



# 1 **Seismic wave modeling of fluid-saturated fractured porous rock:** 2 **Including fluid pressure diffusion effects of discrete distributed large-** 3 **scale fractures**

4 Yingkai Qi<sup>1,2</sup>, Xuehua Chen<sup>1,2</sup>, Qingwei Zhao<sup>1,2</sup>, Xin Luo<sup>1</sup>, Chunqiang Feng<sup>3</sup>

5 <sup>1</sup>State Key Laboratory of Oil & Gas Reservoir Geology and Exploitation, Chengdu University of Technology, Chengdu,  
6 610059, China

7 <sup>2</sup>Key Laboratory of Earth Exploration & Information Techniques of Ministry of Education, Chengdu University of Technology,  
8 Chengdu, 610059, China

9 <sup>3</sup>Exploration & Development Research Institute of Henan Oilfield, Sinopec, 473000, China

10 *Correspondence to:* Xuehua Chen (chen\_xuehua@163.com)

11 **Abstract.** The scattered seismic waves of fractured porous rock are strongly affected by the wave-induced fluid pressure  
12 diffusion effects between the compliant fractures and the stiffer embedding background. To include these poroelastic effects  
13 in seismic modeling, we develop a numerical scheme for discrete distributed large-scale fractures embedded in fluid-saturated  
14 porous rock. Using Coates and Schoenberg's local effective medium theory and Barbosa's dynamic linear slip model  
15 characterized by complex-valued and frequency-dependent fracture compliances, we derive the effective viscoelastic  
16 compliances in each spatial discretized cell by superimposing the compliances of the background and the fractures. The  
17 effective governing equations of the fractured porous rock are then characterized by the derived anisotropic, complex-valued,  
18 and frequency-dependent effective compliances. We numerically solved the effective governing equations by mixed-grid  
19 stencil frequency-domain finite-difference method. The good consistency between the scattered waves off a single horizontal  
20 fracture calculated using our proposed scheme and those calculated using the poroelastic linear slip model shows that our  
21 modeling scheme can properly include the FPD effects. We also find that for a P-point source, the amplitudes of the scattered  
22 waves from a single horizontal fracture are strongly affected by the fluid stiffening effects due to fluid pressure diffusion, while  
23 for an S-point source, the scattered waves are less sensitive to fluid pressure diffusion. In the case of the conjugate fracture  
24 system, the scattered waves from the bottom of the fractured reservoir and the reflected waves from the underlying formation  
25 are attenuated and dispersed by the FPD effects for both P- and S-point sources. The proposed numerical modeling scheme  
26 can also be used to improve migration quality and the estimation of fracture mechanical characteristics in inversion.

## 27 **1 Introduction**

28 Fluid saturated porous **rock in the reservoir** characterized by a heterogeneous internal structure consisting of a solid skeleton  
29 and interconnected fluid-filled voids, are often permeated by much more compliant and permeable fractures. Although the  
30 fractures typically occupy only a small volume, they tend to dominate the overall mechanical and hydraulic properties of the



31 reservoir (Liu et al., 2000; Gale et al., 2014). Thus, fracture detection, characterization and imaging are of great importance  
32 for **reservoir prediction** and production. Seismic waves are widely used for these purposes because their behaviors (amplitude,  
33 phase and anisotropy) are strongly affected by the fractures (Chapman, 2003; Gurevich, 2003; Brajanovski et al., 2005;  
34 Carcione et al., 2011; Rubino et al., 2014). Therefore, appropriate numerical modeling methods are required for the  
35 interpretation, migration and inversion of seismic data from porous media containing discrete distributed fractures.  
36 Biot's poroelastic theory (Biot, 1956a; b) is the fundamental theory to describe elastic wave propagation in fluid porous media,  
37 including the dynamic interactions between rock and pore fluid. However, the original theory, assuming a macroscopically  
38 homogeneous porous media saturated by a single fluid phase, is fail to explain the measured velocity dispersion and attenuation  
39 of seismic waves (Nakagawa et al., 2007). In recent decades, many researchers found that if porous media contains mesoscale  
40 heterogeneity (ignored by Biot), a local fluid-pressure gradient will be induced by the passing wave at scale comparable to the  
41 wave-induced fluid pressure diffusion length (the wavelength of slow P-wave), causing significant velocity dispersion and  
42 velocity attenuation at seismic frequency band (White et al., 1975; Dutta and Odé, 1979; Johnson, 2001; and Müller et al. 2008;  
43 Norris, 1993; Gurevich et al., 1997; Gelinsky and Shapiro, 1997; Kudarova et al., 2016). Fractures embedded in homogeneous  
44 porous background are special heterogeneities, exhibiting strong mechanical contrasts with background. When seismic waves  
45 travel through fluid saturated fractured porous rocks, local fluid pressure gradients will be induced between the fractures and  
46 the background in response to the strong compressibility contrast. To return the equilibrium state, fluid pressure diffusion (FPD)  
47 occurs between the fractures and the embedding background, which in turn changes the fluid stiffening effect on the fractures  
48 and thus their mechanical compliances depending on frequency (Barbosa et al., 2016a, b).  
49 When fractures with apertures and lengths much smaller than the wavelengths are unified distributed in porous rock, the  
50 properties of fractured rock are homogeneous at macroscopic scale and can be described by a representative elementary volume  
51 (REV). Various effective medium theories are available for estimating the fracture-induced anisotropy, attenuation and  
52 dispersion behaviors (Hudson, 1981; Thomsen, 1995; Chapman, 2003; Brajanovski et al., 2005; Krzikalla et al. 2011; Galvin  
53 et al., 2015; Guo et al., 2017a; b). The discrete distributed large-scale fractures (**the presence of spatial correlations of fractures**),  
54 however, cannot be modeled by any above-mentioned effective medium theories originally for macroscopically uniformly  
55 distributed fractures. The seismic response of individual fracture is mostly assessed in the framework of the linear slip model  
56 (LSM) by modeling a fracture as a nonwelded interface across which the displacement tensors are assumed to be discontinuous  
57 while the stress tensors are continuous (Schoenberg, 1980). Various **local** numerical schemes have been developed for discrete  
58 distributed large-scale fractures. The most widely used scheme is local **effective-medium** schemes (Coates and Schoenberg,  
59 1995; Igel et al., 1997; Vlastos et al., 2003; Oelke, et al., 2013) that determine and incorporate the behavior of fracture-induced  
60 media within each spatial discretized cell. The advantage of using the local effective medium is that it requires no special  
61 treatment of the displacement discontinuity conditions on the fractures. An alternative scheme is the explicit interface scheme  
62 that directly treat the displacement discontinuity across each fracture (Zhang, 2005; Cui et al., 2018; Khokhlov, et al., 2021).



63 The common aspect of the aforementioned numerical modeling schemes is that they are all implemented in a purely elastic  
64 framework with real-valued compliances boundary and represent both the embedding background and fractures as elastic solids,  
65 thus the impact of FPD effects on seismic scattering can't be accounted for. A dynamic linear slip model incorporating FPD  
66 effects should be considered when implementing numerical modeling of seismic wave propagating in fluid saturated porous  
67 rocks containing discrete distributed large-scale fractures. Rubino et al. (2015) proposed a frequency-dependent complex-  
68 valued normal compliance for regularly distributed planar fractures (a set of aligned fractures) with a separation much smaller  
69 than the prevailing seismic wavelength. Despite the ability of including the FPD across the fractures, the model is not suitable  
70 for modeling discrete distributed fractures. Nakagawa and Schoenberg (2007) developed an extended LSM for a single fracture  
71 in the context of poroelasticity. The proposed model representing both the background and the fracture as poroelastic media  
72 can appropriately incorporate the frequency related effects, but it will also result in a higher computational consuming and  
73 more memory requirements. In the context of viscoelasticity, Barbosa et al. (2016a) developed a viscoelastic linear slip model  
74 (VLSM) for an individual fracture with explicit complex-valued and frequency-dependent fracture compliances, to account  
75 for the impact of FPD on the fracture stiffness. That provides a viscoelasticity-based modeling algorithm for discrete distributed  
76 large-scale fractures with smaller computational costs and memory requirements than the poroelasticity based modeling.  
77 In this paper, we develop a viscoelastic numerical modeling scheme to simulate seismic wave propagation in fluid-saturated  
78 porous media containing discrete distributed large-scale fractures. To capture the FPD effects between the fractures and  
79 background, we use the local effective medium theory based on Barbosa's VLSM to derive the effective anisotropic  
80 viscoelastic compliances in each numerical cell by superimposing the compliances of the background and the fractures. The  
81 effective anisotropic viscoelastic governing equations of the fractured porous rock are then numerically solved using mixed-  
82 grid stencil frequency-domain finite-difference method (FDFD) (Hustedt, et al. 2004; Operto, et al. 2009; Liu et al., 2018). To  
83 validate the proposed viscoelastic modeling scheme can capture the impact of FPD effects on seismic wave scattering, we  
84 compare the scattered waves of a single horizontal fracture obtained using our proposed modeling scheme with those obtained  
85 using poroelastic modeling scheme and elastic modeling scheme. Numerical examples of a fractured reservoir are presented  
86 to demonstrate that the proposed modeling scheme can properly simulate the wave attenuation and dispersion due to the FPD  
87 effects between the fracture system and background. A **complex modified Marmousi model** is also use to test the proposed  
88 modeling scheme and code. The scheme can be used not only to study the impact of mechanical and hydraulic of fracture  
89 properties on seismic scattering but can also to improve migration quality and the estimation of fracture mechanical  
90 characteristics in inversion.

## 91 **2 The elastic models**

92 The two most widely used non-attenuated and non-dissipative elastic models for fractured porous media are the low- and high-



93 frequency limits elastic LSM that ignore the FPD effects between the background and the fractures. The two elastic models  
 94 can be used to determine the effective anisotropic-elastic-moduli of the fractured porous rock.

## 95 **2.1 The low-frequency limits elastic linear slip models (LFLSM)**

96 The presence of fractures in a homogeneous and isotropic porous rock results in an effective anisotropic medium. The effective  
 97 compliance matrix of the dry fractured rock  $\mathbf{S}^{dry}$  can be obtained using the LSM (Schoenberg and Sayers, 1995):

$$98 \mathbf{S}^{dry} = \mathbf{S}_b^{dry} + \mathbf{Z}_0, \quad (1)$$

99 where  $\mathbf{S}_b^{dry}$  is the isotropic compliance matrix of the dry background medium in the absent of fractures, and  $\mathbf{Z}_0$  is the excess  
 100 compliance matrix due to the dry fractures. For a single set of rotationally invariant fractures,  $\mathbf{Z}_0$  can be written as  
 101 (Schoenberg and Sayers, 1995):

$$102 Z_{ij,0} = \frac{Z_T}{4} (\delta_{ik}n_in_j + \delta_{jk}n_in_i + \delta_{il}n_kn_j + \delta_{jl}n_kn_i) + (Z_{N_d} - Z_T)n_in_jn_kn_l, \quad (2)$$

103 where  $n_i$  is the component of the local unit normal to the fracture surface,  $Z_{N_d}$  and  $Z_T$  are the drained normal fracture  
 104 compliance and tangential fracture compliance, respectively, as functions of fracture thickness  $h^c$  and the drained  
 105 longitudinal modulus  $H_d^c$  and shear moduli  $\mu^c$  of the fracture (Brajanovski et al., 2005):

$$106 Z_{N_d} \equiv \frac{h^c}{H_d^c}, \quad Z_T \equiv \frac{h^c}{\mu^c}. \quad (3)$$

107 Since the fluid pressure is uniform in the low-frequency limit, the corresponding effective stiffness matrix  $\mathbf{C}_{ij}^{sat}$  of the fluid  
 108 saturated rock can be obtained using the anisotropic Gassmann equation (Gurevich, 2003):

$$109 C_{ij,lf}^{sat} = C_{ij}^{dry} + \alpha_i \alpha_j M_{dry}, \quad i, j = 1, \dots, 6. \quad (4)$$

110 The anisotropic Biot-Willis coefficients  $\alpha_m$  are:

$$111 \alpha_m = 1 - \frac{\sum_{n=1}^3 C_{mn}^{dry}}{3K_g}, \quad m = 1, 2, 3, \quad (5)$$

112  $\alpha_4 = \alpha_5 = \alpha_6 = 0$ . The Biot's fluid-storage modulus  $M$  is

$$113 M_{dry} = \frac{K_g}{(1 - K_0^*/K_g) - \phi(1 - K_g/K_f)}, \quad (6)$$

114 where  $K_g$  denotes the grain solid bulk modulus,  $K_f$  the pore fluid bulk modulus, and  $K_0^*$  the generalized drained bulk  
 115 modulus, defined as

$$116 K_0^* = \frac{1}{9} \sum_{i=1}^3 \sum_{j=1}^3 C_{ij}^{dry}. \quad (7)$$

## 117 **2.2 The high-frequency limits elastic linear slip models (HFLSM)**

118 In the high-frequency limit, the fractures are hydraulically isolated from the saturated background medium. The effective  
 119 compliance matrix of the saturated background medium permeated by the dry fractures can be expressed as (Guo et al., 2016):

$$120 \mathbf{S}_{hf}^1 = \mathbf{S}_b^{sat} + \mathbf{Z}_0, \quad (8)$$



121 where  $\mathbf{S}_b^{sat}$  is the isotropic compliance matrix of the saturated background medium in the absent of fractures. The effective  
 122 stiffness coefficients of the saturated fractured rock can be written as:

$$123 \quad C_{ij,hf}^{sat} = C_{ij,hf}^1 + \alpha_i^1 \alpha_j^1 M_1, \quad i, j = 1, \dots, 6, \quad (9)$$

124 where  $\alpha^1$  and  $M_1$  can be again calculated using Eqs. (5)-(7) but replacing the solid grains bulk modulus  $K_g$  with saturated  
 125 bulk modulus of the background  $K_m^{sat}$ , the overall porosity  $\phi$  with the fracture porosity  $\phi_c$ .

### 126 3 Nakagawa's poroelastic LSM (PLSM)

127 Nakagawa and Schoenberg (2007) presented a PLSM in the framework of poroelasticity, representing the fracture as a highly  
 128 compliant and porous thin layer embedded in a much stiffer and much less porous background (Barbosa et al., 2016a). Similar  
 129 to the classic LSM, the PLSM assumes that across a fracture surface the stress tensor is continuous while the displacement  
 130 tensor is discontinuous. The discontinuous displacement components for a horizontal fracture are (Nakagawa and Schoenberg,  
 131 2007):

$$132 \quad [u_x] = Z_T \tau_{xz}, \quad (10a)$$

$$133 \quad [u_y] = Z_T \tau_{yz}, \quad (10b)$$

$$134 \quad [u_z] = Z_{N_D} (\tau_{zz} + \alpha P_f), \quad (10c)$$

$$135 \quad [w_z] = -\alpha Z_{N_D} \left( \tau_{zz} + \frac{1}{B} P_f \right), \quad (10d)$$

136 where the parameter  $B = \alpha M / H_u$ , and the definition of drained normal fracture compliance  $Z_{N_D}$  and tangential fracture  
 137 compliance  $Z_T$  are the same as those in LFLEM. Since the PLSM represents both the background and the fracture as  
 138 poroelasticity, it is capable to describe the discontinuous displacement of the relative fluid in addition to the solid, implying  
 139 that it can properly handle the FPD effects between the background and the fracture. Although it is difficult to incorporate the  
 140 PLSM into the effective medium theory to obtain the effective moduli of the fractured porous rock, these boundary conditions  
 141 can be easily incorporated into poroelastic finite-difference algorithm for modeling seismic wave scattering off large-scale  
 142 fractures parallel to the coordinate axis. An alternative wavenumber domain method for modeling the scattered waves by  
 143 poroelastic fractures is presented by Nakagawa and Schoenberg (2007) based on the PLSM.

### 144 4 Barbosa's viscoelastic LSM (VLISM)

145 Barbosa et al. (2016a) derived a VLISM that account for the FPD effects between a fracture and background and the resulting  
 146 stiffening effect impact on the fracture. The background is assumed to be not impacted by the FPD and can be represented by  
 147 an elastic solid, whose properties are computed according to Gassmann's equation. By representing fractures as extremely thin  
 148 viscoelastic layers, the poroelastic effects were incorporated into the classical LSM through complex-valued and frequency-  
 149 dependent compliances. These compliances characterize the mechanical properties of the fluid-saturated fracture.



150 **4.1 The boundary conditions of VLSM**

151 The discontinuous displacement components of the VLSM (Barbosa et al., 2016a) for a horizontal fracture are

152  $[u_x] = Z_T \tau_{xz},$  (11a)

153  $[u_y] = Z_T \tau_{yz},$  (11b)

154  $[u_z] = Z_N \tau_{zz} + Z_X \varepsilon_{xx},$  (11c)

155 where  $Z_N$  and  $Z_T$  are generalized normal and tangential compliances respectively, and  $Z_X$  is related to the coupling  
 156 between horizontal and vertical deformation of the fracture. The normal compliance  $Z_N$  and additional parameter  $Z_X$  are  
 157 complex-valued and frequency-dependent, while the tangential compliance  $Z_T$  is the same as for elastic and poroelastic  
 158 models. The three effective fracture parameters are given by Barbosa et al. (2016a)

159 
$$\eta_N = \frac{\eta_{ND} [\alpha \eta_{NU} D_{P_2}^b - 2B \gamma_{P_2}^b i k_{P_2}^b - 2\alpha i k_{P_2}^b (1/\gamma_{P_2}^b + 2B)]}{\alpha \eta_{ND} D_{P_2}^b - 2B \gamma_{P_2}^b i k_{P_2}^b},$$
 (12a)

160 
$$\eta_X = \frac{-4k_{P_2}^b \alpha^b \eta_T M^b \mu^b (\alpha H_U^b M - \alpha^b H_U M^b)}{(H_U^b)^2 (h_{HU} \omega \eta_T^b + 2k_{P_2}^b M H_D \kappa^b)}.$$
 (12b)

161 We rewrite Eqs. (12a)-(12b) as

162 
$$Z_N = Z_{NU} + Z_{ND} \frac{G_1(1+i)}{\sqrt{\omega + G_2(1+i)}},$$
 (13a)

163 
$$Z_X = -\frac{G_3(1+i)}{\sqrt{\omega + G_4(1+i)}},$$
 (13b)

164 where  $Z_{NU}$  and  $Z_{ND}$  are the undrained and drained normal fracture compliance respectively,  $\omega$  is the angular frequency.

165 The four real-valued parameters  $G_1$ ,  $G_2$ ,  $G_3$  and  $G_4$  are defined as

166 
$$G_1 = \sqrt{\frac{\kappa^b (B^b - B^c)^2}{\eta_N^b \eta_{ND}}}, \quad G_2 \approx \sqrt{\frac{\kappa^b}{\eta_N^b \eta_{ND}}},$$
 (14a)

167 
$$G_3 = \frac{2\sqrt{2} \alpha^b \mu^b (B^b - B^c) \sqrt{D^b}}{H_D^b}, \quad G_4 = \frac{\sqrt{2} \kappa^b D^c}{h^c \kappa^c \sqrt{D^b}}$$
 (14b)

168 where the parameters with superscripts  $b$  correspond to background properties and the parameters with superscripts  $c$   
 169 correspond to fracture parameters. In Eqs. (14a)-(14b),  $D$  is the diffusivity defined as  $D = \kappa N / \eta$  ( $N = H_D M / H_U$ ), and the  
 170 dimensionless parameter  $B$  defined as  $B = \alpha M / H_U$ .  $H_U$ ,  $H_D$  and  $\mu$  are the corresponding undrained  $P$  wave modulus,  
 171 drained  $P$  wave modulus and shear modulus. The Barbosa's VLSM can properly capture the FPD effects between a fracture  
 172 and background.

173 **4.2 The effective viscoelastic-anisotropic stiffness matrix based on Barbosa's VLSM**

174 To incorporate the VLSM into viscoelastic finite-difference modeling algorithms, we give the specific derivation of the  
 175 effective viscoelastic-anisotropic stiffness matrix of the numerical grids on a fracture based on Coates and Schoenberg's local  
 176 effective medium theory (1995). The porous background is assumed to be unaffected by the FPD in the presence of fractures  
 177 because of the small amount of diffusing fluid and large compliance contrast between background and fluid. Thus, the rock  
 178 background can be represented by an elastic homogeneous solid and the strain  $\epsilon^b$  of the background can be expressed as



179  $\varepsilon_{ij}^b = s_{ijkl}^b \sigma_{kl},$  (15)

180 where the compliance tensor  $\mathbf{s}^b$  are computed according to Gassmann's equation (Rubino et al., 2015; Barbosa et al., 2016a),  
 181 and  $\boldsymbol{\sigma}$  is the average stress tensor. The exceed strain tensor  $\boldsymbol{\varepsilon}^c$  induced by a single fracture with surface  $S$  in a representative  
 182 volume  $V$  (e.g. the volume of numerical cell) is given by (Hudson and Knopoff, 1989; Sayers and Kachanov, 1995; Liu, et  
 183 al., 2000)

184  $\varepsilon_{ij}^c = s_{ijkl}^c \sigma_{kl} = \frac{1}{2V} \int ([u_i]n_j + [u_j]n_i) dS,$  (16)

185 where  $\mathbf{s}^c$  is the extra compliance tensor resulting from the fractures,  $[u_i]$  is the  $i$ th component of the displacement  
 186 discontinuity on  $S$  and  $n_i$  is the  $i$ th component of the fracture normal. Note that Eq. (16) is applicable to finite, nonplanar  
 187 fractures in the long wavelength limit, i.e., the applied stress is assumed to be constant over the representative volume.

188 If we assume that the interface of the fracture is normal to the  $z$ -axis (fracture normal vector  $\mathbf{n}$  is (0,0,1)), substituting Eqs.  
 189 (11a)-(11c) into Eq. (16), we can obtain the nonzero element of the exceed fracture strain tensor

190  $\varepsilon_{xz}^c = \frac{S}{V} Z_T \tau_{xz},$  (17a)

191  $\varepsilon_{yz}^c = \frac{S}{V} Z_T \tau_{yz},$  (17b)

192  $\varepsilon_{zz}^c = \frac{S}{V} (Z_N \tau_{zz} + Z_X \varepsilon_{xx}^b),$  (17c)

193 Then the exceed fracture strain tensor  $\varepsilon_{ij}^c$  and the background strain tensor  $\varepsilon_{ij}^b$  can be written in matrix form in Voigt notation

194  $\mathbf{e}^b = \mathbf{S}^b \boldsymbol{\sigma},$  (18)

195  $\mathbf{e}^c = \frac{S}{V} (\mathbf{Z}_1 \boldsymbol{\sigma} + \mathbf{Z}_2 \mathbf{e}^b) = \frac{S}{V} (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \boldsymbol{\sigma},$  (19)

196 where the strain matrix  $\mathbf{e} = [\varepsilon_{11}, \varepsilon_{22}, \varepsilon_{33}, 2\varepsilon_{23}, 2\varepsilon_{13}, 2\varepsilon_{12}]^T$ , and the stress matrix  $\boldsymbol{\sigma} = [\sigma_{11}, \sigma_{22}, \sigma_{33}, \sigma_{23}, \sigma_{13}, \sigma_{12}]^T$ . The  
 197  $6 \times 6$  fracture compliance matrix  $\mathbf{Z}_1$  and additional dimensionless matrix  $\mathbf{Z}_2$  according to the Voigt notation are defined as

198  $\mathbf{Z}_1 = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & Z_N & 0 & 0 & 0 \\ 0 & 0 & 0 & Z_T & 0 & 0 \\ 0 & 0 & 0 & 0 & Z_T & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}, \mathbf{Z}_2 = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ Z_X & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}.$  (20)

199 The average strain  $\mathbf{e}$  in a homogeneous porous rock containing single fracture can be expressed as the sum of the strains of  
 200 background and the fractures

201  $\mathbf{e} = \mathbf{e}^b + \mathbf{e}^c.$  (21)

202 Substituting Eq. (15) and Eq. (19) into Eq. (21), we can obtain the average strain matrix

203  $\mathbf{e} = \left[ \mathbf{S}^b + \frac{S}{V} (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \right] \boldsymbol{\sigma}.$  (22)

204 Thus, the effective stiffness matrix  $\mathbf{C}$  can be expressed as

205  $\mathbf{C} = \left[ \mathbf{S}^b + \frac{S}{V} (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \right]^{-1} = \mathbf{C}^b \left[ \mathbf{I} + \frac{S}{V} (\mathbf{Z}_1 \mathbf{C}^b + \mathbf{Z}_2) \right]^{-1}.$  (23)



206 The effective stiffness matrix of case of an inclined fracture can be obtained by rotating the coordinate axis to keep  $z$ -axis  
 207 perpendicular to fracture interface. We first define the inclined fracture have an angle  $\varphi$  and an azimuth angle  $\theta$ , and then  
 208 the rotation matrix can be obtained:

$$209 \mathbf{R} = \begin{bmatrix} \cos\theta\cos\varphi & -\sin\theta & \cos\theta\sin\varphi \\ \sin\theta\cos\varphi & \cos\theta & \sin\theta\sin\varphi \\ -\sin\varphi & 0 & \cos\varphi \end{bmatrix}, \quad (24)$$

210 as well as the corresponding stress Bond matrix  $\mathbf{A}_\sigma$  and strain Bond matrix  $\mathbf{A}_\varepsilon$ . The new stress matrix  $\mathbf{e}'$  and strain matrix  
 211  $\boldsymbol{\sigma}'$  can be expressed as the multiplication of the old one and Bond matrix

$$212 \mathbf{e}' = \mathbf{A}_\varepsilon \mathbf{e}, \quad \boldsymbol{\sigma}' = \mathbf{A}_\sigma \boldsymbol{\sigma}. \quad (25)$$

213 By substituting Eq. (25) into Eq. (19), the new exceed fracture strain matrix can be obtained

$$214 \mathbf{e}^c = \frac{S}{V} \mathbf{A}_\varepsilon (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \mathbf{A}_\varepsilon^T \boldsymbol{\sigma}. \quad (26)$$

215 Finally, substituting Eq. (6) into Eq. (21), the average strain matrix of each numerical cell containing discrete distributed  
 216 fractures with the same arbitrary direction can be expressed as

$$217 \mathbf{e} = \left[ \mathbf{S}^b + \frac{S}{V} \mathbf{A}_\varepsilon (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \mathbf{A}_\varepsilon^T \right] \boldsymbol{\sigma}, \quad (27)$$

218 and the corresponding effective stiffness matrix  $\mathbf{C}$  is

$$219 \mathbf{C} = \left[ \mathbf{S}^b + \frac{S}{V} \mathbf{A}_\varepsilon (\mathbf{Z}_1 + \mathbf{Z}_2 \mathbf{S}^b) \mathbf{A}_\varepsilon^T \right]^{-1}, \quad (28)$$

220 If the background media is isotropic, the  $\mathbf{C}$  can be simplified as

$$221 \mathbf{C} = \mathbf{C}^b \left[ \mathbf{I} + \frac{S}{V} \mathbf{A}_\varepsilon (\mathbf{Z}_1 \mathbf{C}^b + \mathbf{Z}_2) \mathbf{A}_\varepsilon^T \right]^{-1}, \quad (29)$$

222 If we ignore the interaction between different fractures and the FPD along the fracture interfaces, the result can be easily  
 223 extended to the case of multiple sets of discrete distributed large-scale fractures with arbitrary orientation:

$$224 \mathbf{C} = \mathbf{C}^b \left[ \mathbf{I} + \sum_{r=1}^{N_c} \frac{S_r}{V} \mathbf{A}_{\varepsilon r} (\mathbf{Z}_{1r} \mathbf{C}^b + \mathbf{Z}_{2r}) \mathbf{A}_{\varepsilon r}^T \right]^{-1}, \quad (30)$$

225 where  $N_c$  is total number of the fracture directions and the subscript  $r$  denotes the  $r$ th direction. The derived effective  
 226 stiffness matrix is to be employed in the viscoelastic finite-difference modeling of discrete distributed large-scale fractures in  
 227 porous rock.

## 228 5. Seismic modeling of fractured porous rock

229 In this section, we focus on the implementation of seismic modeling of fluid-saturated porous media containing discrete  
 230 distributed large-scale fractures in 2D case. We develop a viscoelastic modeling scheme based on the VLSM and local effective  
 231 medium theory (Coates and Schoenberg, 1995) to incorporate the FPD effects between fractures and background. To validate  
 232 that the proposed viscoelastic modeling scheme can capture the impact of FPD effects on seismic wave scattering of fractures,  
 233 we outline the implementation of poroelastic modeling scheme using an explicit application of the PLSM.





234 **5.1 viscoelastic modeling based on VLSM**

235 For viscoelastic modeling, we adopt local effective media theory based on VLSM to derive the effective anisotropic  
 236 viscoelastic compliances in each numerical cell by superimposing the compliances of the background and the fractures. Since  
 237 the real structure of the rock is substituted by ideally continua, the balance equations of classical continuum mechanics can be  
 238 applied without considering the discontinuity at the fracture interfaces (Lewis and Schrefler, 1998; Gavagnin et al., 2020), and  
 239 the constitutive equations are characterized by effective complex-valued and frequency-dependent TTI viscoelastic stiffness.  
 240 Thus, the second-order heterogeneous governing equations of fractured porous rock with **PML** in frequency domain can be  
 241 expressed as:

242 
$$\omega^2 \rho u_x + \frac{1}{\xi_x} \partial_x \left( \frac{c_{11}}{\xi_x} \partial_x u_x + \frac{c_{13}}{\xi_z} \partial_z u_z + \frac{c_{15}}{\xi_z} \partial_z u_x + \frac{c_{15}}{\xi_x} \partial_x u_z \right) + \frac{1}{\xi_z} \partial_z \left( \frac{c_{15}}{\xi_x} \partial_x u_x + \frac{c_{35}}{\xi_z} \partial_z u_z + \frac{c_{55}}{\xi_z} \partial_z u_x + \frac{c_{55}}{\xi_x} \partial_x u_z \right) = 0, \quad (31a)$$

243 
$$\omega^2 \rho u_z + \frac{1}{\xi_x} \partial_x \left( \frac{c_{15}}{\xi_x} \partial_x u_x + \frac{c_{35}}{\xi_z} \partial_z u_z + \frac{c_{55}}{\xi_z} \partial_z u_x + \frac{c_{55}}{\xi_x} \partial_x u_z \right) + \frac{1}{\xi_z} \partial_z \left( \frac{c_{15}}{\xi_x} \partial_x u_x + \frac{c_{33}}{\xi_z} \partial_z u_z + \frac{c_{35}}{\xi_z} \partial_z u_x + \frac{c_{35}}{\xi_x} \partial_x u_z \right) = 0, \quad (31b)$$

244 where  $u_x$  and  $u_z$  are the horizontal and vertical components of particle displacement vector,  $\rho$  is the effective density, and  
 245  $c_{ij}$  are the components of complex-valued and frequency-dependent effective stiffness matrix,  $\xi_x$  and  $\xi_z$  are the frequency  
 246 domain PML damping functions.

247 In time domain, the governing equations are integral differential equations, which require special processing for the  
 248 convolution operations, resulting in high computational costs. Although the problem can be relieved (mitigated) by memory  
 249 functions, it still requires high memory requirements. Instead, the governing equations can be straightforwardly solved using  
 250 **DFD**. To efficiently and accurately modelling of seismic wave propagation in fluid saturated fractured porous rock, we solve  
 251 the second-order heterogeneous governing equations with mixed-grid stencil FDFD method (Jo et al., 1996; Hustedt et al.  
 252 2004). The mixed system of governing equations is formulated by combining the classical Cartesian coordinate system (CS)  
 253 and the 45°-rotated coordinate system (RS):

254 
$$\omega^2 \rho u_x + w_1(A_c u_x + B_c u_z) + (1 - w_1)(A_r u_x + B_r u_z) = 0, \quad (32a)$$

255 
$$\omega^2 \rho u_z + w_1(C_c u_x + D_c u_z) + (1 - w_1)(C_r u_x + D_r u_z) = 0, \quad (32b)$$

256 where the optimal averaging coefficient  $w_1 = 0.5461$  (Jo et al., 1996). The coefficients  $A_c, B_c, C_c, D_c$  and  $A_r, B_r, C_r, D_r$   
 257 are functions of the damping functions, effective stiffness coefficients and spatial derivative operators and the detailed  
 258 expressions are given in Appendix A. We follow Hustedt et al., (2004) and Liu et al., (2018) to discretize the derivative  
 259 operation on the mixed systems using mixed grid stencil. After discretization and arrangement, the mixed system of governing  
 260 equations can be written in matrix form as

261 
$$\begin{bmatrix} \mathbf{M} + w_1 \mathbf{A}_c + (1 - w_1) \mathbf{A}_r & w_1 \mathbf{B}_c + (1 - w_1) \mathbf{B}_r \\ w_1 \mathbf{C}_c + (1 - w_1) \mathbf{C}_r & \mathbf{M} + w_1 \mathbf{D}_c + (1 - w_1) \mathbf{D}_r \end{bmatrix} \begin{bmatrix} \mathbf{u}_x \\ \mathbf{u}_z \end{bmatrix} = \begin{bmatrix} \mathbf{0} \\ \mathbf{0} \end{bmatrix}, \quad (36)$$

262 where  $\mathbf{M}$  denotes the diagonal mass matrix of coefficients  $\omega^2 \rho$ , and blocks  $\mathbf{A}_c, \mathbf{B}_c, \mathbf{C}_c, \mathbf{D}_c$  and  $\mathbf{A}_r, \mathbf{B}_r, \mathbf{C}_r, \mathbf{D}_r$  form the  
 263 stiffness matrices for the CS and RS stencils, respectively, and the corresponding coefficients of submatrices are given in



264 Appendix B.

265 To improve the modelling accuracy of mixed-grid stencil, the acceleration term  $\omega^2\rho$  are approximated using a weighted  
266 average over the mixed operator stencil nodes

$$267 \quad [\omega^2\rho]_{ij} \approx \omega^2 \left[ w_{m1}\rho_{ij} + w_{m2}(\rho_{i+1,j} + \rho_{i-1,j} + \rho_{ij+1} + \rho_{ij-1}) + \frac{(1-w_{m1}-4w_{m2})}{4}(\rho_{i+1,j+1} + \rho_{i-1,j-1} + \rho_{i-1,j+1} + \rho_{i+1,j-1}) \right], (37)$$

268 where the optimal coefficients  $w_{m1} = 0.6248$  and  $w_{m2} = 0.09381$  are computed by Jo et al. (1996).

269 In order to assess the FPD effects on seismic response, the similar procedure was adopted in the implementation of elastic  
270 modeling by replacing the VLSM with the LFLSM (assuming fluid pressure is equilibrium) or the HFLSM (assuming fluid  
271 pressure is unequilibrium).

## 272 5.2 Poroelastic modeling based on PLSM

273 The poroelastic modeling means that we numerically solve the Biot's equations and adopt an explicit implementation of the  
274 PLSM across each fracture instead of using the **effective media theory**. Hence, the poroelastic modeling can naturally deal with  
275 the FPD between fracture and background and account for its impact on wave scattering. Although it is difficult to implement  
276 an explicit application of PLSM for arbitrary orientated fracture, it is relatively straightforward for horizontal or vertical fracture.  
277 In frequency domain, the governing equations for an isotropic poroelastic media in the absent of fractures can be written as  
278 (Biot, 1962):

$$279 \quad \omega^2\rho\mathbf{u} + \omega^2\rho_f\mathbf{w} + \nabla \cdot \boldsymbol{\sigma} = 0, \quad (38a)$$

$$280 \quad \omega^2\rho_f\mathbf{u} + i\omega\frac{\eta}{\kappa}\mathbf{w} - \nabla P_f = 0, \quad (38b)$$

$$281 \quad \boldsymbol{\sigma} = [(H_U - 2\mu)\nabla \cdot \mathbf{u} + \alpha M\nabla \cdot \mathbf{w}]\mathbf{I} + \mu(\nabla\mathbf{u} + \nabla\mathbf{u}^T), \quad (38c)$$

$$282 \quad -P_f = \alpha M\nabla \cdot \mathbf{u} + M\nabla \cdot \mathbf{w}. \quad (38d)$$

283 By discretizing Eqs. (38a)-(38d) using second-order differences, we can obtain:

$$284 \quad \omega^2\rho u_{xij} + \omega^2\rho_f w_{xij} + \frac{\sigma_{xx\ i+1j} - \sigma_{xx\ ij}}{\Delta} + \frac{\sigma_{xz\ ij+1} - \sigma_{xz\ ij}}{\Delta} = 0, \quad (39a)$$

$$285 \quad \omega^2\rho u_{xij} + \omega^2\rho_f w_{xij} + \frac{\sigma_{xx\ i+1j} - \sigma_{xx\ ij}}{\Delta} + \frac{\sigma_{xz\ ij+1} - \sigma_{xz\ ij}}{\Delta} = 0, \quad (39b)$$

$$286 \quad \omega^2\rho_f u_{xij} + i\omega\frac{\eta}{\kappa}w_{xij} - \frac{P_{f\ i+1j} - P_{f\ ij}}{\Delta} = 0, \quad (39c)$$

$$287 \quad \omega^2\rho_f u_{zij} + i\omega\frac{\eta}{\kappa}w_{zij} - \frac{P_{f\ ij+1} - P_{f\ ij}}{\Delta} = 0, \quad (39d)$$

$$288 \quad \sigma_{xx\ ij} = H_U \frac{u_{x\ i+1j} - u_{x\ ij}}{\Delta} + (H_U - 2\mu) \frac{u_{z\ ij+1} - u_{z\ ij}}{\Delta} + \alpha M \left( \frac{w_{x\ i+1j} - w_{x\ ij}}{\Delta} + \frac{w_{z\ ij+1} - w_{z\ ij}}{\Delta} \right), \quad (39e)$$

$$289 \quad \sigma_{zz\ ij} = (H_U - 2\mu) \frac{u_{x\ i+1j} - u_{x\ ij}}{\Delta} + H_U \frac{u_{z\ ij+1} - u_{z\ ij}}{\Delta} + \alpha M \left( \frac{w_{x\ i+1j} - w_{x\ ij}}{\Delta} + \frac{w_{z\ ij+1} - w_{z\ ij}}{\Delta} \right), \quad (39f)$$

$$290 \quad \sigma_{xz\ ij} = \mu \left( \frac{u_{x\ ij+1} - u_{x\ ij}}{\Delta} + \frac{u_{z\ i+1j} - u_{z\ ij}}{\Delta} \right), \quad (39g)$$

$$291 \quad -P_f = \alpha M \frac{u_{x\ i+1j} - u_{x\ ij}}{\Delta} + \alpha M \frac{u_{z\ ij+1} - u_{z\ ij}}{\Delta} + M \left( \frac{w_{x\ i+1j} - w_{x\ ij}}{\Delta} + \frac{w_{z\ ij+1} - w_{z\ ij}}{\Delta} \right). \quad (39h)$$



292 In the presence of horizontal fracture passing through the numerical cell  $(i, j_0)$ , the PLSM can be written as:

$$293 \quad u_{x \ i_{j_0+1}} - u_{x \ i_{j_0}} = (Z_T \sigma_{xz})_{i_{j_0}}, \quad (40a)$$

$$294 \quad u_{z \ i_{j_0+1}} - u_{z \ i_{j_0}} = (Z_{ND} \sigma_{zz} + Z_{ND} \alpha P_f)_{i_{j_0}}, \quad (40b)$$

$$295 \quad w_{z \ i_{j_0+1}} - w_{z \ i_{j_0}} = - \left( \alpha Z_{ND} \sigma_{zz} + \frac{\alpha Z_{ND}}{B} P_f \right)_{i_{j_0}}. \quad (40c)$$

296 Rearrange the Eqs. (39e)-(39h), i.e. use the displacement components to represent the stress components, and superimpose the  
297 discrete Eqs. (40a)-(40c), we get the following discrete equations:

$$298 \quad \frac{u_{x \ i_{j_0+1}} - u_{x \ i_{j_0}}}{\Delta} = \left[ \frac{H_D}{4\mu(H_D - \mu)} \sigma_{xx} + \frac{(2\mu - H_D)}{4\mu(H_D - \mu)} \sigma_{zz} + \frac{2\alpha\mu}{4\mu(H_D - \mu)} P_f \right]_{i_{j_0}}, \quad (41a)$$

$$299 \quad \frac{u_{z \ i_{j_0+1}} - u_{z \ i_{j_0}}}{\Delta} = \left[ \frac{(2\mu - H_D)}{4\mu(H_D - \mu)} \sigma_{xx} + \left[ \frac{H_D}{4\mu(H_D - \mu)} + \frac{Z_{ND}}{\Delta} \right] \sigma_{zz} + \left[ \frac{2\alpha\mu}{4\mu(H_D - \mu)} + \frac{\alpha Z_{ND}}{\Delta} \right] P_f \right]_{i_{j_0}}, \quad (41b)$$

$$300 \quad \frac{u_{x \ i_{j_0+1}} - u_{x \ i_{j_0}}}{\Delta} + \frac{u_{z \ i_{j_0+1}} - u_{z \ i_{j_0}}}{\Delta} = \left[ \left( \frac{1}{\mu} + \frac{Z_T}{\Delta} \right) \sigma_{xz} \right]_{i_{j_0}}, \quad (41c)$$

$$301 \quad \frac{w_{x \ i_{j_0+1}} - w_{x \ i_{j_0}}}{\Delta} + \frac{w_{z \ i_{j_0+1}} - w_{z \ i_{j_0}}}{\Delta} = \left[ \frac{2\alpha\mu}{4\mu(H_D - \mu)} \sigma_{xx} + \left( \frac{2\alpha\mu}{4\mu(H_D - \mu)} - \frac{\alpha Z_{ND}}{\Delta} \right) \sigma_{zz} - \frac{1}{M} \left( \frac{H_U - \mu}{H_D - \mu} + \frac{H_U Z_{ND}}{\Delta} \right) P_f \right]_{i_{j_0}}. \quad (41d)$$

302 For a numerical cell, if  $j \neq j_0$ , we set  $Z_{ND} = Z_T = 0$ . By re-injecting Eqs. (41a)-(41d) into the discretized Eqs. (39a)-(39c),  
303 we eliminate the stress terms and obtain the compact discretized system of wave equations that contain only the displacement  
304 field:

$$305 \quad \begin{bmatrix} \mathbf{G}_{11} & \mathbf{G}_{12} & \mathbf{G}_{13} & \mathbf{G}_{14} \\ \mathbf{G}_{21} & \mathbf{G}_{22} & \mathbf{G}_{23} & \mathbf{G}_{24} \\ \mathbf{G}_{31} & \mathbf{G}_{32} & \mathbf{G}_{33} & \mathbf{G}_{34} \\ \mathbf{G}_{41} & \mathbf{G}_{42} & \mathbf{G}_{43} & \mathbf{G}_{44} \end{bmatrix} \begin{bmatrix} \mathbf{u}_x \\ \mathbf{u}_z \\ \mathbf{w}_x \\ \mathbf{w}_z \end{bmatrix} = \begin{bmatrix} \mathbf{0} \\ \mathbf{0} \\ \mathbf{0} \\ \mathbf{0} \end{bmatrix}, \quad (42)$$

306 where blocks  $\mathbf{G}_{ij}$  ( $i, j = 1 \dots 4$ ) form the stiffness matrices of the discretized system of the poroelastic wave equations. The  
307 poroelastic modeling based on PLSM will be used to validate the other modeling schemes.

## 308 6. Numerical examples

**Table1** Physical Properties of the Materials Employed in the Numerical Modeling

Parameters	Background	Fracture	Underlying
Porosity, $\phi$	0.15	0.8	0.05
Permeability, $\kappa$	0.1 D	100 D	0.01 D
Solid bulk modulus, $K_s$	36 GPa	36 GPa	36 GPa
Frame bulk modulus, $K_m$	20.3 GPa	0.055 GPa	30.6 GPa
Frame shear modulus, $\mu_m$	18.6 GPa	0.033 GPa	32.2 GPa
Solid density, $\rho_s$	2700 kg/m <sup>3</sup>	2700 kg/m <sup>3</sup>	2700 kg/m <sup>3</sup>
Fluid density, $\rho_f$	1000 kg/m <sup>3</sup>	1000 kg/m <sup>3</sup>	1000 kg/m <sup>3</sup>
Fluid shear viscosity, $\eta_f$	0.01 Poise	0.01 Poise	0.01 Poise
Fluid bulk modulus, $K_f$	2.25 GPa	2.25 GPa	2.25 GPa
Thickness, $h$		1 mm	

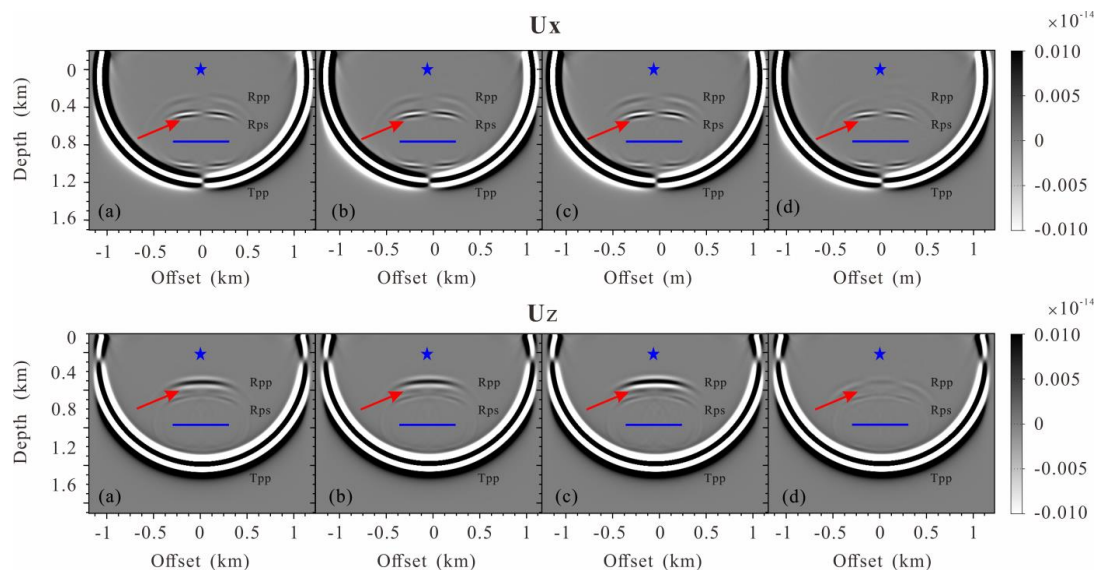
309 In this section, we apply different numerical modeling schemes on three fractured models to examine the FPD effects on  
310 seismic wave scattering. We mainly focus on the amplitudes and phases of the scattered and reflected waves generated by



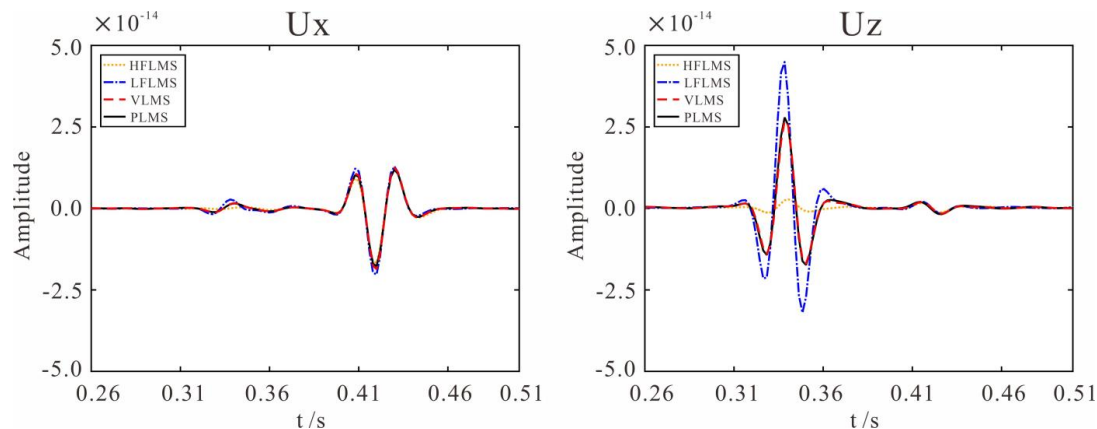
311 pressure source and shearing source.

312 **6.1 Single horizontal fracture model**

313 Here, we numerically simulate the scattering of seismic waves from a single horizontal fracture embedded in a homogeneous  
 314 background. The model measures  $2000\text{m} \times 1500\text{m}$  with a grid interval 5m (namely, the numerical grids size is  $401 \times 301$ )  
 315 surrounded by a 200m thick PML boundary. The fracture is located 750m directly below the source (1000m, 30m), with a  
 316 500m horizontal extending. A Ricker wavelet with a central frequency of 35Hz is used as the temporal source excitation. The  
 317 material properties of the fracture and background are given in Table 1 modified from Nakagawa and Schoenberg (2007) and  
 318 Barbosa et al. (2016a). For comparison, we present the seismic wavefields obtained using the poroelastic modeling based on  
 319 PLSM, the viscoelastic modeling based on VLSM, as well as the elastic modeling based on LFLSM and HFLSM. To further  
 320 study the impact of FPD effects on P- and S-wave, we also apply the pressure source and shearing source in all four schemes,  
 321 respectively.



322 **Figure 1: Snapshots of the wavefields components  $U_x$  and  $U_z$  for a single horizontal fracture model at 280ms due to a P-wave point**  
 323 **source: (a) the PLSM based poroelastic modeling, (b) the VLSM based viscoelastic modeling, (c) the LFLSM based elastic modeling**  
 324 **and (d) the HFLSM based elastic modeling.**  
 325

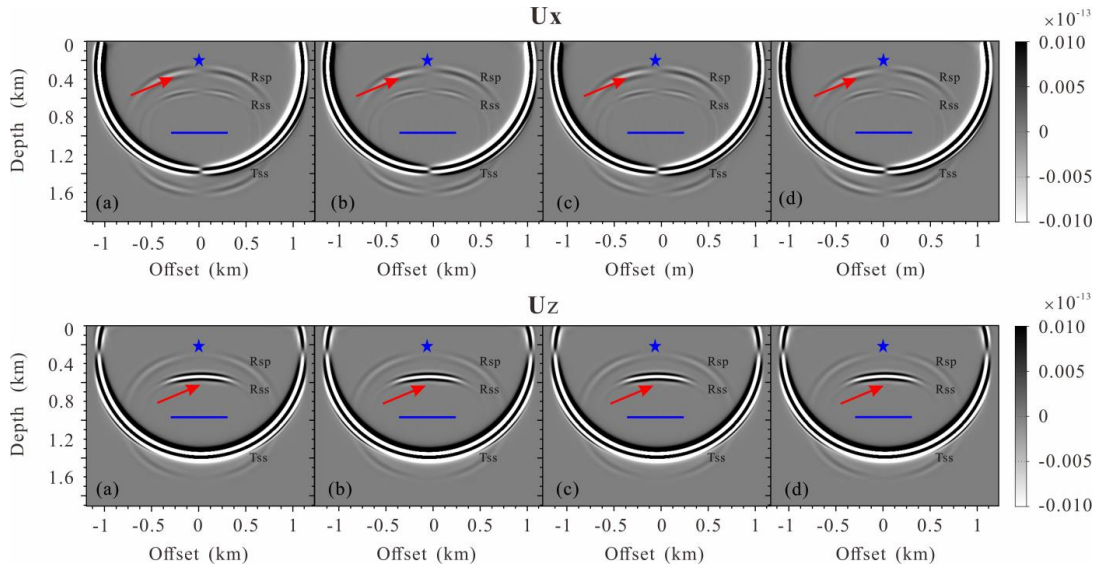


326

327 **Figure 2: Comparison of 1-D seismograms components  $U_x$  and  $U_z$  at (1200m, 0m) for a single horizontal fracture model due to a P-**  
 328 **wave point source.**

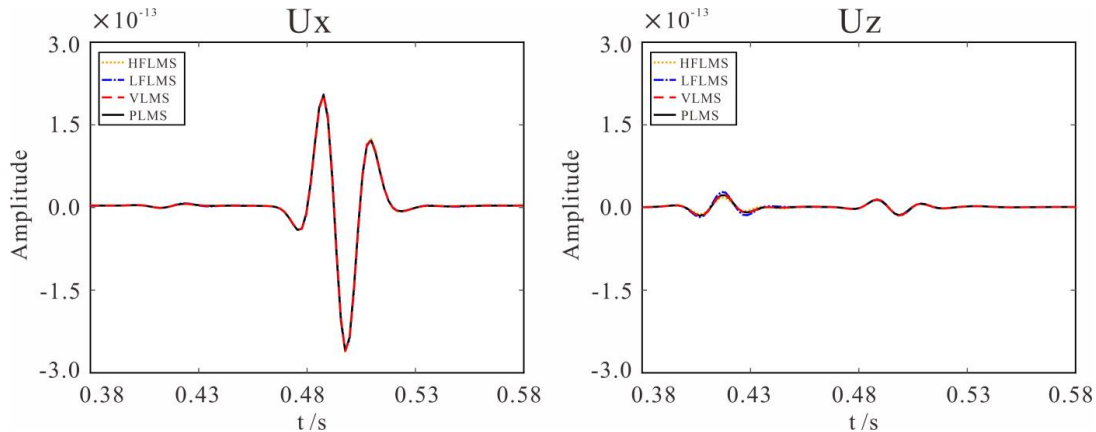
329 Figure 1 shows the 280ms snapshots of the displacement fields for the single horizontal fracture model models with P-wave  
 330 point source. The displacement fields are calculated by the PLSM-based poroelastic modeling, the VLMSM-based viscoelastic  
 331 modeling, the LFLSM-based elastic modeling and the HFLSM-based elastic modeling, respectively. The asterisk represents  
 332 the source and the blue line represents the fracture. To make the small scattered wave visible, large amplitude is clipped, thus  
 333 the transmitted compressional wave ( $T_{pp}$ ), scattered compressional wave ( $S_{pp}$ ) and scattered converted wave ( $S_{ps}$ ) can be seen  
 334 clearly. Figure 2 present the comparison of 1-D seismograms at (1200m, 0m).

335 We consider the poroelastic modeling as a reference scenario because it can naturally incorporate the FPD effects. Figure 1  
 336 and Figure 2 suggest very good agreement between the  $S_{pp}$  amplitude calculated using the PLSM-based and VLMSM-based  
 337 modeling, while the HFLSM-based modeling obviously underestimate the  $S_{pp}$  amplitude, and the LFLSM-based modeling  
 338 overestimate the  $S_{pp}$  amplitude. This is to be expected, since the scattering behavior of a fracture is mainly controlled by the  
 339 stiffness contrast with respect to the background. The HFLSM assumes there is insufficient time for fluid exchange at the  
 340 fracture interface, the fracture behaves as being sealed and the stiffness of the saturated fracture is maximal, resulting in an  
 341 underestimated stiffness contrast between fracture and background. The LFLSM assumes there is enough time for fluid flow  
 342 between the fracture and background, the deformation of the fracture is maximal, resulting in an overestimated stiffness  
 343 contrast with background. However, the VLMSM derived from poroelastic theory can properly incorporate the FPD effects,  
 344 leading to a frequency-dependent stiffness contrast equivalent to the PLSM. It can be note that the  $S_{pp}$  amplitudes obtained  
 345 using the LFLSM-based modeling is comparable to that of the PLSM based modeling, because the FPD effects mainly occur  
 346 at seismic frequencies closer to the low frequency limit. The  $S_{pp}$  travel time obtained using the four modeling schemes shows  
 347 good consistency. Figure 2 also shows that the discrepancy of the  $S_{ps}$  amplitudes is almost negligible. Figure 1 and Figure 2  
 348 demonstrate that the DLSM-based viscoelastic modeling can appropriately capture the FPD effects on wave scattering of a  
 349 fluid saturated fracture. However, the two elastic modeling cannot correctly estimate the  $S_{pp}$  amplitudes.



350

351 **Figure 3: Snapshots of the wavefields components  $U_x$  and  $U_z$  for a single horizontal fracture model at 280ms due to a S-wave point**  
 352 **source: (a) the PLSM based poroelastic modeling, (b) the VLSM based viscoelastic modeling, (c) the LFLSM based elastic modeling**  
 353 **and (d) the HFLSM based elastic modeling.**



354

355 **Figure 4: Comparison of 1-D seismograms components  $U_x$  and  $U_z$  at receiver (1200m, 0m) for a single horizontal fracture model**  
 356 **due to a S-wave point source.**

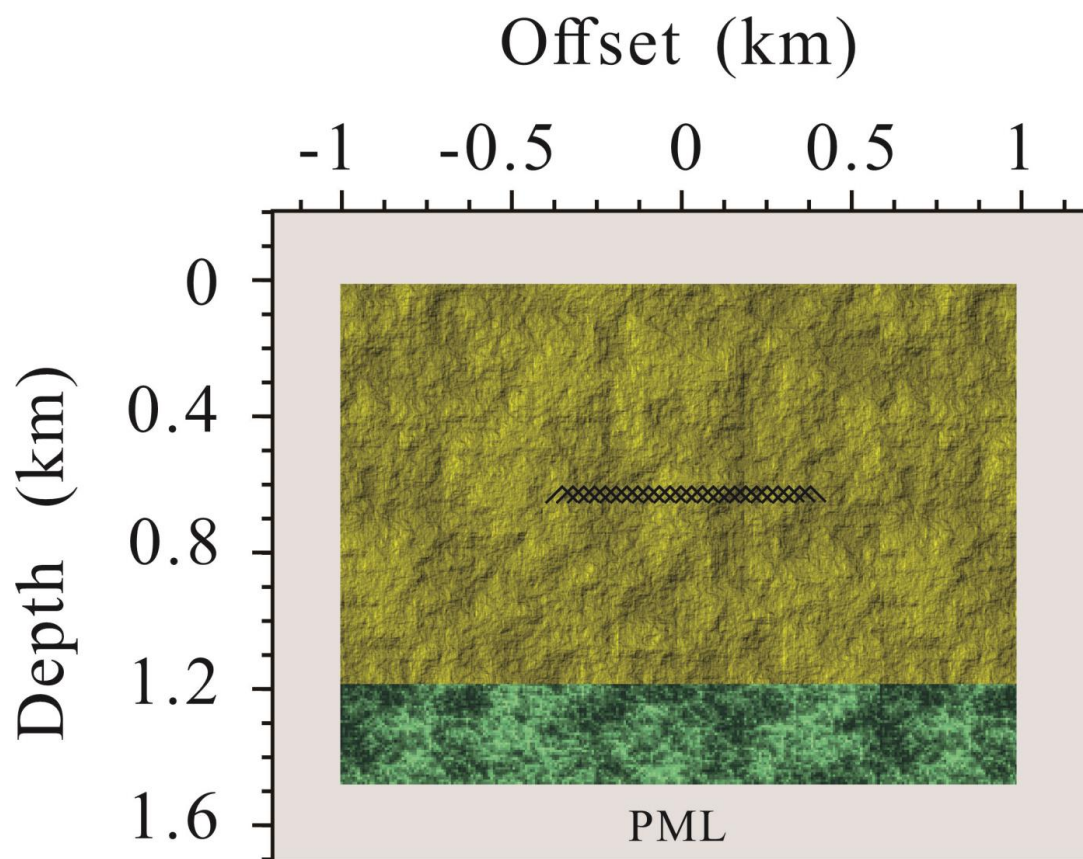
357 Figure 3 shows the 360ms snapshots of the displacement fields for the single horizontal fracture model models with S-wave  
 358 point source. Figure 4 is the comparison of 1-D seismograms at (1200m,0m). Figure 3 and Figure 4 show that the amplitudes  
 359 of the calculated  $S_{SP}$  and  $S_{SS}$  using four modeling schemes have good consistency, indicating that S-wave point source  
 360 exploration survey is less sensitive to fluids or FDP effects for a single fracture. The scattering behavior is mainly controlled  
 361 by the drained stiffness contrast between the fracture and the background.

## 362 6.2 Fractured reservoir model

363 In addition to a single fracture, we are more interested in the scattering behavior of discretely distributed fractures system. To



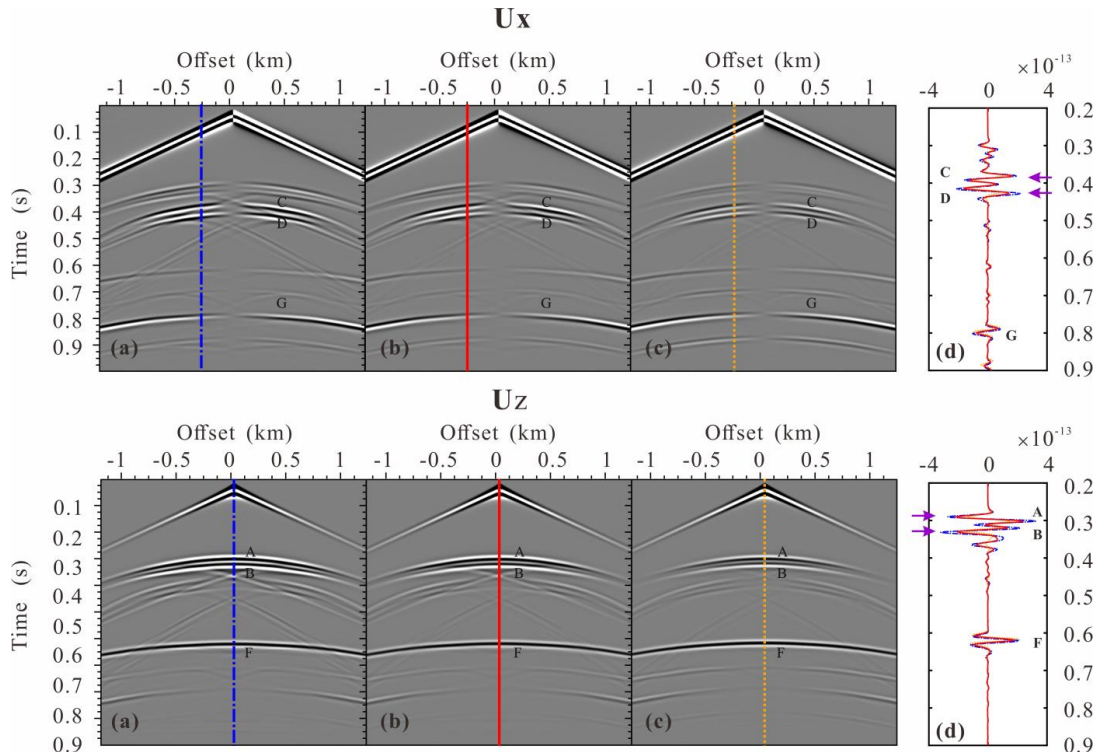
364 this end, we designed a fractured reservoir model containing a conjugate fracture system (consisting of two sets of mutually  
365 perpendicular fractures, as illustrated in Figure 5). The normal spacing and extending of this set of conjugate fractures are  
366 1.768m and 70.7m, respectively. The material properties of the fracture, background (yellow region) and underlying (green  
367 region) formation are given in Table 1. The model size, grid interval and source location are the same as those in the previous  
368 numerical examples.



369

370 **Figure 5: Schematic diagram of the fractured reservoir model I with a conjugate fracture system. The black segments present the**

371 **fracture system. The normal spacing and extending of each fracture are 1.768m and 70.7m, respectively.**



372

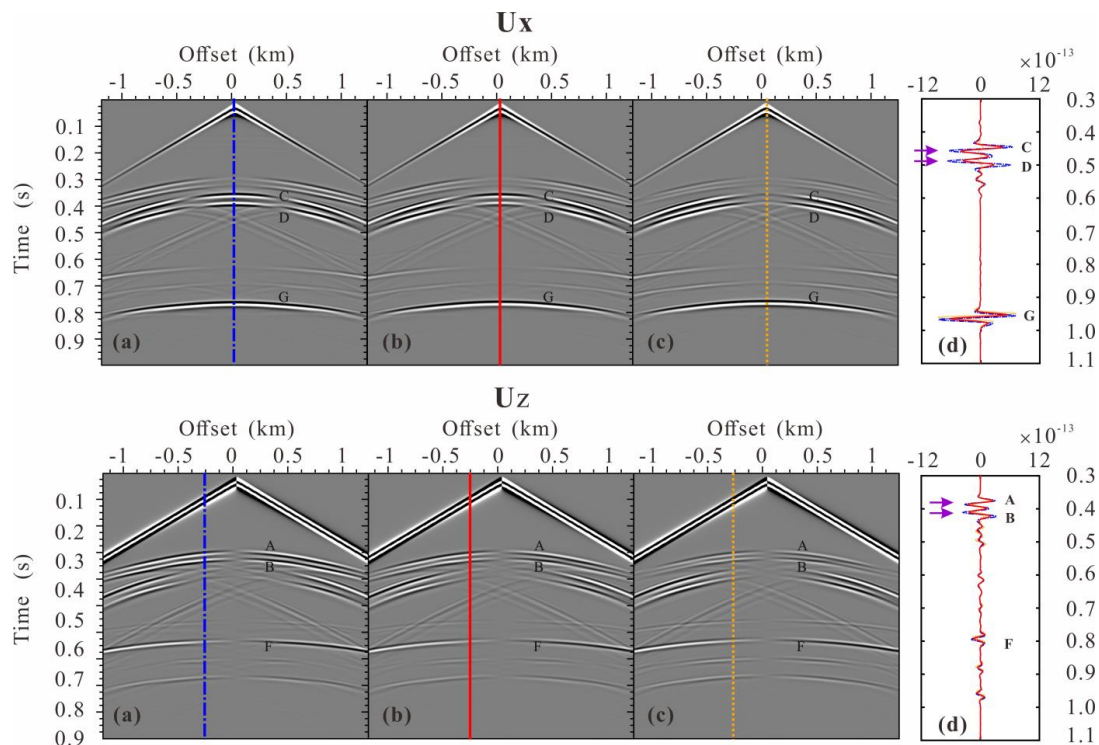
373 **Figure 6: Seismogram components  $U_x$  and  $U_z$  of the fractured reservoir model I due to a P-wave point source: calculated using (a)**  
 374 **the LFLSM, (b) the VLSM, (c) the HFLSM. (d) is the comparison of single trace extracted from the three gathers. A and B are**  
 375 **scattered compressional wave from top and bottom, respectively, C and D are scattered converted wave top and bottom, respectively,**  
 376 **F and G are reflected compressional wave and converted wave, respectively, E is scattered diffracted wave.**

377 Figure 6 presents the seismograms of fractured reservoir model I for a P-wave point source. The scattered compressional wave  
 378 ( $S_{PP}$ ) and scattered converted wave ( $S_{PS}$ ) from the top and bottom of the fractured reservoir, the reflected compressional wave  
 379 ( $R_{PP}$ ), converted wave ( $R_{PS}$ ) from the underlying formation, diffracted wave at the edge of the fractured reservoir can be clearly  
 380 identified. Similar to the single fracture case, the amplitude of the  $S_{PP}$  from the top of the fractured reservoir obtained by the  
 381 HFLSM-based modeling is weakest (underestimated), that obtained by LFLSM-based modeling is strongest (overestimated),  
 382 and that obtained by the VLSM-based modeling is intermediate (accurate). The purple arrows in the Figure 6 (d) indicate that  
 383 the  $S_{PP}$  from the bottom of the fractured reservoir obtained by the LFLSM-based and HFLSM-based modeling has a slightly  
 384 larger amplitude than that from the top, while the  $S_{PP}$  from the bottom of the fractured reservoir obtained by the VLSM-based  
 385 modeling has a slightly smaller amplitude than that from the top. This is expected, since the VLSM-based modeling scheme  
 386 can capture the wave attenuation and dispersion due to the FDP effects between the fracture system and background, while the  
 387 LFLSM and HFLSM represent non-attenuated and non-dispersive elastic processes. However, due to the weak degree of  
 388 dispersion, the  $S_{PP}$  travel time obtained by the three modeling schemes is almost consistent. Figure 6 shows that the amplitudes  
 389 of the  $R_{PP}$  from the underlying formation calculated by the HFLSM-based and LFLSM-based modeling are almost equal, while



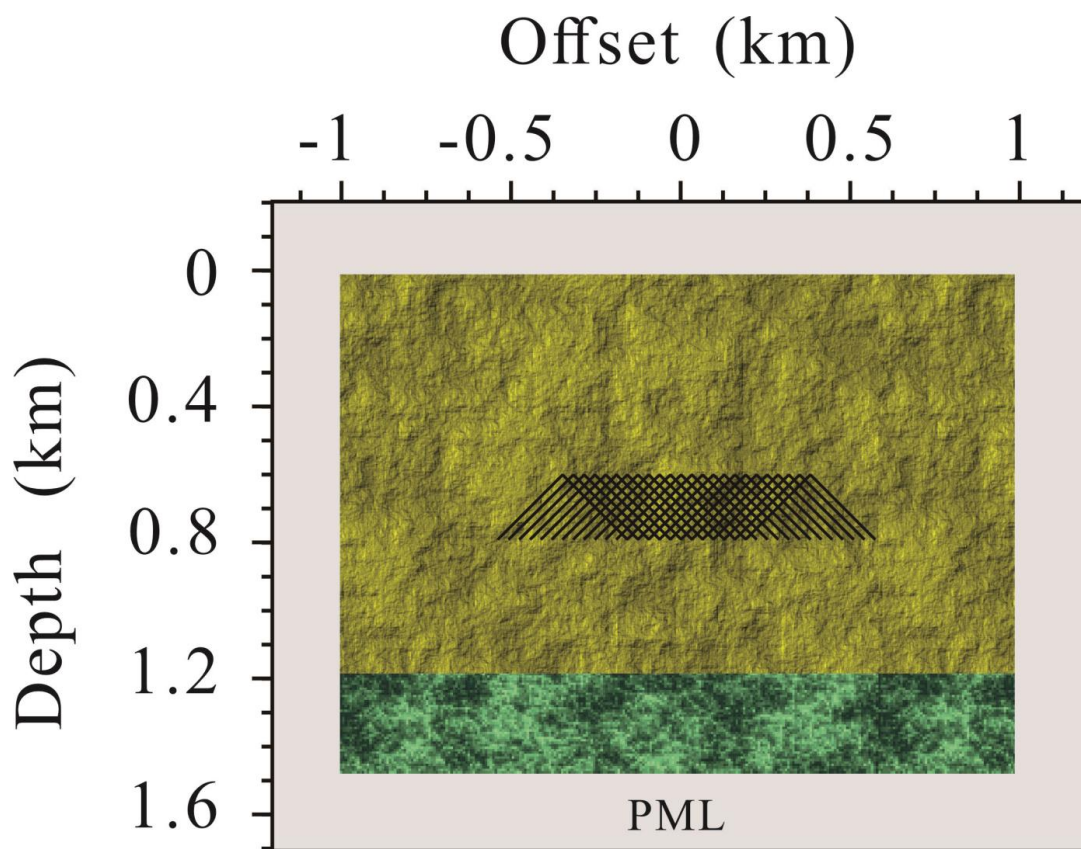


390 that calculated by the VLSM-based modeling is attenuated and dispersed. That again indicates the VLSM-based modeling can  
 391 capture the FPD effects. The  $S_{PS}$  and  $R_{PS}$  show similar behavior as the  $S_{PP}$  and  $R_{PP}$ . Figure 6 suggests that the scattered waves  
 392 from the bottom of the fractured reservoir are attenuated and dispersed by the FPD effects and the reflected waves can retain  
 393 the relevant attenuation and dispersion information.



394 **Figure 7: Seismogram components  $U_x$  and  $U_z$  of the fractured reservoir model I due to a S-wave point source: calculated using (a)**  
 395 **the LFLSM, (b) the VLSM, (c) the HFLSM. (d) is the comparison of single trace extracted from the three gathers. A, B are scattered**  
 396 **converted SP-wave from top and bottom, respectively, C and D are scattered shear SS-wave from top and bottom, respectively, F**  
 397 **and G are reflected converted SP-wave and shear SS-wave, respectively, E is scattered diffracted wave.**

399 Figure 7 presents the seismograms of fractured reservoir model I for a S-wave point source. The scattered converted wave ( $S_{SP}$ )  
 400 and shearing wave ( $S_{SS}$ ) from the top and bottom of the fractured reservoir, the reflected converted wave ( $R_{SP}$ ) and shearing  
 401 wave ( $R_{SS}$ ) from the underlying formation can be identified in Figure 7. Unlike the case of single horizontal fracture, the FPD  
 402 effects between a conjugate fracture system and background can attenuate and disperse the  $S_{PP}$ ,  $S_{PS}$ ,  $R_{PP}$  and  $R_{PS}$  for a S-wave  
 403 point source exploration survey.



404

405

406

**Figure 8: Schematic diagram of the fractured reservoir model II. The normal spacing and extending of each fracture are 1.768m and 282.8m, respectively.**

407

The attenuation and dispersion caused by FDP effects are strongly affected by the thickness of the reservoir. In general, the

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thicker the fractured reservoir, the more severe attenuation and dispersion of the seismic wave. To demonstrate the strong

409

attenuation and dispersion caused by FDP effect, we modify the fractured model I, increase each fracture to 282.8m without

410

changing other parameters, and obtain a fractured model II. Figure 9 presents the seismograms of fractured reservoir model II

411

for a P-wave point source. Figure 9 shows that the  $S_{PP}$  and  $S_{PS}$  from the bottom of the fractured reservoir and the  $R_{PP}$  and  $R_{PS}$

412

from the underlying formation obtained by the VLSM-based modeling are strongly attenuated and dispersed, proving that the

413

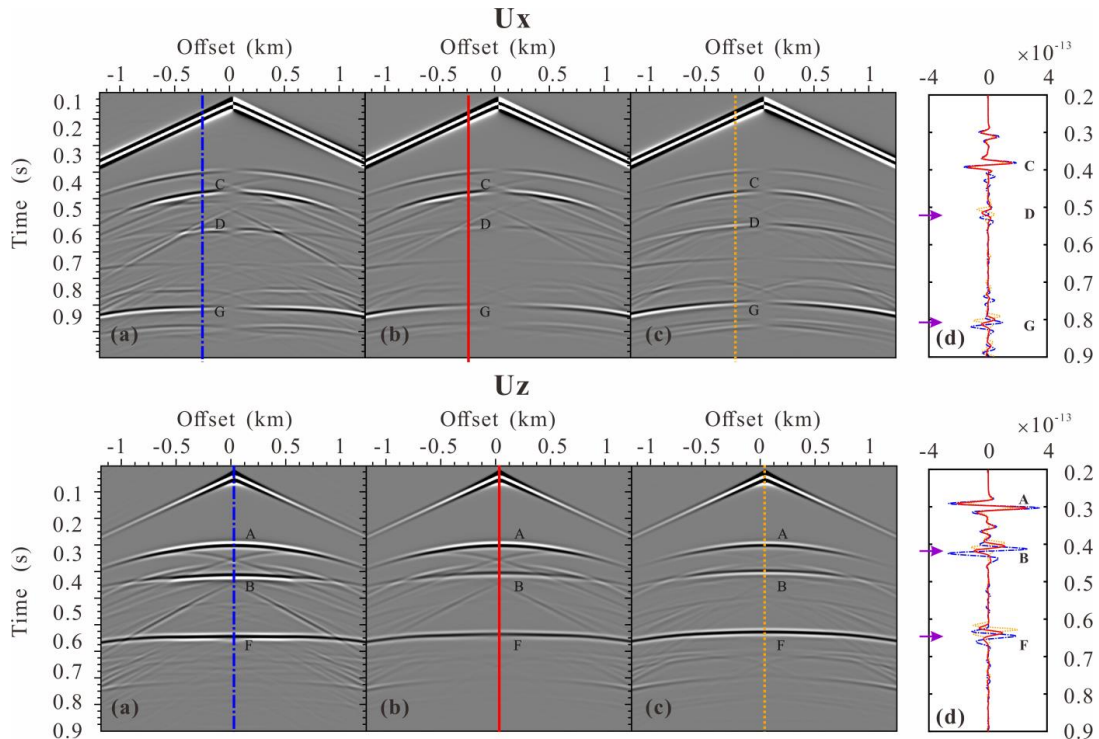
VLSM-based modeling can be captured the FPD effects when seismic waves travel through the fractured reservoir. Figure 10

414

presents the seismograms of the fractured reservoir model II for a S-wave point source. Figure 10 shows that the scattered and

415

reflected waves obtained by VLSM-based modeling are also strongly attenuated and dispersed.



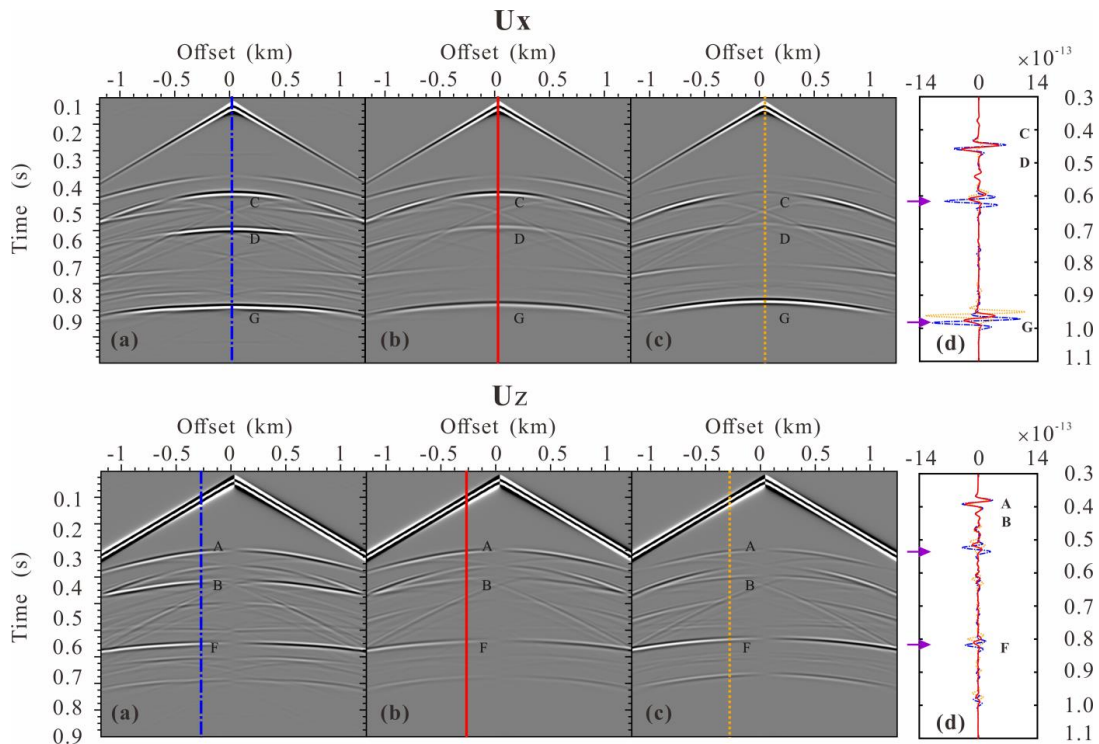
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**Figure 9: Seismogram components  $U_x$  and  $U_z$  of the fractured reservoir model II due to a P-wave point source: calculated using (a) the LFLSM, (b) the VLSM, (c) the HFLSM. (d) is the comparison of single trace extracted from the three gathers. The meanings of A, B, C, D, E, F and G are same as those in Figure 9.**

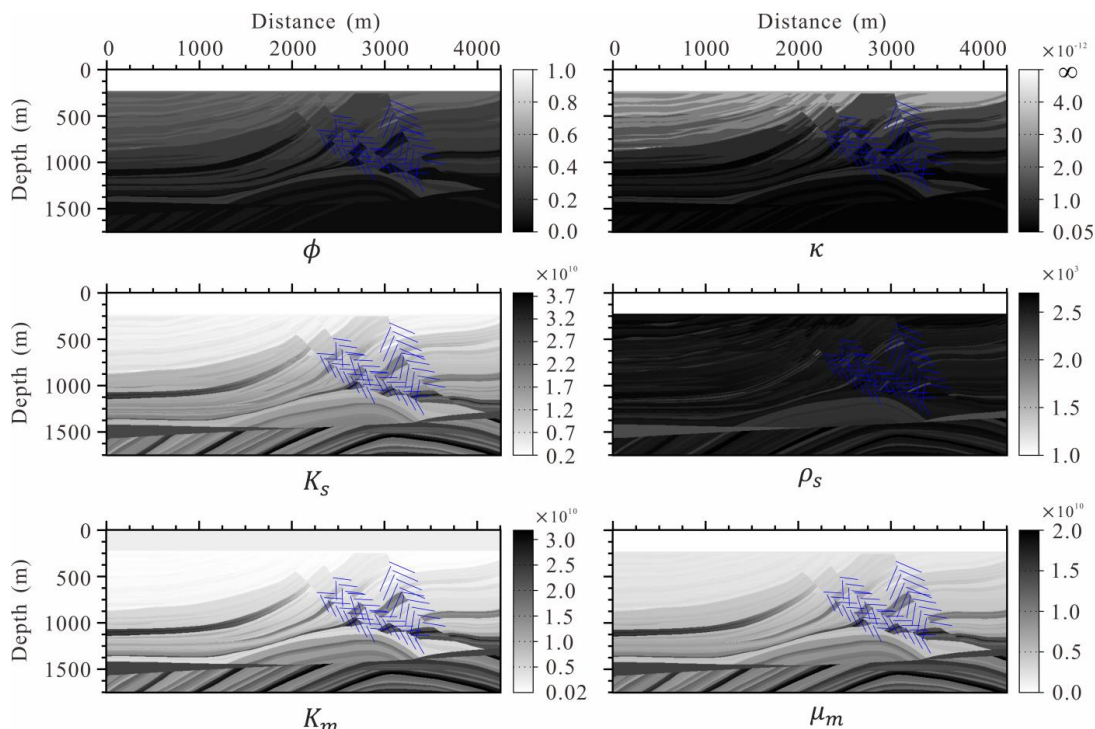


420



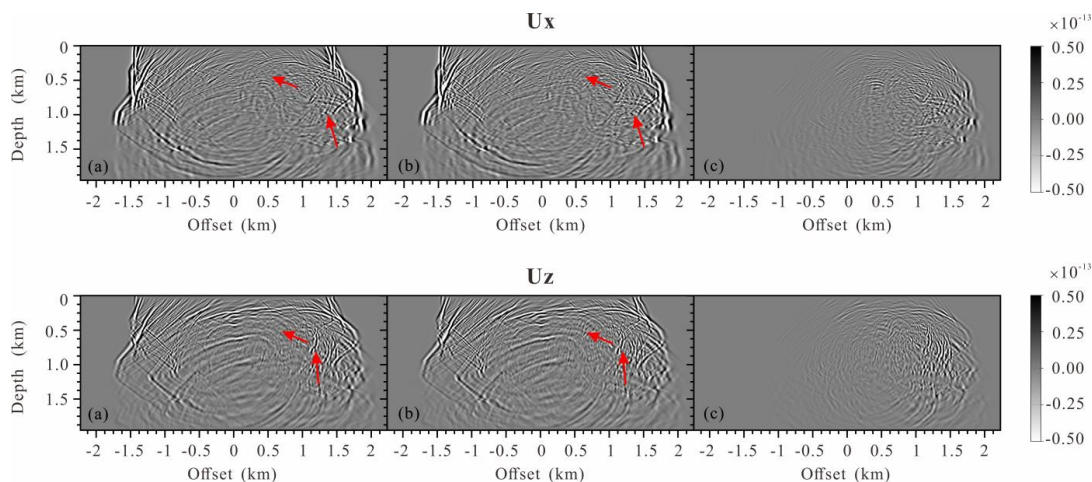
421 **Figure 10: Seismogram components  $U_x$  and  $U_z$  of the fractured reservoir model I due to a S-wave point source: calculated using (a)**  
 422 **the LFLSM, (b) the VLSM, (c) the HFLSM. (d) is the comparison of single trace extracted from the three gathers. The meanings of**  
 423 **A, B, C, D, E, F and G are same as those in Figure 10.**

424 **6.3 Modified Marmousi model**



425 **Figure 11: The physical properties and elastic modulus models of the modified Marmousi model.**

427 We test the proposed VLSM-based modeling scheme on a more complex modified Marmousi model. To modify the Marmousi  
 428 model, we generate a porosity model, permeability model and discrete large-scale fracture system, and transform the original  
 429 P-wave velocity and density into the fluid saturated bulk and shear modulus of the background by a constant Poisson's ratio  
 430 0.5, and finally obtain the grain bulk modulus, the frame bulk and shear modulus of the background through Gassmann  
 431 equation and empirical formula ( $K_m = (1 - \phi)^3 / (1 - \phi) K_s$ ). The input physical properties and elastic modulus models of the  
 432 modified Marmousi model are present in Figure 11. The fluid density, bulk modulus and viscosity are the same as in Table 1.  
 433 The model size is 4250m×1750m with grid interval 5m and a 100m thick PML boundary. The source is located at the surface  
 434 (2125m, 0m). A Ricker wavelet with a central frequency of 25Hz is used as the temporal source excitation.

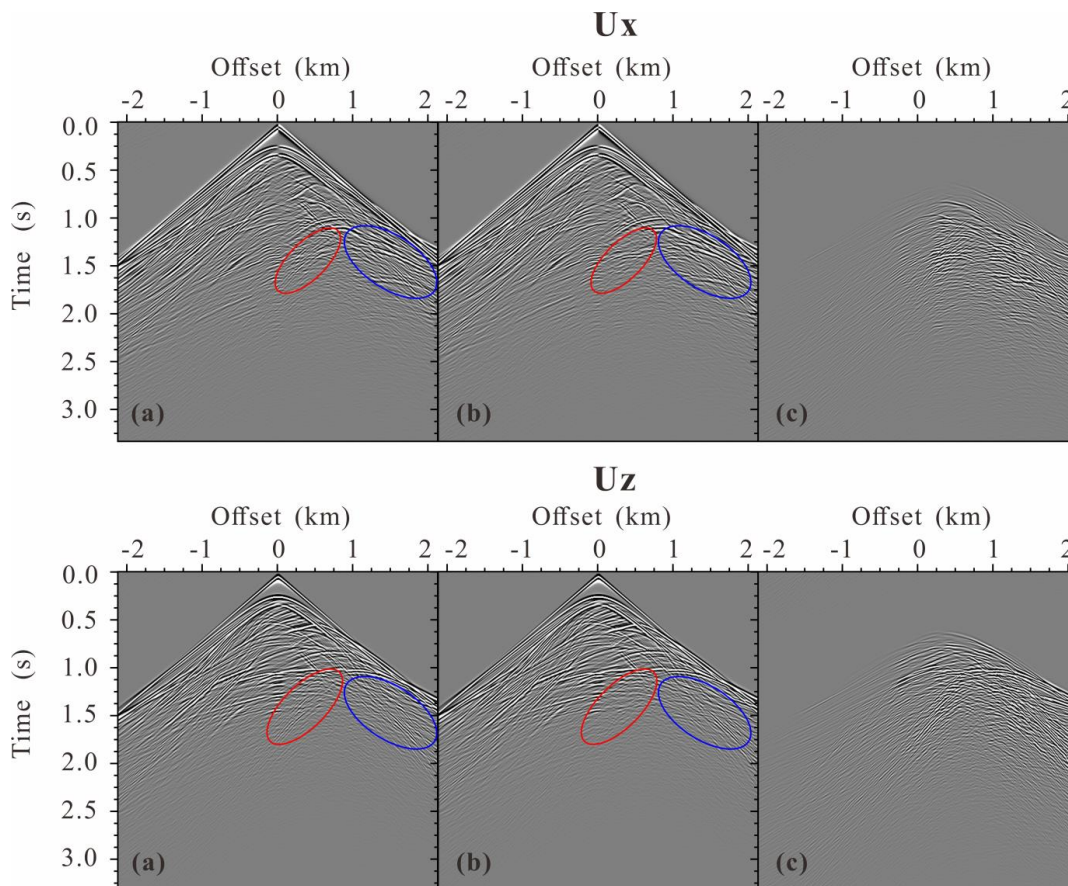


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**Figure 12: Snapshots of the wavefields components  $U_x$  and  $U_z$  at 1000ms: (a) the original Marmousi model without fractures, (b) the modified Marmousi model with fractures and (c) the differences.**



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**Figure 13: Seismogram components  $U_x$  and  $U_z$ : (a) the modified Marmousi model with fractures, (b) the original Marmousi model without fractures and (c) the differences.**

441

Figures 12 shows the snapshots of displacement fields at 1000ms. The figure clearly shows the scattered P- and S-waves by



442 the discrete distributed large-scale fractures. The results with such a complex model clearly verify the numerical  
443 implementation and the code. We also calculate the seismograms of the displacement shown in Figure 13. The seismograms  
444 obtained by our proposed modeling scheme present the scattered seismic waves by the discrete fractures.

## 445 7. Conclusions

446 In this work, we have developed a numerical modeling scheme including FPD effects for discrete distributed large-scale  
447 fractures embedded in fluid saturated porous rock. To capture the FPD effects between the fractures and background, the  
448 fractures are represented as Barbosa's VLSM with complex-valued and frequency-dependent fracture compliances. Using  
449 Coates and Schoenberg's local effective medium theory and Barbosa's VLSM, we derive the effective anisotropic viscoelastic  
450 compliances in each spatial discretized cell by superimposing the compliances of the background and the fractures. The local  
451 effective governing equations of numerical cells are expressed by the derived effective compliances and discretized by mixed-  
452 grid stencil FDFD. The proposed modeling scheme can be used to study the impact of mechanical and hydraulic of fracture  
453 properties on seismic scattering.

454 The numerical results of the single horizontal fracture model with a P-point source valid that the proposed VLSM-based  
455 modeling can include the FPD effects and thus accurately estimate the scattered wave of the horizontal fracture. In contrast,  
456 the LFLSM-based modeling overestimates the scattered wave and the HFLSM-based modeling underestimates the scattered  
457 wave. The numerical results with an S-point source show that the scattered waves off a single horizontal fracture is less  
458 sensitive to FDP effects. Due to the differences in fracture orientation, the results of the conjugate fractured reservoir model  
459 are quite different from those of the single horizontal fracture model. For both P- and S-point sources, the amplitudes of the  
460 scattered waves from the top of the fractured reservoir are affected by the fluid stiffening effects due to the FPD effects. The  
461 scattered waves from the bottom of the fractured reservoir are also attenuated and dispersed by the FPD effects in addition to  
462 the fluid stiffening effects and the reflected waves can retain the relevant attenuation and dispersion information. The results  
463 of the modified Marmousi model clearly show the scattered P- and S-waves by the discrete distributed large-scale fractures  
464 and verify the proposed numerical modeling scheme. The proposed numerical modeling scheme is expected not only to  
465 improve the estimations of seismic wave scattering from discrete distributed large-scale fractures but can also to improve  
466 migration quality and the estimation of fracture mechanical characteristics in inversion.

## 467 Appendix A: The coefficients related to spatial derivative operators

468 We define coefficient vectors  $\mathbf{T}_k (k = 1,2,3,4)$  and the derivative operate vector  $\mathbf{D}(c)$  as

$$469 \mathbf{T}_1 = \frac{1}{\xi_x \xi_x} [1 \ 0 \ 0 \ 0], \mathbf{T}_2 = \frac{1}{\xi_x \xi_z} [0 \ 1 \ 0 \ 0], \mathbf{T}_3 = \frac{1}{\xi_x \xi_z} [0 \ 0 \ 1 \ 0], \mathbf{T}_4 = \frac{1}{\xi_z \xi_z} [0 \ 0 \ 0 \ 1], \quad (\text{A-1})$$

$$470 \mathbf{D}(c) = [\partial_x(c\partial_x) \ \partial_x(c\partial_z) \ \partial_z(c\partial_x) \ \partial_z(c\partial_z)], \quad (\text{A-2})$$



471 where  $\xi_x$  and  $\xi_z$  are the PML damping function,  $c$  represents effective stiffness. Then, the expression of  $A_c, B_c, C_c, D_c$  are  
 472 written in matrix form:

$$473 \begin{bmatrix} A_c \\ B_c \\ C_c \\ D_c \end{bmatrix} = \begin{bmatrix} \mathbf{D}(c_{11}) & \mathbf{D}(c_{15}) & \mathbf{D}(c_{15}) & \mathbf{D}(c_{55}) \\ \mathbf{D}(c_{15}) & \mathbf{D}(c_{55}) & \mathbf{D}(c_{13}) & \mathbf{D}(c_{35}) \\ \mathbf{D}(c_{15}) & \mathbf{D}(c_{13}) & \mathbf{D}(c_{55}) & \mathbf{D}(c_{35}) \\ \mathbf{D}(c_{55}) & \mathbf{D}(c_{35}) & \mathbf{D}(c_{35}) & \mathbf{D}(c_{33}) \end{bmatrix} \begin{bmatrix} \mathbf{T}_1 \\ \mathbf{T}_2 \\ \mathbf{T}_3 \\ \mathbf{T}_4 \end{bmatrix}. \quad (\text{A-3})$$

474 We formulate  $A_r, B_r, C_r, D_r$  in a similar way by defining the coefficient vectors  $\mathbf{T}'_k (k = 1,2,3,4)$  and  $\mathbf{D}'(c)$  as

$$475 \mathbf{T}'_1 = \frac{1}{2\xi_x\xi_x} [1 \quad 1 \quad 1 \quad 1]^T, \quad \mathbf{T}'_2 = \frac{1}{2\xi_x\xi_z} [-1 \quad 1 \quad -1 \quad 1]^T,$$

$$476 \mathbf{T}'_3 = \frac{1}{2\xi_x\xi_z} [-1 \quad -1 \quad 1 \quad 1]^T, \quad \mathbf{T}'_4 = \frac{1}{2\xi_z\xi_z} [1 \quad -1 \quad -1 \quad 1]^T, \quad (\text{A-4})$$

$$477 \mathbf{D}'(c) = [\partial_{x'}(c\partial_{x'}) \quad \partial_{x'}(c\partial_{z'}) \quad \partial_{z'}(c\partial_{x'}) \quad \partial_{z'}(c\partial_{z'})]. \quad (\text{A-5})$$

478 The expression of  $A_r, B_r, C_r, D_r$  are written as

$$479 \begin{bmatrix} A_r \\ B_r \\ C_r \\ D_r \end{bmatrix} = \begin{bmatrix} \mathbf{D}'(c_{11}) & \mathbf{D}'(c_{15}) & \mathbf{D}'(c_{15}) & \mathbf{D}'(c_{55}) \\ \mathbf{D}'(c_{15}) & \mathbf{D}'(c_{55}) & \mathbf{D}'(c_{13}) & \mathbf{D}'(c_{35}) \\ \mathbf{D}'(c_{15}) & \mathbf{D}'(c_{13}) & \mathbf{D}'(c_{55}) & \mathbf{D}'(c_{35}) \\ \mathbf{D}'(c_{55}) & \mathbf{D}'(c_{35}) & \mathbf{D}'(c_{35}) & \mathbf{D}'(c_{33}) \end{bmatrix} \begin{bmatrix} \mathbf{T}'_1 \\ \mathbf{T}'_2 \\ \mathbf{T}'_3 \\ \mathbf{T}'_4 \end{bmatrix}. \quad (\text{A-6})$$

## 480 Appendix B: Parsimonious staggered-grid stencil

481 The nine coefficients of the CS stencil for the submatrix  $A_c$  of Eq. (36):

$$482 A_{c\ i+1,j} = \frac{c_{11\ i+\frac{1}{2},j}}{\Delta^2\xi_x\xi_x\ i+\frac{1}{2}}, \quad A_{c\ i-1,j} = \frac{c_{11\ i-\frac{1}{2},j}}{\Delta^2\xi_x\xi_x\ i-\frac{1}{2}}, \quad A_{c\ i,j+1} = \frac{c_{55\ i,j+\frac{1}{2}}}{\Delta^2\xi_z\xi_z\ j+\frac{1}{2}}, \quad A_{c\ i,j-1} = \frac{c_{55\ i,j-\frac{1}{2}}}{\Delta^2\xi_z\xi_z\ j-\frac{1}{2}},$$

$$483 A_{c\ i,j} = -\frac{c_{11\ i+\frac{1}{2},j}}{\Delta^2\xi_x\xi_x\ i+\frac{1}{2}} - \frac{c_{11\ i-\frac{1}{2},j}}{\Delta^2\xi_x\xi_x\ i-\frac{1}{2}} - \frac{c_{55\ i,j+\frac{1}{2}}}{\Delta^2\xi_z\xi_z\ j+\frac{1}{2}} - \frac{c_{55\ i,j-\frac{1}{2}}}{\Delta^2\xi_z\xi_z\ j-\frac{1}{2}}, \quad A_{c\ i+1,j+1} = \frac{c_{15\ i+1,j}+c_{15\ i,j+1}}{4\Delta^2\xi_x\xi_z\ j},$$

$$484 A_{c\ i+1,j-1} = -\frac{c_{15\ i+1,j}+c_{15\ i,j-1}}{4\Delta^2\xi_x\xi_z\ j}, \quad A_{c\ i-1,j+1} = -\frac{c_{15\ i-1,j}+c_{15\ i,j+1}}{4\Delta^2\xi_x\xi_z\ j}, \quad A_{c\ i-1,j-1} = \frac{c_{15\ i-1,j}+c_{15\ i,j-1}}{4\Delta^2\xi_x\xi_z\ j}. \quad (\text{B-1})$$

485 The nine coefficients of the RS stencil for the submatrix  $A_r$  of Eq. (36):

$$486 A_{r\ i+1,j} = \frac{c_{11\ i+\frac{1}{2},j-\frac{1}{2}}-c_{55\ i+\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j-\frac{1}{2}} + \frac{c_{11\ i+\frac{1}{2},j+\frac{1}{2}}-c_{55\ i+\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j+\frac{1}{2}}, \quad A_{r\ i-1,j} = \frac{c_{11\ i-\frac{1}{2},j-\frac{1}{2}}-c_{55\ i-\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j-\frac{1}{2}} + \frac{c_{11\ i-\frac{1}{2},j+\frac{1}{2}}-c_{55\ i-\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j+\frac{1}{2}},$$

$$487 A_{r\ i,j+1} = \frac{c_{55\ i-\frac{1}{2},j+\frac{1}{2}}-c_{11\ i-\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j+\frac{1}{2}} + \frac{c_{55\ i+\frac{1}{2},j+\frac{1}{2}}-c_{11\ i+\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j+\frac{1}{2}}, \quad A_{r\ i,j-1} = \frac{c_{55\ i-\frac{1}{2},j-\frac{1}{2}}-c_{11\ i-\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j-\frac{1}{2}} + \frac{c_{55\ i+\frac{1}{2},j-\frac{1}{2}}-c_{11\ i+\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j-\frac{1}{2}},$$

$$488 A_{r\ i,j} = -\frac{c_{11\ i+\frac{1}{2},j-\frac{1}{2}}-2c_{15\ i+\frac{1}{2},j-\frac{1}{2}}+c_{55\ i+\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j-\frac{1}{2}} - \frac{c_{11\ i-\frac{1}{2},j+\frac{1}{2}}-2c_{15\ i-\frac{1}{2},j+\frac{1}{2}}+c_{55\ i-\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j+\frac{1}{2}} - \frac{c_{11\ i+\frac{1}{2},j+\frac{1}{2}}+2c_{15\ i+\frac{1}{2},j+\frac{1}{2}}+c_{55\ i+\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j+\frac{1}{2}} - \frac{c_{11\ i-\frac{1}{2},j-\frac{1}{2}}+2c_{15\ i-\frac{1}{2},j-\frac{1}{2}}+c_{55\ i-\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j-\frac{1}{2}},$$

$$489 A_{r\ i+1,j+1} = \frac{c_{11\ i+\frac{1}{2},j+\frac{1}{2}}+2c_{15\ i+\frac{1}{2},j+\frac{1}{2}}+c_{55\ i+\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j+\frac{1}{2}}, \quad A_{r\ i+1,j-1} = \frac{c_{11\ i+\frac{1}{2},j-\frac{1}{2}}-2c_{15\ i+\frac{1}{2},j-\frac{1}{2}}+c_{55\ i+\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j-\frac{1}{2}},$$

$$490 A_{r\ i-1,j+1} = \frac{c_{11\ i-\frac{1}{2},j+\frac{1}{2}}-2c_{15\ i-\frac{1}{2},j+\frac{1}{2}}+c_{55\ i-\frac{1}{2},j+\frac{1}{2}}}{4\Delta^2\xi_x\xi_z\ j+\frac{1}{2}}, \quad A_{r\ i-1,j-1} = \frac{c_{11\ i-\frac{1}{2},j-\frac{1}{2}}+2c_{15\ i-\frac{1}{2},j-\frac{1}{2}}+c_{55\ i-\frac{1}{2},j-\frac{1}{2}}}{4\Delta^2\xi_z\xi_x\ j-\frac{1}{2}}. \quad (\text{B-2})$$

491 The coefficients of the submatrices  $B_c, C_c, D_c$  and  $B_r, C_r, D_r$  can be inferred easily from those of submatrix  $A_c$  and  
 492  $A_r$ , respectively.



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