that vary over several
413 orders of magnitude. This sensitivity to calibration performance works well for guiding the MCMC algorithm as it
414 searches parameter space based on changes in the relative value of $L(\theta)$.

415 Additionally, one of the major advantages of using an MCMC approach for model calibration is that after an adequate
416 number of model runs have been performed, the resulting histogram of parameter indicates the posterior PDF of the
417 parameter values (Gelman et al., 2021; Renard et al., 2006), which can then be used in Monte Carlo modeling runs.
418 Typically the initial series of jumps are considered “warm up” and depending on context, may be discarded from the
419 histogram (Gelman et al., 2021). Also, the minimum number of iterations needed for convergence on best-fit
420 parameters (maximum $L(\theta)$) can be determined as a function of the ratio of between sequence variance (e.g., variance
421 between two sequences of *Ncycles* parameters) and variance within each sequence.

422 In the Model Validation section, we run the calibration utility using a single chain of 2000 repetitions to estimate the
423 posterior PDF for each parameter. At most sites, the model converges relatively quickly on a solution and we therefore
424 don't consider burn-in, or evaluate convergence. Future implementations of the calibration utility may include multiple



425 chains, burn-in and a check for convergence. We use the parameter set with the highest $L(\theta)$ value (best-fit parameter
 426 set) to compare model performance relative to observed runoff. Then, in Section 5, we iteratively run MWR with
 427 parameter values sampled from the posterior PDFs to illustrate the calculation of runoff probability as a function of
 428 parameter uncertainty.
 429

Algorithm 3 Adaptive Markov Chain Monte Carlo calibration algorithm

N : user defined number of times the while loop will run

θ_{t-1} : initial set of model parameter from which the first jump in parameter space is made

$jump_size$: ratio of standard deviation of the jump size expressed as the ratio of the range between the
 minimum and maximum parameter values

$L(\theta_{t-1}) = 1$

$t = 0$

while $t < N$

 from θ_{t-1} , jump random distance to new parameter set, (θ_t)

 compute likelihood of parameter values using prior distribution ($p(\theta_t)$)

 run MWR with θ_t

 evaluate model calibration with $\Delta\eta_E^2$, Q_{SE}^2 and Ω_T

 compute posterior likelihood of parameter values ($L(\theta_t)$) using (25)

$acceptance_ratio = \min(1, L(\theta_t)/L(\theta_{t-1}))$

 append $acceptance_ratio$ to $acceptance_ratio_list$

$rv =$ random value between 0 and 1

if $rv < acceptance_ratio$

$\theta_{t-1} = \theta_t$

if remainder of $t/Ncycles = 0$

if mean $acceptance_ratio > a_{max}$

 increase variance of jump size distribution by 10%

if mean $acceptance_ratio < a_{min}$

 decrease variance of jump size distribution by 10%

$t = t+1$

end while

Where $Ncycles = 100$, $a_{max} = 0.7$, $a_{min} = 0.3$, $jump_size = 0.09$

430

431 **3.2 Mapping landslide runoff risk**

432 MWR includes an additional program called MWR Probability, also written in Python as a utility for the package
 433 Landlab, that relies on a Monte Carlo implementation of MWR using the parameter PDF's produced by the calibration
 434 utility to produce landslide runoff hazard maps. MWR Probability repeatedly runs MWR for a user specified N times,
 435 each repetition with a different S_c , qs_c pair, randomly sampled from the parameter PDFs. After the N model runs, it
 436 determines probability of runoff impact at each model node as:

437
$$P(\Delta\eta) = \frac{n(|\Delta\eta| > 0)}{N} \quad (31)$$

438 where $n(|\Delta\eta| > 0)$ is the number of times topographic elevation at the node changed as a result of erosion or
 439 deposition from the N model runs.



440 MWR Probability includes three options for specifying the initial landslide that is be used to iteratively run MWR N
441 times: (1) a single, fixed-size landslide, represented as a polygon with a fixed regolith depth, (2) a single, landslide
442 whose size varies each iteration within a user defined potentially unstable slope with fixed regolith depth (3) multiple
443 landslides that change location, area and depth with each iteration. If option 2 is used, for each model iteration, MWR
444 Probability randomly selects MWR parameter values from the parameter PDFs and a landslide area within the
445 potentially unstable slope polygon. The randomly selected landslide area is at least as large as a user defined minimum
446 size but no larger than the potentially unstable slope and can form anywhere within the potentially unstable slope. If
447 option (3) is used, for each model repetition, MWR reads externally determined regolith depth and landslide locations
448 from a total of N regolith depth and landslide maps.

449 **4. Model Validation:**

450 **4.1 Overview**

451 In this section, we demonstrate calibrated model performance relative to observed runout at field sites located in the
452 western USA. Note that simply calibrating a model to match field data does not constitute a satisfactory test of model
453 predictive ability (Iverson, 2003). Strategic testing, which involves calibrating the model to one site or period of time
454 and then running the calibrated model at a separate site or period of time (Murray 2013), is a better indicator. Two of
455 our validation sites, the Cascade Mountain and Olympic Mountain sites, include two separate landslides and
456 subsequent runout and we test model predictive ability at these sites in Section 5.

457 Calibrated model performance is demonstrated at the following field sites (see Figure 5a for locations): (1) two runout
458 events in the Cascade Mountains (Washington state [WA], USA): a large debris avalanche in 2009 and a moderately-
459 sized debris flow in 2022 that inundated and flowed within a first-order channel until perpendicularly intersecting a
460 narrow river valley several hundred meters below the landslide (Figure 1a); (2) channelized debris flows sourced from
461 a small failure along the toe of a deep-seated landslide and a moderately-sized debris avalanche from a large road fill
462 in the Black Hills (WA) that flowed several kilometers along a flume-like channel before stopping (Figure 1b); (3) a
463 single, moderately-sized debris avalanche in the Rocky Mountains (Colorado state), the majority of which flowed
464 several hundred meters over a broadly convergent to divergent hillslope (Figure 1c); and (4) a 30-year chronology of
465 small landslides and subsequent debris flows in steep, highly convergent channels in the Olympic Mountains (WA)
466 that flowed well over a kilometer and coalesced into a single runout deposit in a dendritic, channelized watershed
467 (Figure 1d).

468 These validation sites encapsulate the three runout morphologies categorized in terms of runout-energy-dissipation by
469 Nicoletti and Sorriso-Valvo (1991): channelized runout (low-energy-dissipation; Figure 1d), unobstructed spreading
470 of the runout (moderate-energy-dissipation; Figure 1c) and runout that perpendicularly intersects a channel and
471 collides with the opposite channel wall (high-energy-dissipation; Figure 1a). The validation sites also span different
472 climates and land use. The Cascade Mountain, Black Hills and Olympic Mountain sites are located in a wet, maritime
473 climate, in landscapes actively managed as tree plantations. The Rocky Mountain site is located in a semi-arid climate,



474 in a landscape managed as a national park. Despite differences in runout morphology, land use and climate, all
 475 landslides initiated during heavy rainfall or rain-plus-snowmelt storm events (WRCC, 2022; NRCS, 2022; Table 1).
 476 In the next sub-sections, we summarize the remote sensing and field methods used to parameterize MWR and the
 477 calibration utility. Then, for each site, we describe observed runout characteristics, modeled parameterization and
 478 model performance. At all locations, we use (13) to approximate shear stress as a function of grain size. Model
 479 performance is discussed in terms of the planimetric, topographic and volumetric flow metrics in $L(\theta)$ and compared
 480 to topographic characteristics of the runout path.

481

482 **Table 1.** Landslide and runout characteristics

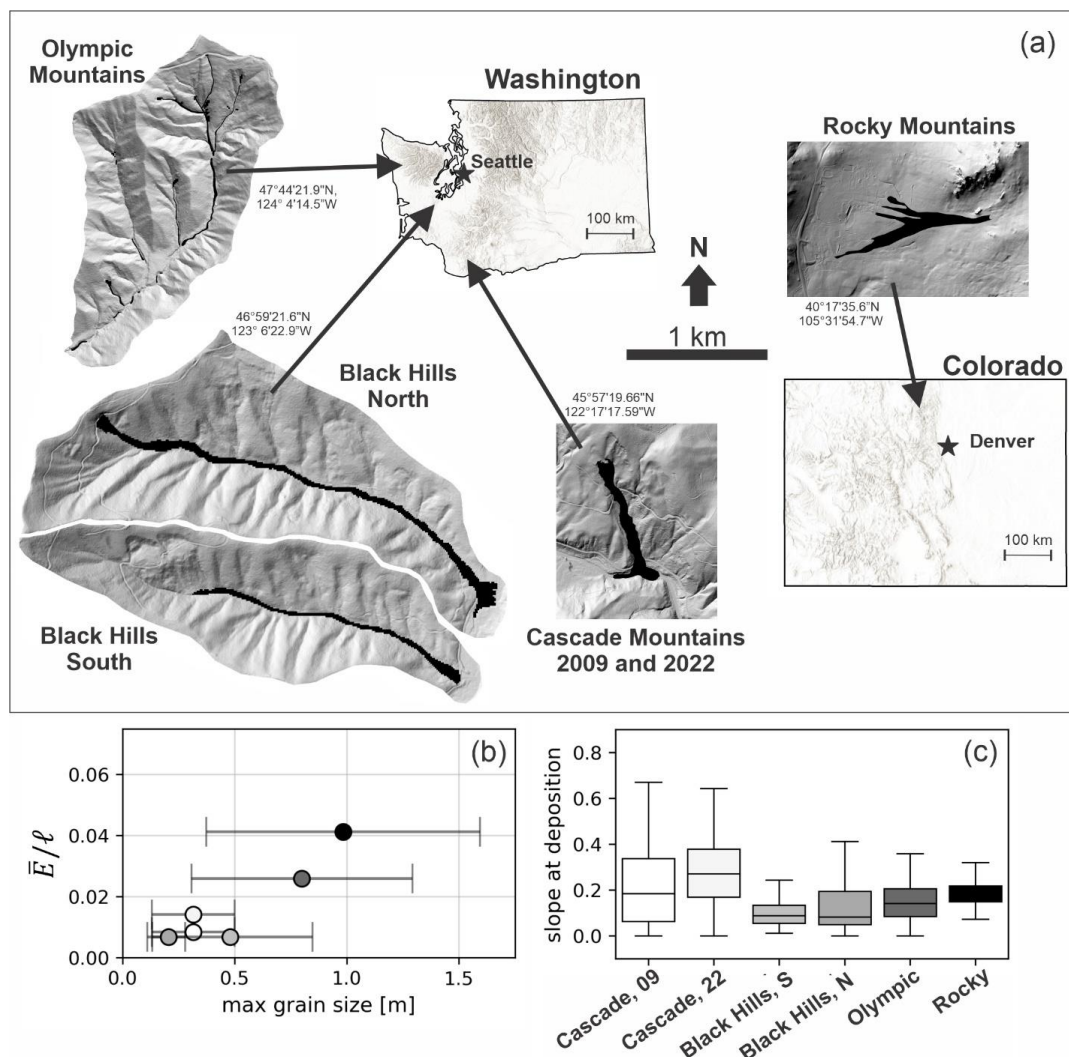
site	Cascade Mountain, 09	Cascade Mountain, 22	Black Hills, south	Black Hills, north	Rocky Mountain	Olympic Mountain
landslide length, ℓ [m]	185	55	80	75	40	45
landslide width [m]	80	50	15	65	35	15
landslide volume [m ³]	110,000	22,000	1,500	18,500	4,600	400 - 2,200
2-day cumulative precipitation + snowmelt [mm]	120+85	140+75	205+50	205+50	193+0	100 - 220 + ?
maximum grain size [m]	0.316	0.316	0.48	0.206	0.984	0.8
slope at positive-net deposition [%]	1 - 15	1 - 15	<1 - 10	<1 - 8	16 - 25	5 - 15
average flow depth in scour zone [m] *	4	2	2	3	3	3
average channel slope in scour zone [m/m]	0.25	0.25	0.15	0.15	0.4	0.3
average channel width in scour zone [m]	45	20	25	35	55	10
length of erosion, [m]	600	340	1210	1345	360	2550
erosion area, A [m ²]	28,400	6,600	22,800	52,400	20,800	28,900
total erosion volume, $\sum E\Delta x^2$ [m ³]**	44,547	5,125	12,332	26,815	34,275	33,725
average erosion per unit length of landslide, \bar{E}/ℓ , [m/m]	0.0085	0.014	0.0068	0.0068	0.041	0.026
k	0.020	0.034	0.017	0.020	0.076	0.051
growth factor, [m ³ /m]	74.2	15.1	10.2	19.9	95.2	13.2
average observed $ \Delta\eta $	2.4	2.2	0.53	0.63	0.89	1.4
total erosion volume / total mobilized volume***	0.29	0.19	0.89	0.59	0.88	0.97

483 * rough approximation based on landslide volume, channel width and height of scour marks in the scour zone of the
 484 channel

485 ** excludes landslide volume

486 ***total mobilized volume = total erosion volume+ landslide volume

487



488

489 **Figure 5** (a) Landslide locations in Washington and Colorado states. Coordinates next to each site are WGS84.

490 Shaded DEMs of each site are sized to the same scale. (b) Observed average erosion rate per unit landslide length

491 (\bar{E}/ℓ) relative to the observed average-maximum grainsize. Error bars indicate standard deviation. (c) Underlying

492 topographic slope of observed deposition locations.

493

494 4.2 Model setup, data acquisition and field reconnaissance

495 For each validation site, we used a combination of field and remote sensing observations to set up and parameterize

496 MWR. Each model was set up on a 10-meter grid, with the following gridded inputs: (1) the pre-event DEM; (2) a

497 map of regolith depth and (3) landslide boundary polygon, represented by all nodes within the polygon. If the landslide



498 slip plane depth differed from the regolith depth, a list of the slip plane depths at all nodes in the landslide polygon
499 were used to define the slip plane. For implementing the calibration utility, we prepared a DoD of each site.
500 In the field, we walked the runout path from the head of the landslide to the terminus of the runout deposit noting the
501 location and thickness of deposition and erosion, deposit composition, the slope of channel-filling deposits and
502 maximum grain size in the deposit. Geometric measurements in the runout path were measured using a combination
503 of a clinometer, steel tape and range finder. Location was measured using a handheld GPS with maximum accuracy
504 = +/- 1.8 m. Maximum grain size (which at all sites was cobble to boulder sized) was visually approximated in the
505 field or from photos that included a scale object, to the nearest 5 cm at eight to sixteen points along the runout path.
506 We estimated deposition depth from the thickness of fluvially eroded terraces and burial depth of stumps or trees. In
507 channels that were eroded to the fresh bedrock surface, we estimated erosion depth using the regolith thickness visible
508 along the edges of scoured channel walls (Olympic Mountain site). At other locations, we used vegetative indicators
509 such as the remnants of in-situ tree roots (Rocky Mountain site).
510 Remote observations included air photos (DNR, 2022; Google Earth, 2022) and lidar DEMs (DNR, 2022; Open
511 Topography, 2022) of the landslide and runout path. The lidar DEMs were used to define the model domain. The air
512 photos were used in combination with field observations and the lidar DEM to interpret the lateral extent of the
513 observed runout. Where pre- and post-mass wasting event lidar DEMs were available, we subtracted the pre-event
514 DEM from the post-event DEM to create the DoD. We found that not all DEMs or regions of the DEM were of equal
515 quality and therefore, where possible, DoD maps derived from lidar differencing were checked in the field. At sites
516 with only one post-event lidar DEM (Rocky Mountain and Olympic Mountain), we created DoD entirely from field
517 observations of deposition and erosion. Model setup details specific to each site are included in the Supplementary
518 Material.

519 **4.3 Estimating model parameter values from the observed runout**

520 We estimated all parameter values from field/remote observations of the observed runout except S_c and qs_c (See Table
521 1). For both S_c and qs_c , we defined uniform prior distributions based on field observations and then used the
522 calibration utility to find the best-fit parameter values (parameter values corresponding to the highest $L(\theta)$). Minimum
523 and maximum values of S_c were initially estimated from the range of observed slope of areas of positive-net deposition
524 (Table 1). Minimum and maximum values of qs_c were set as 0.01 to 1.75, which roughly represents the range of
525 minimum observed thickness of debris flow termini in the field at all of the validation sites. The range of S_c and qs_c
526 were later adjusted if we found the calibration utility tended to sample parameter values near the edge of the initial
527 parameter range. We approximated k from the DoD as follows:

528 The average erosion depth caused by the observed runout (\bar{E}) can be determined from the DoD as the total erosion
529 volume ($\sum E\Delta x^2$) divided by the erosion area (A) in the DoD:

$$530 \quad \bar{E} = \frac{\sum E\Delta x^2}{A} \quad (32)$$

531 where $\sum E\Delta x^2$ and A exclude the initial landslide volume and area, areas of deposition ($+\Delta\eta$) and areas with no
532 change in elevation ($\Delta\eta = 0$). In terms of the debris flow conceptualization used in MWR, \bar{E} can also be written as a



533 function of the mean number of times a debriton would need to pass over a grid cell (\bar{n}) multiplied by an average
534 erosion depth per debriton (\bar{h}_e) to equal \bar{E} as:

$$535 \quad \bar{E} = \bar{n}\bar{h}_e \quad (33)$$

536 An estimate for \bar{n} can be determined from the average length of the runout material, which we approximate simply as
537 the mapped landslide length (ℓ) divided by the cell width:

$$538 \quad \bar{n} = \frac{\ell}{\Delta x} \quad (34)$$

539 Note that if the observed runout formed as a result of multiple landslides (as was the case at the Olympic Mountain
540 site), then ℓ was determined as the sum of the initial landslide lengths. Also, as the debritons move down slopes in
541 excess of S_c , they entrain material, split, and spread, and the runout material tends to lengthen. Using the initial
542 landslide length to represent the runout length thus represents a minimum value for \bar{n} and if needed, (34) can be
543 multiplied by a coefficient to scale ℓ into a more representative runout length. Combining (33) and (34), \bar{h}_e can be
544 defined as the average erosion rate per unit length of landslide (\bar{E}/ℓ) times the cell width:

$$545 \quad \bar{h}_e = \frac{\bar{E} \Delta x}{\ell} \quad (35)$$

546 Rewriting (11) as a function of the average shear stress in the erosion-dominated reaches of the runout path ($\bar{\tau}$) and
547 assuming $\tau_c \cong 0$, debris flow erodibility parameter k can be estimated as:

$$548 \quad k = \frac{\bar{h}_e}{\bar{\tau}^m} \quad (36)$$

549 To solve for k , we estimated $\bar{\tau}$ from field-approximated debris flow depth and channel slope measurements in the
550 erosion-dominated reaches of the runout path. To estimate flow depth, we used the height of scour marks on the
551 channel wall or tree trunks, above the channel bed (Table 1). We used (13) to define $\bar{\tau}$. For D_s , we used the average
552 maximum grain size observed over the whole runout path. As noted in Section 2.3, if τ is defined as a function of
553 grain-collision dependent shear stress and k is determined as a function of m , as in (36), the impact of m on model
554 behavior is relatively small; however, the calibration utility did tend to more quickly converge on a set of best-fit
555 parameter values when a lower value for m was used and results herein use $m = 0.2$.

556 **4.4 Summary of observed runout characteristics**

557 The initial landslides (e.g., the initial landslide body) ranged in volume from 400 to 110,000 m³. At all sites, erosion
558 and subsequent entrainment added to the total mobilized volume (total erosion volume plus the initial landslide
559 volume, also referred to as the total flow volume or total debris flow volume; e.g., Barnhart et al. 2021; Reid et al;
560 2016), but the contribution was highly variable. The erosion volume divided by the total mobilized volume was as
561 low as 0.19 at the Cascade Mountain, 2022 landslide to as high as 0.97 at the Olympic Mountain landslides (Table 1).
562 The average maximum grain size varied from 0.2 m at the Black hills sites to nearly 1 meter at the Rocky Mountain
563 Site (Figure 5b, Table 1). Values of \bar{E}/ℓ ranged from 0.007 to 0.041 [m/m] with the highest rate occurring at the
564 Rocky Mountain landslide and the lowest at the Black Hills sites. For comparison with other reported debris flow
565 erosion rates, the observed erosion rates are also listed in terms of a yield rate or growth factor (average volumetric



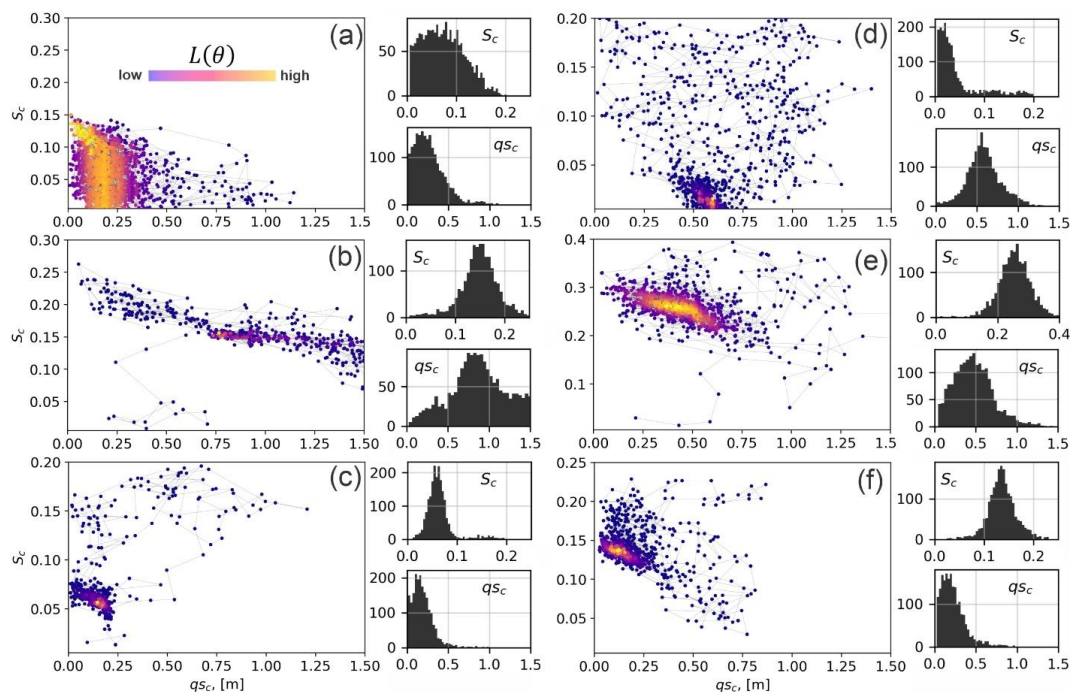
566 erosion per unit length of the erosion-dominated region of the runout path, Hungr et al. 1984; Reid et al., 2016). The
567 observed growth factors ranged from $10 \text{ m}^3/\text{m}$ at the Black Hills South site to $95 \text{ m}^3/\text{m}$ during the Rocky Mountain
568 landslide (Table 1).

569 The topographic slope at which observed deposition occurred, measured as the slope of all cells of the 10 m DEM
570 representation of the terrain in which observed $\Delta\eta > 0$, was as high as 65% at some sites (Figure 5c); however positive
571 net-deposition tended to occur at lower slopes (Table 1). The slope of areas with positive net deposition tended to be
572 lowest at the Black Hills site (<1% to 10%) and highest at Rocky Mountain site (16% to 25%).

573 **4.5 Calibration and model performance**

574 Markov chains, colored according to the likelihood index, $L(\theta)$ are plotted in the $S_c - qs_c$ domain, along with
575 histograms of sampled S_c and qs_c values for each landslide in Figure 6. The chains show a wide array of sampling
576 patterns and parameter ranges. Broadly speaking, at all sites, the chains converged, with growing $L(\theta)$, towards either
577 a small bullseye (b, c, d, f), or wider aerial patterns (a, e). In the former, the model shows high performance with only
578 a single or a very narrow range of few S_c - qs_c pairs while in the latter, similar model performance (nearly equivalent
579 $L(\theta)$ values) can be achieved or a wider range of sample pairs. The convergence patterns also vary.

580 The Cascade Mountain, 2009 (Figure 6a) and Cascade Mountain, 2022 (Figure 6b) landslides, which flowed over the
581 same hillslope, reveal that best-fit model parameters can vary at the same location. The calibration utility converged
582 to smaller qs_c and S_c values for the Cascade Mountain, 2009 event, which permitted thinner flow over lower slopes
583 and effectively made the 2009 modeled runout more mobile relative to the 2022 modelled runout. Even though MWR
584 model does not include an explicit representation of the rheology or momentum of the runout material, to a certain
585 degree, through calibration, it can be parameterized in relation to the initial volume and runout behavior of the slide
586 to implicitly represent those processes. On the other hand, the Cascade Mountain, 2009 and 2022 models used different
587 underlying initial DEMs and differences in parameterization could reflect the degree to which those DEMs represented
588 actual terrain (See Supplementary Material).



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Figure 6. MWR calibration results to landslides described in Section 4 for (a) Cascade Mountain, 2009; (b) Cascade Mountain, 2022, (c) Black Hills, South; (d) Black Hills, North; (e) Rocky Mountain and; (f) Olympic Mountain. Each result shows a scatter plot of the sampled S_c and qS_c values, colored by their relative $L(\theta)$ value. To the right of each scatter plot are histograms of the iterated S_c and qS_c parameters that improved the model results, which represent an empirical PDF of the possible S_c and qS_c values that calibrate MWR to each site.

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Additionally, inspection of MWR flow behavior at of the Cascade Mountain, 2009 slide (Figure 7) shows how modeled runout generally flows perpendicular to the topography lines of the underlying terrain, reflecting the impact of Equation 3 on flow direction. In Figure 8, profile plots of modeled Q_s (Equation 29) and maps of the modeled planimetric runout extent, colored to indicate where the runout matched (α), overestimated (β) or underestimated (γ) the observed runout (Equation 26) also illustrate observed and modeled flow behavior. At the Cascade Mountain, 2009 site, because a large portion of the transported sediment was lost to fluvial erosion in the valley, we limited the comparison of observed and modeled Q_s to the upper 750 m of the hillslope (see Supplementary Material).

In general, MWR successfully captures the spatial variation in Q_s and runout extent at most sites (Figure 8). The profile plots of Q_s at the Cascade Mountain site (Figure 8a and 8b) show that during the 2009 landslide, all of the runout material flowed past the first 750 meters of the runout path (Figure 8a) while during the 2022 landslide, material began to deposit just down slope of the initial landslide scar (Figure 8b). The model captures this behavior. At the Olympic Mountain site, MWR successfully replicates the observed spatially variable pattern of deposition and erosion (profile plot of Q_s in Figure 8f). This finding is notable, because at the Olympic Mountain site, the observed runout extent and sediment depositional pattern was formed by a sequence of landslides and subsequent runout events, rather than a single event, and each event altered the location of available sediment for the next. Also, at the Olympic

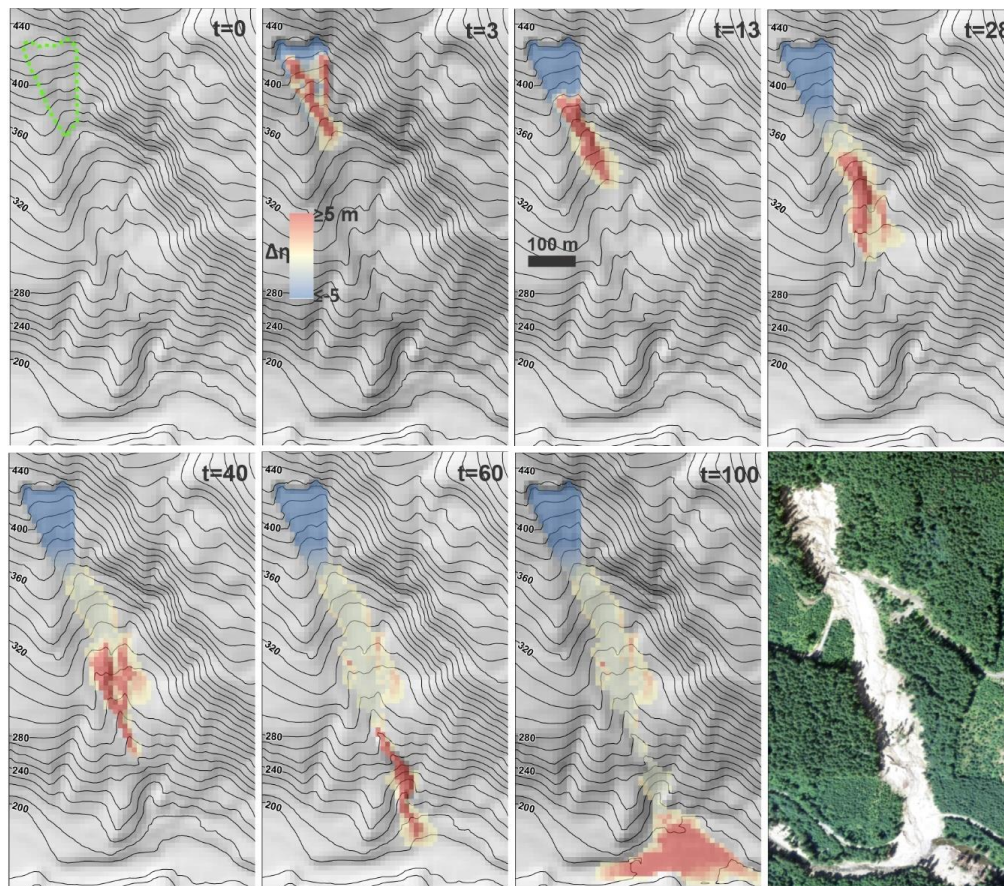


611 Mountain site, woody debris played a major role in determining the terminal extent of the runout (See Supplementary
612 Information). MWR does not include an explicit representation of woody debris impacts on runout. Furthermore, as
613 will be detailed below, topographic features of the channels that appeared to control observed runout were not well
614 represented in the 10-m DEM of the Olympic Mountain site. Nonetheless, the calibration utility found parameter
615 values that allowed the model to replicate the observed sediment transport and depositional behavior.

616 Regarding how well modelled runout replicated the planimetric extent of the observed runout, values of Ω_T are
617 comparable or higher than reported values of Ω_T for landslides in California that were modeled using a variety of
618 detailed, mechanistic and top-down models (Gorr et al., 2022; Barnhart et al., 2022; Note, to compare Ω_T values to
619 those studies, subtract 1 from values reported in this study). Overall, calibration was best at the Cascade Mountain,
620 2009 landslide (values of Ω_T are highest and values of $\Delta\eta_E$ and Q_{sE} are lowest) and poorest at the Rocky Mountain
621 and Olympic Mountain sites (Values of Ω_T are lowest Q_{sE} and $\Delta\eta_E$ are highest). As noted in Section 4.2, at both the
622 Rocky Mountain and Olympic Mountain sites, because we lacked repeat lidar, we created the DoD from a map of
623 field estimated erosion and deposition depths and estimated the pre-event DEM. The lower calibration scores may
624 indicate that field estimated DoDs or pre-event DEMs were not as accurate as those determined from repeat lidar
625 DEMs. Additionally, in the field at the Rocky Mountain site, standing trees and woody debris along the edge of the
626 runout path appeared to have controlled the lateral extent of the observed runout (See Supplementary Material). As a
627 noted above, MWR model does not explicitly represent of the impacts of wood on runout behavior, but by selecting
628 high qs_c values, the lateral extent of the modelled flow can be reduced. At the same time, increasingly qs_c reduces
629 the longitudinal extent of the modelled flow. The calibration utility identified S_c - qs_c pairs that allowed MWR to best
630 mimic the combined impact of woody debris and topography on the observed runout.

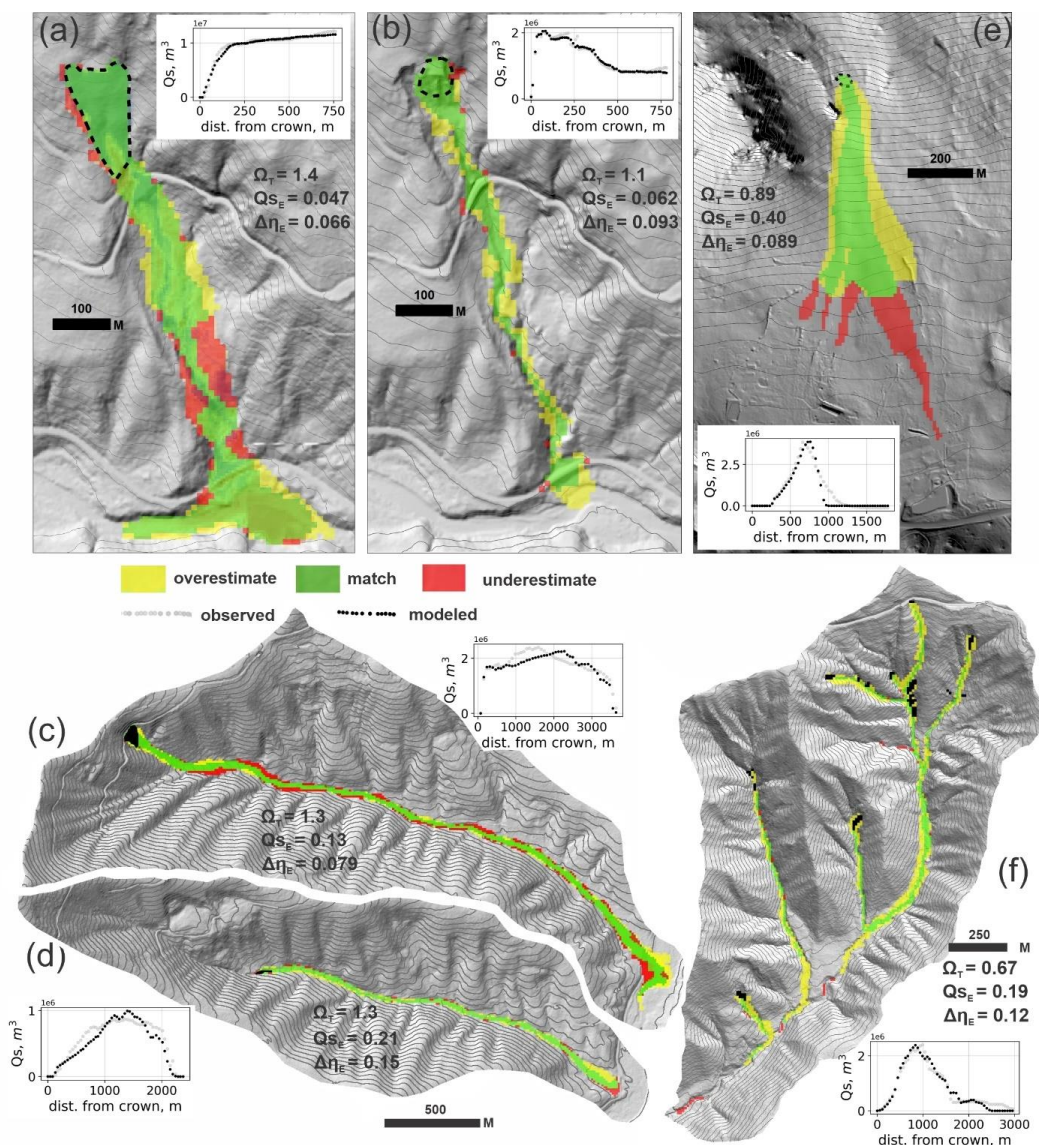
631 Finally, MWR also tended to underestimate the runout width of large landslides where the apparent momentum of the
632 observed the runout allowed it to flow upslope or slope-parallel. For example, after the modeled Cascade Mountain,
633 2009 landslide initially failed in the uplands, the debritons split and then rejoined to flow around a broad ridge in the
634 middle of the runout path and then were funneled into a narrow ravine (iteration $t=40$ in Figure 7), rather than flowing
635 over and inundating the broad ridge as was observed (Large red zone in Figure 8a; see Supplementary Material). On
636 the contrary, for small to moderately sized landslides that flowed in narrow, highly convergent channels, (Olympic
637 Mountain and Cascade Mountain, 2022 sites; Figures 8f and 8b). Overestimation of width is particularly pronounced
638 across most locations of the channel network at the Olympic Mountain site.

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642 **Figure 7.** Illustration of modeled runout at the Cascade Mountain, 2009 landslide. At iteration $t = 0$, Algorithm 1
643 determines the direction and flux of the initial debritons (all nodes located in the landslide green-dashed polygon). In
644 later iterations, Algorithm 2 routes the debritons down slope, entrains material, forms new debritons and updates the
645 terrain. By the end of the modeled runout, a colluvial fan forms at the base of the slope. Topography lines reflect the
646 underlying terrain, which is updated after each iteration.



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Figure 8. Calibrated model performance as indicated by modeled runout extent, profile plots of Q_s , and reported values of Ω_T , $\Delta\eta_E$ and Q_{SE} . In all maps, up is north except in (e), north is towards the left. (a) Cascade Mountain, 2009; (b) Cascade Mountains, 2022; (c) Black Hills, South; (d) Black Hills, North; (e) Rocky Mountain; (f) Olympic Mountain.

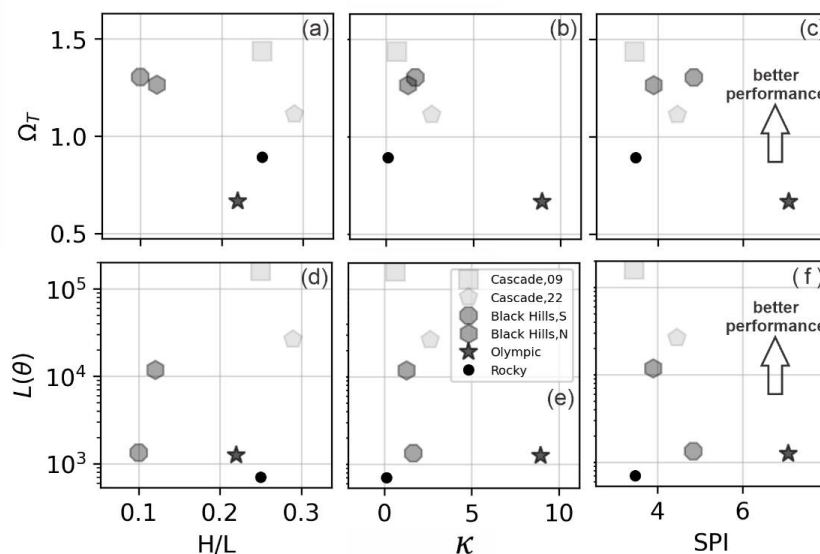
653 To understand whether the ability to calibrate MWR systematically varies with topography of the runout path, we
654 compared model performance with three topographic indices computed from the terrain in the observed runout extent.
655 The indices include the relief ratio (H/L), mean total curvature (κ) and the mean specific stream power index (SPI).
656 The index H/L equals the average slope of the runout path (or relative relief), determined as the total topographic relief



657 of the runout (measured from the top of the initial landslide to the end of the runout path) divided by the horizontal
658 length of the runout and indicates the internal and external resistance to the observed mass wasting runout (Iverson,
659 1997). Index κ represents topographic curvature, which is the second derivative of the terrain surface, with
660 increasingly positive values of index κ reflecting increasing topographic concave-up and convergent topography (e.g.,
661 Istanbuloglu et al., 2008). The index SPI is determined as the natural log of the product of the contributing area and
662 slope. We computed indices κ and SPI at each node in the observed runout extent and the mean was determined from
663 all nodes in observed runout extent. For a more detailed description of the topographic indices, see , Chen and Yu,
664 (2011).

665 Comparison of model performance with respect to the topographic indices in Figure 9 shows: slightly improved model
666 performance over runout-paths that are less convergent (SPI and κ values of the observed runout path are lower) and
667 slightly improved performance on steeper terrain (higher H/L). The former finding may have to do with how well
668 topographic features that controlled the observed runout were represented by the model DEM. In all models, we used
669 a 10-meter DEM (see Section 4.2), which may have lacked sufficient topographic detail to represent the narrow first
670 and second order channels that controlled observed runout of the Olympic Mountain and the Cascade Mountain, 2022
671 sites (Supplementary Material). Without a sufficiently accurate topographic representation of the topographic features
672 that controlled observed flow, modeled flow tended to flow over a wider area of the hillslope than observed (see Figure
673 8b and 8g). The latter finding appears to be mostly a result of how well modeled sediment transport and topographic
674 change (Q_{sE} and $\Delta\eta_E$) replicated observed, as there does not appear to be a trend in Ω_T with H/L and the two best
675 performing models (Cascade Mountain landslides) both had the lowest Q_{sE} values and low $\Delta\eta_E$ values.

676 In summary, using a variety of methods to prepare the DoD and model terrain, the calibration utility was able to
677 identify clear best-fit S_c and qs_c values at a range of different landslide types. Calibration likelihood ($L(\theta)$) tended to
678 vary with topographic indices of the runout path (H/L, κ and SPI). Model performance was impacted by how well the
679 DEM represented topographic features that controlled observed runout, the importance of runout processes not
680 explicitly represented in MWR on observed runout behavior (such standing trees and woody debris along the edge of
681 the Rocky Mountain landslide) and the accuracy of the DoD dataset (field mapped vs lidar DEM determined). To a
682 certain degree, though calibration, MWR can be parameterized to compensate for deficiencies in the DEM or processes
683 not explicitly represented in the model.



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685 **Figure 9.** Illustration of model calibration, as reflected by the posterior parameter likelihood $L(\theta)$ and planimetric
686 fit (Ω_T) relative to topographic indices.

687 **5. Discussion**

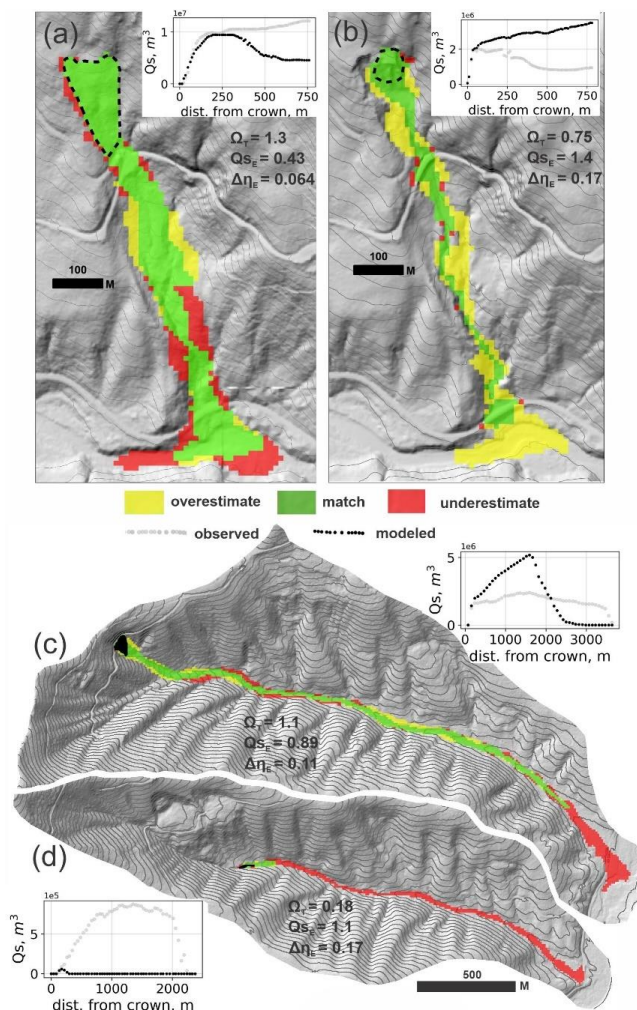
688 **5.1 Strategic testing of MWR for hazard mapping applications**

689 Having demonstrated that MWR can be calibrated to a variety of landslides and runout terrains, we now strategically
690 test MWR using the Cascade Mountain and Black Hills sites. Since both of these sites consisted of two separate
691 landslides, we can thus test model performance by swapping best-fit model parameters, rerunning the models and
692 comparing results with the original, calibrated results. At the Cascade Mountain site, the 2009 and 2022 landslides
693 originated on the same hillslope (Figure 8a and 8b). At Black Hills site, the two landslides occurred on different
694 hillslopes but in adjacent east-west oriented watersheds (Figure 8c and 8d).

695 As shown in Figure 10, at three of the landslides (both Cascade Mountain landslides and the Black Hills, North
696 landslide), when the best-fit parameters from the other landslide are used to predict runout, the accuracy of modeled
697 runout planimetric extent drops but resultant Ω_T values can still be as high or higher than values reported in other
698 studies (compare to equivalent Ω_T values in Gorr et al., 2022 and Barnhart et al., 2022). In terms of modeled sediment
699 transport and topographic change, swapping best-fit parameters has a larger effect. At the Cascade Mountain, 2009
700 landslide, using the 2022 best-fit parameter values causes about half of the modeled runout material to prematurely
701 deposit on the hillslope, reducing the amount of sediment that reaches the valley floor (Q_{sE} increases by a factor of
702 nine; Figure 10). Using the Cascade Mountain, 2009 parameter values for Cascade Mountain, 2022 landslide (Figure
703 10b) increases modeled runout extent and results in nearly four times the entrainment and transport of sediment to the
704 valley floor, causing Q_{sE} to increase by a factor of 20 and $\Delta\eta_E$ by 83%. At the Black Hills site, using the South basin
705 best-fit model parameters at the North basin causes Q_{sE} and $\Delta\eta_E$ increase by 83% and 39% respectively (Figure 10c).



706 Unlike the other three landslides, swapping best-fit parameters at the Black Hills, South landslide results in both large
 707 sediment transport and runout extent error because the North basin best-fit parameters cause modeled landslide to
 708 entrain too little and stop only a few hundred meters from the initial source area (Figure 10d).



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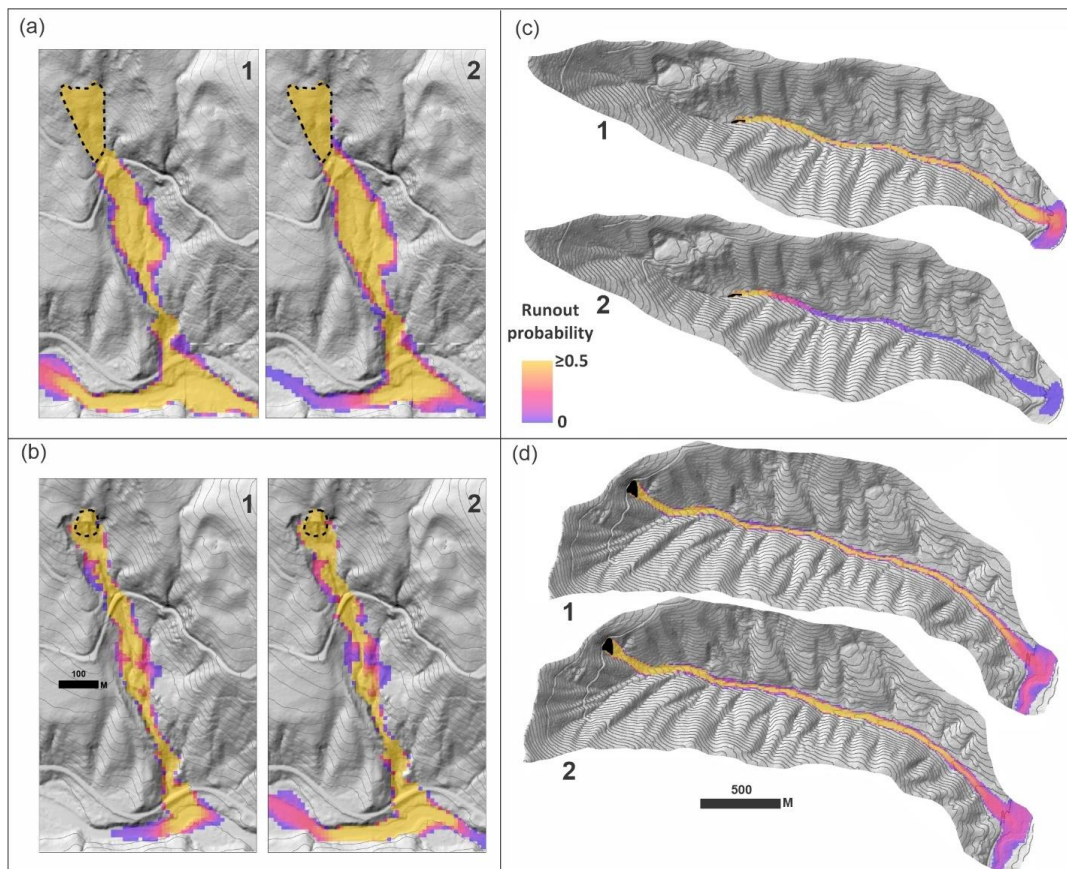
710 **Figure 10.** Model performance using the neighboring landslide parameter values, as indicated by modeled runout
 711 extent, profile plots of Q_s , reported values of Ω_T , $\Delta\eta_E$ and Q_{sE} and profile plots of Q_s . Compare with Figure 8. (a)
 712 Cascade Mountain, 2009; (b) Cascade Mountain, 2022; (c) Black Hills, North; (d) Black Hills, South

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714 An alternative test to simply swapping the best-fit parameters is to swap parameter PDFs determined from the
 715 calibration utility and compare probabilistic runout. For this test, we run MWR Probability with option (1), using the
 716 field mapped landslide polygons. As shown in Figure 11, similar to the first test, at three of the landslides, using the
 717 parameter distribution associated with the neighboring landslide results in relatively minor changes in the probability
 718 of runout (probability of $\geq 50\%$; Figures 11a, 11b and 11d) and mostly changes the terminal extent of the possible



719 landslide runout area. At the Black Hills South landslide, swapping parameter PDFs causes a large change in runout
 720 probability (Figure 11c).
 721



722 **Figure 11.** Model tests by swapping parameter PDFs and comparing runout probability at the (a) Cascade Mountain,
 723 2009; (b) Cascade Mountain, 2022; (c) Black Hills, South and; (d) Black Hills, North sites. (1) runout using
 724 parameter distributions of the site and (2) runout using parameter distributions of the neighboring site.
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727 The results of these two tests suggest that in most cases, once best-fit parameters or parameter PDFs have been
 728 established for a landslide, those parameter/PDF values can then be used to assess runout extent at nearby sites.
 729 Unfortunately, it appears the same cannot be said for sediment transport and topographic change prediction, both of
 730 which appear to be more sensitive to model parameterization. However, we suspect that this finding is a consequence
 731 of comparing very different landslide types to conduct the tests. Had landslide processes at the Cascade Mountain and
 732 Black Hills sites been more uniform, test results would have been better. For example, at the Olympic Mountain site,
 733 a single parameterization seemed to capture both the runout extent and depositional patterns of all slides. There, most
 734 landslides tended to be stream adjacent and relatively uniform in size. In contrast, at the Cascade Mountain site, the
 735 2009 landslide was five times the volume of the 2022 landslide and both the volume and apparent momentum of the
 736 2009 slide allowed it to flow topographically unconstrained while the much smaller 2022 landslide was



737 topographically confined to a narrow channel ravine. The calibration utility found that relatively low S_c and qs_c values
738 best replicated the Cascade Mountain, 2009 flow behavior while higher S_c and qs_c values best replicated the Cascade
739 Mountain, 2022 behavior. Similarly, at the Black Hills site, the north watershed landslide was over ten times the
740 volume of the southern watershed landslide, and initiated 150 hundred meters above the channel, on a steep hillside
741 (detailed in Supplementary Material) while the much smaller south watershed landslide initiated at the elevation of
742 the channel bed. Again, differences in landslide behavior equated to different parameterizations of MWR. Had the
743 Cascade Mountain, 2009 and 2022 landslides or the Black Hills North and South landslides been similar in size and
744 runout behavior, best-fit S_c and qs_c values and PDFs may have been similar and test results may have been better.
745 In summary, the Cascade Mountain and Black Hills sites each contained two landslides that we could use to
746 independently calibrate MWR and thereby evaluate model predictive ability by swapping parameter values and
747 comparing model results with calibrated model results. At three of the landslides, the tests revealed a modest decrease
748 in the accuracy of modeled runout extent (though accuracy of the reduced model extent was still comparable to values
749 reported in the literature) but a substantial decrease in the accuracy of modelled sediment transport and modeled
750 topographic change. We suspect that the poor sediment transport and topographic change test results are a consequence
751 of comparing very different landslide types and runout processes. In regions where landslide processes are relatively
752 uniform (like the Olympic Mountain site), calibration to one landslide might be sufficient to predict the depositional
753 patterns of another. At sites like the Cascade Mountain and Black Hills sites, which consisted of a diverse range of
754 landslide processes, MWR may need to be calibrated to each type of landslide (e.g., small-low-slope vs. large-steep-
755 slope) and predictive application of might involve applying the appropriate parameter set based on landslide type.

756 **5.2 MassWastingRunout probability applications**

757 In this section we demonstrate how to determine runout probability from a probabilistically determined landslide
758 hazard map or a specific, potentially unstable slope using MWR. The first application may be appropriate for
759 watershed- to regional-scale runout hazard assessments. The second application is an example site-scale hazard
760 assessment. Both applications are demonstrated at the Olympic Mountain site where landslide size and type tended to
761 be relatively uniform and parameter PDFs determined through calibration may therefore represent typical runout
762 processes in the basin.

763 **5.2.1 Runout probability from a landslide hazard map**

764 To determine runout probability from a landslide hazard map, we ran MWR Probability using option (3) at the
765 Olympic Mountain site. We use LandslideProbability (Strauch et al., 2018), an existing component in Landlab that
766 computes landslide probability by repeatedly determining the hillslope Factor-of-Safety (FS : ratio of the resisting to
767 the driving forces) at each node on the raster model grid from stochastically selected soil (regolith) hydrology
768 properties, (including soil depth, saturated hydraulic conductivity) soil strength (friction angle, cohesion) and recharge
769 rates (precipitation input rate minus evapotranspiration and soil storage). We setup LandslideProbability using soil
770 strength and hydrologic parameters based on the soil class (SSURGO, 2020) and vegetation type following Strauch et
771 al. (2018). We defined soil depth as a function of contributing area, slope gradient and minimum and maximum depths



772 following Westrick (1999). Minimum and maximum soil depths were estimated from field observations at channel
773 banks, tree-throw pits and road cuts. For recharge, we assumed evapotranspiration and soil storage losses were small
774 and used a uniform daily precipitation depth equivalent to the 50 year event (determined from rainfall records, WRCC,
775 2017).

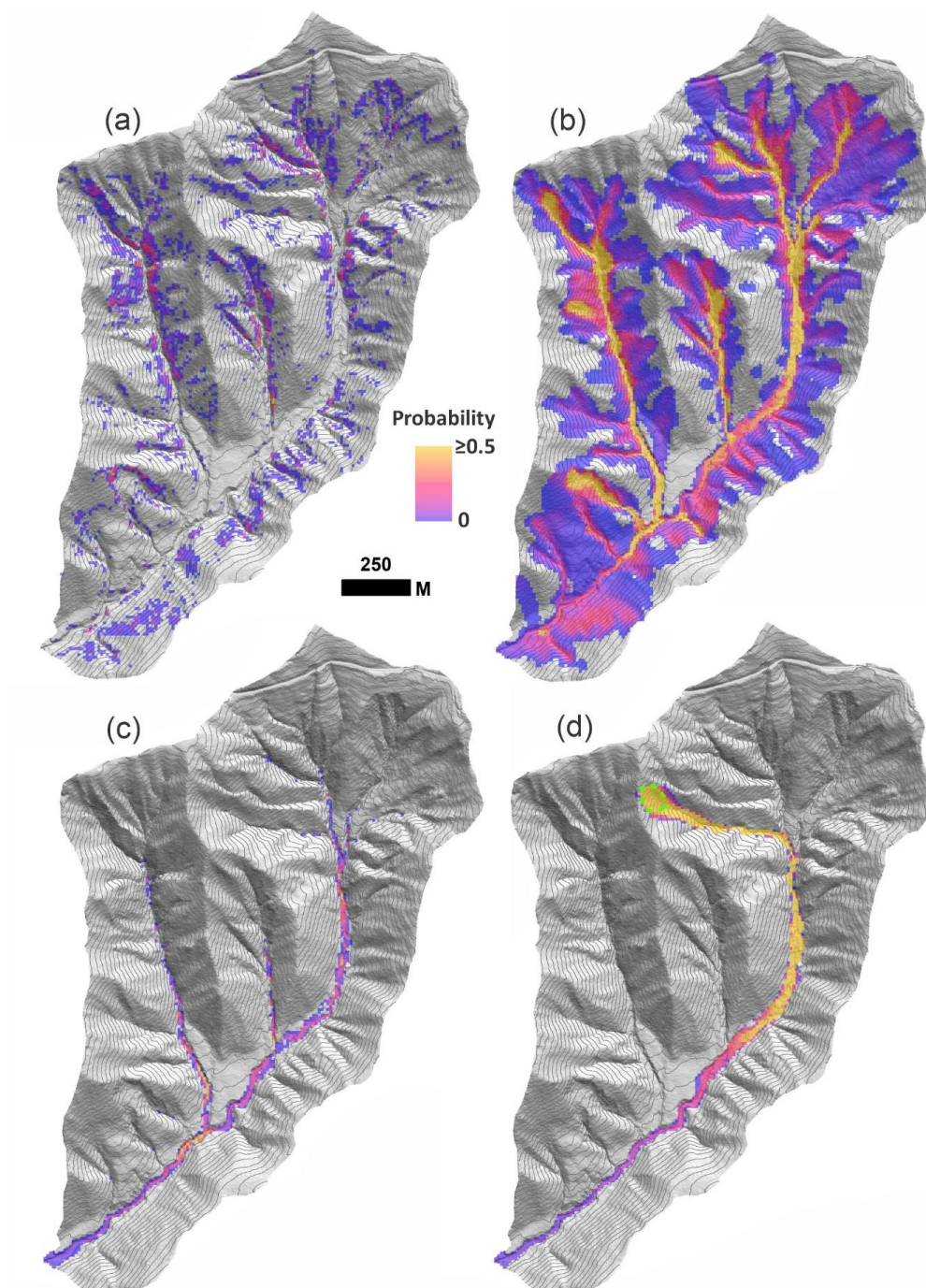
776 We ran $N = 1000$ model runs. For each model run, FS and soil depth maps were saved and after all N runs were
777 completed, a single map of landslide probability was produced. We then ran MWR Probability using each of the N
778 FS and soil depth maps and treated all nodes with $FS < 1$ as a landslide source. Each repetition began with the same
779 initial topography but different landslide locations and regolith thickness. Landslide probability and landslide runout
780 probability are shown in Figures 12a and 12b. If changes in the elevation of the channel associated with landslide
781 runout were a concern, results of MWR Probability can also be used determine the probability of scour or deposition.
782 For example, the probability of deposition greater than 1 meter is shown in Figure 12c.

783 This example application of MWR Probability shows that while landslide probability is relatively low over most of
784 the Olympic Mountain basin, runout probability is high in many of the second order channels but then low again at
785 the basin outlet. In a field study of the spatial and temporal patterns of debris-flows, May and Gresswell (2004)
786 observed that debris flow return interval tends to be low (debris flow probability tends to be high) in channels which
787 are fed by a greater number of upslope landslide source areas; however, our model results illustrate how debris flow
788 (landslide runout) probability is both a function of the number of upslope landslide source areas as well as the typical
789 runout distance of those source areas. For example, despite having the largest number of potential landslide source
790 areas, runout probability at the basin outlet is low because it is located beyond the typical runout distance of landslides
791 at the Olympic Mountain site.

792 **5.2.2 Runout probability for a specific, potentially unstable slope**

793 When field evidence or other data indicate that a specific slope may be potentially unstable, but the exact area of a
794 potential slide on that slope is unknown, MWR can be used to determine potential runout hazard of the slope. For this
795 application, MWR Probability is run using option (2) which requires a polygon representing the maximum extent of
796 the potentially unstable slope. As noted in Section 3.2, for each model repetition, a landslide area is randomly selected
797 within the potentially unstable slope, it can form anywhere within the potentially unstable slope and is least as large
798 as a user defined minimum size but no larger than the potentially unstable slope. After a user defined N repetitions,
799 runout probability of the potentially unstable slope is determined at each downslope node from (31).

800 As an example application of using MWR Probability option (2), we designated a 0.6 ha, zero-order basin in the
801 headwaters of the Olympic Mountain site as a potentially unstable slope and modeled runout probability using $N =$
802 1000 (Figure 12d). This example shows that if a landslide were to initiate at the potentially unstable slope, the
803 probability of that material running out to the basin outlet as a mass wasting runout process is less than 5%. This result
804 is again a function of the typical runout distance of landslides at the Olympic Mountain site.



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Figure 12. Olympic Mountain site: (a) Landslide probability, $P(FS \leq 1)$. (b) Corresponding runout probability, $P(\Delta\eta)$. (c) Probability of deposition greater than 1 m and (d) Runout probability for the potentially unstable slope (green-dashed polygon).



809 5.3 Model limitations and strengths

810 In the field, we observed a number of runout phenomenon that are not explicitly represented by MWR (detailed in
811 Supplementary Material), such as the apparent affect of momentum, debris fluidization, sorted deposition and the
812 impact of standing vegetation and woody debris on the runout process. For example, observed runout at the Olympic
813 Mountain site appeared to be restricted by log jams which in turn lead to large, valley-filling sediment deposits. At
814 the Rocky Mountain site, the lateral extent of the runout path appeared to have been restricted by standing trees. Also,
815 the momentum or perhaps fluidization of the initial landslide appears to have impacted small areas of the runout extent
816 at Black Hills North landslide and the Cascade Mountain, 2009 landslide.

817 Additionally, grid-size appears to have affected the lateral extent of modeled runout at some of the sites. At all sites,
818 we used a 10-m grid size and the topographic detail of the channels that constrained observed flow to a narrow width
819 at the Olympic Mountain and Cascade Mountain, 2022 sites may not have been well represented. On the other hand,
820 for landslides like the Cascade Mountain, 2009 slide and the Rocky Mountain slide, the 10-m grid size more than
821 adequately captured the key topographic features that controlled runout width and a lower resolution (larger grid size)
822 DEM may have resulted in equivalent model performance. This last point is important because the number of debritons
823 and computations needed to model the initial landslide scales with the initial landslide area divided by the square of
824 the grid size (e.g., a 100 m² landslide on a 10 m grid is represented by 1 debriton but on a 1-m grid is represented by
825 100 debritons). Using an excessively small grid size may result in an unnecessary number of computations. We used
826 a 10-m grid size because we assumed it was the coarsest grid size that also captured the topographic features that
827 controlled observed runout. In future work, we hope to provide a more comprehensive assessment of grid size impact
828 on model performance.

829 Nonetheless, as noted in Section 4.5, to a certain degree, through calibration, MRW can be parameterized to
830 compensate for low DEM resolution or the absence of explicit representation of all processes that controlled the
831 observed flow: at the Rocky Mountain site, the qs_c parameter could be used to mimic the resistance to lateral spreading
832 caused by the trees; at the Olympic Mountain site, even though modeled flows tended to be wider than observed and
833 woody debris affected deposition, the calibration utility identified qs_c and S_c values that allowed MWR to replicate
834 the longitudinal sediment transport behavior of the observed runout; and at the Cascade, 2009 site, the calibration
835 utility identified parameter values that allowed modelled flow to better mimic the high momentum behavior of the
836 observed runout.

837 Finally, through strategic testing, we showed that once MWR is calibrated to a landslide it may reasonably evaluate
838 the extent of runout or runout probability at neighboring slopes. We suspect that this may be particularly true in areas
839 that have relatively uniform landslide processes (Olympic Mountain site). At sites like the Black Hills and Cascade
840 Mountain sites, where landslide processes are highly variable, MWR may have limited predictive ability for sediment
841 transport and topographic change modeling and in some cases even runout extent unless the model is calibrated to
842 each type of landslide.

843 The utility of a geomorphic model is ultimately judged by its ability to explain or predict natural phenomena, even if
844 the interactions within the model greatly simplify the physical processes controlling the phenomena (Murray 2007;
845 Murray 2013). Furthermore, a model should be rejected if it fails to react to changes in forcing in a reasonable way or



846 cannot replicate the intended phenomena. From this standard, MWR appears to pass as an acceptable model, because
847 it was able to replicate a range of landslide behavior, including the Olympic Mountain site, where the final depositional
848 pattern was the result of a sequence of landslides and because it responds in an expected way to changes in terrain or
849 parameters like S_c , qs_c and grain size. MWR is a parameterized model and while the predictive application of MWR
850 may not be as restricted as implied by Iverson (2003), we suspect that calibration may be required for most
851 applications. For the purpose of facilitating rapid calibration to different sites and developing parameter PDFs, we
852 made MWR parameterization as field-relatable as possible and developed a calibration utility.

853 **6.0 Conclusion**

854 In this study, we described, calibrated and tested MassWastingRunout (MWR), a new cellular-automata landslide
855 runout model designed for probabilistic landslide hazard assessments, sediment transport and topographic change
856 applications. MWR is implemented in Python as a component for the Landlab earth surface modeling toolkit. MWR
857 includes a Markov Chain Monte Carlo calibration utility that determines the best-fit parameter values for a site as well
858 as empirical Probability Density Functions (PDF) of the parameter values. MWR also includes a utility called MWR
859 Probability that takes the PDF output from the calibration utility to model runout probability. It has three options for
860 defining the initial landslide: (1) a single, fixed-size landslide, (2) a single, variable-size landslide within a user defined
861 potentially unstable slope or (3) a single or multiple landslides that change in location and size each model iteration.
862 We used the calibration utility to parameterize the model to six different landslides. The following results were
863 obtained:

- 864 • MWR can be calibrated to a range of landslide types. The ability to calibrate MWR is slightly better on a
865 steep (high H/L) and broadly convergent (lower κ and SPI) runout path. But this finding may partly be a
866 consequence of the 10-m grid-size used to model runout at each of the six landslides, and needs further
867 research. The six landslides varied from small to large and the ability to calibrate MWR appears to be better
868 at the sites in which the terrain features that controlled runout extent were larger than the grid size.
- 869 • The two model parameters, critical slope (S_c), and a threshold flux for deposition (qs_c) are found to
870 effectively control model performance over a range of landslide and landscape conditions and, to a certain
871 degree, through calibration, MWR can be parameterized to mimic the effect of processes or terrain not
872 explicitly represented in the model.
- 873 • Once MWR is calibrated to an observed-runout dataset, it may work as a predictive tool for assessing the
874 runout extent of neighboring potential landslides. To predict depositional and topographic change, depending
875 on the diversity of landslide types in the region, a more rigorous calibration may be required such as
876 calibrating the model to different landslide types.
- 877 • We demonstrated how to use MWR to determine mass wasting runout probability from an expert-defined
878 potentially unstable slope polygon and a landslide hazard map produced by the model `LandslideProbability`.



879 **Competing interests**

880 The contact author has declared that none of the authors has any competing interests.

881 **References**

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