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1           **Elucidating the role of soil hydraulic properties on the aspect-dependent landslide**  
2           **initiation**

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8           **Abstract:** Aspect-dependent landslide initiation is an interesting finding, and previous studies have attributed this  
9           observation to the mechanical effects of plant roots. In the present study work, an overwhelming landslide probability  
10          on a south-facing slope over a north-facing slope was found in a localized area with on merely granite underneath  
11          and high cover of *Larix kaempferi* species. These such observations can not be attributed to plant roots, but  
12          may result from factors related to hillslope hydrology the hillslope hydrology related factors. The differential  
13          weathering associated with hillslope hydrology behaviors, such as rainfall water storage and leakage, pore water  
14          pressure, particle component and hillslope stability fluctuation, were used to elucidate these such observations.  
15          Remote sensing interpretation using the high-resolution GeoEye-1 image and digitalized topography  
16          showed revealed found that the landslides on south-facing slopes have a higher probability, larger basal area, and  
17          shallower depth than those on a north-facing slope. The lower limits of the upslope contributing area and slope  
18          gradient condition for south-facing landslides were less than those for north-facing landslides. The higher basal  
19          areas of south-facing landslides than those of the north-facing landslides may be attributed to the high peak  
20          values and slow dissipation of pore water pressure. The absorbed and drained water flow in a given time interval,  
21          together with the calculated water storage and leakage during the measured rainy season measured, demonstrate,  
22          sufficiently prove that the soil mass above the failure zone for the south-facing slopes is more prone to form pore-  
23          water pressure, which and results in slope failures. In comparison, the two stability fluctuation results from the finite  
24          and infinite models further verified that landslides on south-facing slopes may fail under on conditions of prolonged  
25          antecedent precipitation and intensive rainfall. Meanwhile, whereas while those on north-facing slopes may fail  
26          on merely in response to intensive rainfall. The results of this study will deepen our knowledge of work provide an  
27          insightful view of the aspect-dependent landslide initiation from both classical mechanics and the state of stress.

28           **Keywords:** Landslide; Pore pressure; suction stress; Hydraulic conductivity; Slope stability

29           **1 Introduction**

30          In some semi-arid environments of the Northern Hemisphere, aspect-dependent landslide initiation would  
31          provide valuable insights into the relative importance of different factors in developing accurate landslide  
32          susceptibility models (Ebel, 2015; Rengers et al., 2016; Li et al., 2021; Deng et al., 2022). These events provide a  
33          thoroughful understanding of about the amount of direct sunlight that translates into differences in vegetation  
34          communities, bedrock weathering, and soil development processes (Fu, 1983; Wang, 2008; Bierman and  
35          Montgomery, 2014). These typical earth surface processes indirectly affect hillslope hydrology and landscape  
36          dissection at the hillslope scale. Importantly, Rainfall-induced shallow landslides are one of the geomorphic  
37          agents at the hillslope scale and are governed by multiple factors, including hydrology, hillslope materials, bedrock  
38          underneath, and the vegetation (Birkeland, 1999; Geroy et al., 2011; Lu and Godt, 2013). Currently, the observed  
39          aspect-dependent landslide initiation observed has been predominantly is mainly attributed to the mechanical effect  
40          of plant roots. This is, because the differences in vegetation on the south- and north-facing slopes are easier to  
41          examine and more pronounced obvious than other factors (Li et al., 2021; Timilsina et al., 2021; Dai et al., 2022;  
42          Deng et al., 2022). However, it is no denying that vegetation succession takes place over substantially longer

43 ~~timescales is far slowly than the~~ soil development and bedrock weathering (Watakabe and Matsushi, 2019).  
44 ~~In most cases, and the plant# roots are in most cases is~~ not deep enough to penetrate into ~~the~~ bedrock (Schwinning,  
45 2010). Hypothesizing ~~for in~~ a ~~relatively~~ localized area with ~~the~~ same ecosystem or plant species, aspect-  
46 dependent landslide initiation can~~not be~~ attributed to plant roots, ~~but while~~ may result from ~~the~~ differences  
47 in ~~the~~ properties of hillslope materials due to long-term differential weathering.

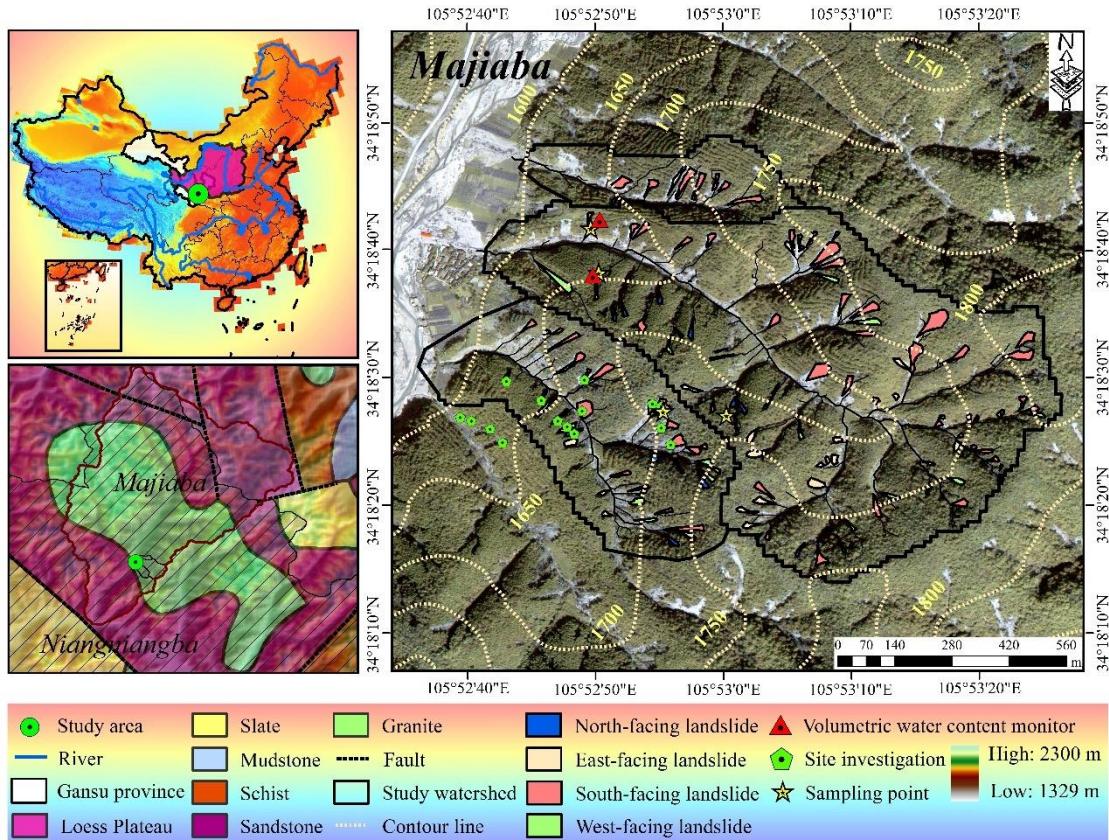
48 ~~AThe~~ aspect-dependent landslides in ~~the~~ Frontal Colorado, USA and the Loess Plateau, China, have attracted  
49 ~~interest because vegetation has interesting focus that vegetation generates a~~ considerable influence on ~~the~~ landslide  
50 distribution. ~~The~~~~In~~ fact, ~~the~~ ~~strong overwhelming~~ propensity for shallow landslide initiation on south-facing  
51 hillslopes in the two regions ~~is~~ closely related~~s~~ to the present-day tree density, regardless of ~~the~~ hillslope aspect  
52 (Ebel, 2015; Rengers et al., 2016; Deng et al., 2022). In the Colorado Frontal Range, field observations ~~have~~  
53 ~~shown proved~~ that south-facing slopes lack thick tree cover and have an abundance of rock outcrops compared to  
54 north-facing slopes. ~~In addition, and~~ the soil layer ~~is~~~~would be~~ thinner on south-facing slopes (Coe et al., 2014; Ebel  
55 et al., 2015). The ~~apparent~~ cohesion supplied by ~~the~~ roots ~~is~~ was responsible for the ~~observed~~ connection ~~observed~~  
56 between landslide distribution and slope aspect (McGuire et al., 2016). ~~On~~~~In~~ the Loess Plateau ~~China~~, vegetation  
57 recovery is ~~one of the main at the major~~ ecological measures ~~for mitigating to mitigate~~ the sediment loss (Fu et al.,  
58 2009). ~~Increased Promoted~~ soil strength and hydraulic conductivity due to strong root networks~~s~~ may enhance ~~the the~~  
59 topographic initiation conditions~~s~~ (Montgomery and Dietrich, 1994; Wang et al., 2020). ~~Another possibility is that~~  
60 ~~the~~ ~~Na~~orth- and west-ward moving storms~~s~~ ~~may potentially produce produced~~ more intense rainfall on ~~the~~ south- and  
61 east-facing slopes. ~~Such an This~~ assumption may be invalid if ~~an~~ aspect-dependent landslide distribution ~~is~~  
62 ~~present exists~~ in a localized catchment with ~~a specific given~~ vegetation community~~ies~~. ~~The~~~~In~~ fact, ~~the~~ above-  
63 mentioned ~~This~~ study highlights the effect of ~~the~~ mechanical function of plants on landslides~~s~~. If ~~an~~ the aspect-  
64 dependent landslide exists in a localized area ~~that are with vegetation cover comprising covered by the~~ same plant  
65 species ~~alongside a high level of and high~~ vegetation coverage, the ~~observed~~ aspect-dependent landslide initiation  
66 ~~observed~~ can~~not be~~ attributed to the mechanical effect ~~offrom~~ plant roots.

67 To ~~determine elucidate~~ the ~~observed~~ relationship ~~observed~~ among vegetation, landslides~~s~~ and slope aspect, the  
68 effects ~~offrom~~ ~~the~~ physical properties and strength of hillslope materials cannot be ~~excluded ignored~~. ~~On~~~~In~~ the  
69 ~~n~~Northern part of ~~the~~ Loess Plateau, China, as well as in many other semi-arid environments, different types and  
70 densities of vegetation and soils develop on north-facing versus south-facing convergent slopes. ~~This is~~, because  
71 systematic differences in the amount of direct sunlight translate into differences in ~~the~~ physical and chemical  
72 weathering. North-facing convergent slopes have lower evaporation rates, retain snow cover longer in spring, and  
73 tend to hold soil moisture longer ~~during to~~ the summer growing season. ~~These~~~~Such~~ differences may result in localized  
74 ecosystem communities in ~~the~~ presence of trees or shrubs ~~on ever~~ grasses. South-facing slopes experience heavier  
75 and more frequent hydration, thermal expansion, or freeze-thaw cycles ~~due to by~~ the day warming and night cooling,  
76 and tend to ~~have favor~~ stronger weathering throughout the year. ~~Such~~~~These~~ differences ~~can could~~ result in local  
77 differences in ~~the~~ grain component, soil strength, and soil profile. ~~This has indirect effects at which indirectly affect~~  
78 the landslide scale ~~through by the~~ mechanics of excessive pore water pressure dissipation and sliding surface  
79 liquefaction (Terzaghi, 1950; Sassa, 1984), and ~~the~~ hillslope hydrology behavior (Godt et al., 2009; Lee and Kim,  
80 2019). Therefore, the physical properties of ~~the~~ hillslope materials may ~~be~~ attributed to the ~~observed~~ aspect-  
81 dependent landslide initiation ~~observed~~.

82 ~~It is well known that As known to All~~, shallow landslides are ~~an one of the~~ examples of debris flow initiation,  
83 which often enlarges~~s~~ their scale by multiple mechanics (Hung et al. 2005; Iverson et al. 2011). When the slope fails,  
84 the pore water pressure abruptly increases within the shear zone (Iverson and LaHusen, 1989; Wang and Sassa, 2003).  
85 If ~~the~~ excessive pore water pressure persists ~~high~~ over the static pressure for a ~~relatively~~ long duration, the displaced  
86 masses ~~will~~ enlarge their volume by widespread liquefaction, and transform into debris flows (Bogaard and Greco,

87 2016). In other words, the magnitude of the pore water pressure is closely related to the scale of the shallow  
 88 landslides. Therefore, the scale of the shallow landslides can be determined by the role of excessive  
 89 pore water pressure during the failure process. However, the aspect-dependent landslide distribution in these two  
 90 above-mentioned areas merely refers to the differences in landslide probability rather than and, not the landslide  
 91 scale.

92 In the present study, we used a combination of field soil moisture observation, strength measurement and  
 93 hydraulic conductivity analysis of hillslope materials, and numerical modeling of slope stability to explain the high  
 94 potential for the overwhelming landslide initiation on south-facing slopes relative to north-facing slopes with the  
 95 same, where the vegetation communities are the same. In addition, the differences in landslide geometry and  
 96 initiation conditions, in the form of the contributing area above the scar area and the landslide gradient, were shown  
 97 using exhibited by the field studies investigations and high-resolution GeoEye-1 images. Then, the differential  
 98 weathering-related physical properties and strength of the soil mass, including the dry unit weights, porosity and  
 99 grain size, effective cohesion, and inner friction angle, were examined. Importantly, we have also highlighted the  
 100 importance of the excessive pore water pressure, hillslope hydrology, and stability in explaining the observed  
 101 aspect-dependent landslide initiation observed. The results of this work will deepen our may provide an insightful  
 102 understanding of the aspect-dependent landslide distribution in some mountainous areas of the Northern  
 103 Hemisphere.



110 City, Gansu Province, Central China. It is also close to the dividing crest of the Yellow River and Yangtze Rivers;  
111 and on the eastern part of the Loess Plateau. ~~The m~~ Majority ~~Most~~ of the hillslope is ~~are~~ underlain by slate, and the  
112 stratigraphic units of granite, sandstone, and mudstone account for a relatively smaller area. This area in semi-humid  
113 climate region and has four distinctive seasons and a semi-humid climate. The annual precipitation is  
114 approximately 491.6 mm and predominantly falls mostly falls during June and August. One branch fault of the  
115 Tianshui–Lanzhou fault system runs through the area and has had no rupture records for the last few decades.

116 The shallow landslides in the study area and nearby surroundings were triggered by the prolonged antecedent  
117 precipitation during July 1–24 to 24 July and the intensive rainstorm on 25 July 25, 2013 (Yu et al., 2014; Guo et  
118 al., 2015). Previous studies found that majority of Most shallow landslides in the entire storm spanned the  
119 mountain area with a gradient of 20–25°, located on south-facing slopes and in areas with relatively sparse  
120 vegetation (Li et al., 2021). In addition Besides, the strong root network may promote the hydraulic conductivity  
121 of the soil–root composite and the landslide initiation condition of the upslope contributing area–slope gradient,  
122 according to the landslide case studies from the Larix kaempferi and Pinus tabulaeformis forests (Dai et al.,  
123 2022). In this study work, the three small catchment areas in the Majiaba Watershed are underlain by granite  
124 units. The total area is 0.88 km<sup>2</sup> with vegetation cover coverage rate of over 90% (Fig. 1). The relative relief was  
125 approximately about 200 m, and the mean hillslope gradient was 37°. The reasons why choose the three catchments  
126 lie in the area were chosen is that the main plant species on the south- and north-facing slope is Larix kaempferi,  
127 which commonly have highly developed lateral roots with depth < 0.4 m. However, landslides in  
128 the three catchments still have a higher exhibit overwhelm propensity for occurrence on south-facing slopes in  
129 comparison with the over north-facing slopes. This Such a finding differentiates from the results from the Frontal  
130 Colorado, USA, and the Central Loess Plateau, where landslides commonly occur in sparsely vegetated areas.  
131 Furthermore, the works of Li et al. (2021) only merely addressed the relationship between landslide probability and  
132 vegetation cover at the coverage on a regional scale, while excluding neglecting the importance of the properties  
133 of hillslope materials at a more localized scale. Therefore, we hypothesize that such observations in the study area  
134 may not be the result of the mechanical effect of plant roots, but may be from the distinctive physical properties  
135 and strength of hillslope materials due to differential weathering.

### 136 3 Materials and methods

#### 137 3.1 Landslide information interpretation

138 The high-resolution (0.5 m × 0.5 m) on October 8, 2013. The GeoEye-1 image was orthorectified and the  
139 landslide boundary was visually interpreted using ENVI 5.1 and e-Cognition 8. An unmanned aerial vehicle (UAV  
140 for abbreviation) was used to obtain the digital elevation model (DEM) with a 5 m resolution. The GeoEye-1  
141 orthographic image and DEM were spatially registered in the ArcGIS 10.2 as a standard layer of orthoimage. The  
142 landslide initiation condition is represented by the competition between the slope gradient and the upslope  
143 contribution area ( $A - S$ ):

$$144 S = kA^{-b}$$

(1)

145 where  $S$  is the local slope (m/m);  $A$  is the contribution area above the landslide head-scar (m<sup>2</sup>);  $k$  is an empirical  
146 constant, which is related to lithology, vegetation, and climate; and  $b$  is an empirically defined index.

147 Field studies investigations were conducted mainly to measure the depth of the head-scar and sidewall area  
148 using tape, and the failure depth was taken as the average of them. Then, the landslide volume could then be  
149 calculated using the interpreted scar area and the measured depth. Finally, detailed  
150 landslide information, including the landslide number and area probability, landslide volume and width, head-scar  
151 and sidewall depth, and as well as the upslope contributing area–slope gradient condition for the south- and north-

153 facing slopes were compared.

### 154 3.2 Field monitoring and soil sampling

155 To investigate the hillslope hydrology on the south- and north-facing slopes, the Frequency Domain  
156 Reflectometry (FDR) soil moisture sensors were used in this work to record the volumetric water content. To avoid  
157 the randomness of data caused by natural factors such as terrain and vegetation, a total of 16 shallow landslides were  
158 investigated to excavate soil profiles and take undisturbed soil samples. Then, the sensors were installed  
159 implemented at depths of 30 cm, 70 cm, and 110 cm on the south- and north-facing slopes to monitor the volumetric  
160 water content during the rainy season of 2021. The soil moisture monitoring was implemented at works were  
161 implemented on two concave sites on the south- and north-facing slopes. The meteorological station was less no  
162 more than 3 km away from the study area to record the rainfall on a 30 min basis. During the sensor installations  
163 implementation, the undisturbed soil samples near the sensor location were taken for indoor tests, including the  
164 dry unit weight, porosity, grain size, shear strength, and hydraulic conductivity. The grain size was analyzed using  
165 by Malvern MS 3000 instrument (Malvern, England). In each layer, at least four samples were collected for the  
166 consolidated undrained triaxial compression test (CU). And two samples were collected for unsaturated hydraulic  
167 conductivity measurement using transient release and imbibition method tests (Lu and Godt, 2013). Saturated  
168 hydraulic conductivity was determined using the constant water head method (Table 1).

### 169 3.3 Pore water pressure dissipation

170 CU tests were performed to obtain the effective cohesion, effective internal friction angle, and the  
171 pore pressure dissipation curves. The soil samples, with a diameter of 50 mm and height of 100 mm, were  
172 firstly saturated in a vacuum pump. They were then consolidated in the chamber of the GDS apparatus at 50,  
173 100, 150, and 200 kPa confining pressures and 10 kPa backpressure. During each test, the shearing rate was set to  
174 0.1 mm/min, and the device automatically recorded data every 10 s. Owing to the varied particle  
175 components and soil texture, the increasing and dissipation ratio varied. Furthermore, this ratio is closely  
176 related to the widespread generation of excessive pore water pressure, which increases with  
177 enlarge the landslide scale. A High excessive pore water pressure, rapid increase ratio, and slow dissipation ratio  
178 could cause widespread Coulomb failure within the sliding zone. To demonstrate that the pore water pressure  
179 increases or dissipates, the ratio is:

$$i = \frac{p_{t+\Delta t} - p_t}{\Delta t} \quad (2)$$

180 where  $i$  is the increase or dissipation ratio of the excessive pore water pressure, and  $p_t$  and  $p_{t+\Delta t}$  are the measured  
181 pore water pressures measured during the time interval of  $\Delta t$ .

### 182 3.3 Water storage and drainage

183 The unsaturated permeability of soil mass (diameter 61.8 mm, height 25.4 mm) was measured using the  
184 Transient Release and Imbibition method (TRIM) (Lu and Godt, 2013). In this test, the water outflow mass was  
185 measured on a 10 minutes basis. In each test, the air pressures of 250 kPa and 0 kPa corresponded to the drying and  
186 wetting processes, respectively. Thus, the Soil Water Characteristic Curve (SWCC) and the Hydraulic Conductivity  
187 Function (HCF) were obtained by using the Hydrus 1-D (Wayllace and Lu, 2012). Using the models  
188 proposed by Mualem (1976) and van Genuchten (1980), the constitutive relations between the suction head ( $h$ ),  
189 water content ( $\theta$ ), and hydraulic conductivity ( $K$ ) under drying and wetting states could be represented by the  
190 following equation:

$$\frac{\theta - \theta_r}{\theta_s - \theta_r} = \left[ \frac{1}{1 + (\alpha|h|)^n} \right]^{1-\frac{1}{n}} \quad (3)$$

191 and

---

194

$$K = K_s \frac{\left\{1 - (\alpha|h|)^{n-1}[1 + (\alpha|h|)^n]^{\frac{1}{n}-1}\right\}^2}{[1 + (\alpha|h|)^n]^{\frac{1}{2}-\frac{1}{2n}}} \quad (4)$$

195 where  $\theta_r$  is the residual moisture content (%)<sup>196</sup>;  $\theta_s$  is the saturated moisture content (%)<sup>197</sup>;  $\alpha$  and  $n$  are empirical fitting parameters<sup>198</sup>, with  $\alpha$  being the inverse of the air-entry pressure head<sup>199</sup>, and  $n$  is the pore size distribution parameter<sup>200</sup>;  $K_s$  is the saturated hydraulic conductivity (cm/s)<sup>201</sup>.

198 The soil water storage ( $S_s$ ) and drainage ( $S_d$ ) during a rainfall event can be evaluated by the soil depth and the  
199 difference between the maximum soil moisture and the antecedent soil moisture:

200

$$S_e = \frac{\theta - \theta_r}{\theta_s - \theta_r} \quad (5)$$

201

$$S_s = S_e^w \Delta h \quad (6)$$

202

$$S_d = P - S_e^d \Delta h \quad (7)$$

203 where  $S_e$  is the degree of saturation<sup>204</sup>;  $\theta$  is the measured volumetric moisture content measured (%)<sup>205</sup>;  $\Delta h$  is the average soil thickness<sup>206</sup> (400 mm in this studywork)<sup>207</sup>;  $S_e^w$  and  $S_e^d$  are the residual soil moisture in the wetting and drying processes<sup>208</sup> (%);  $P$  is the accumulated rainfall (mm)<sup>209</sup>.

### 206 3.4 Stability fluctuation

207 In this studywork, we applied a finite and infinite stability model to assess the slope stability fluctuation  
208 during the rainy season, as an attempt to examine the aspect-dependent landslide initiation from the perspective  
209 of classical mechanics and the state of stress (Schmidt et al., 2001). The finite-slope model evaluates the stability  
210  $F_s$ :

211

$$F_s = \frac{S_{sr}}{\tau} = \frac{c_l A_l + c_b A_b + A_b (\rho_s - \rho_w) S_e g z \cos^2 \beta \tan \varphi'}{A_b \rho_s g z \sin \beta \cos \beta} \quad (8)$$

212 where  $\beta$  is the topographic slope angle (°)<sup>213</sup>;  $A_l$  is the lateral area,  $\text{m}^2$ <sup>214</sup>;  $A_b$  is the basal area,  $\text{m}^2$ <sup>215</sup>;  $z$  is the sliding depth (m)<sup>216</sup>;  $c_l$  is the sum of the effective soil cohesion and the root additional cohesion along the perimeter (kPa)<sup>217</sup>;  $\text{kPa}$ <sup>218</sup>;  $c_b$  is the basal soil cohesion (kPa)<sup>219</sup>;  $\text{kPa}$ <sup>220</sup>;  $\rho_s$  is the soil particle density,  $\text{g/cm}^3$ <sup>221</sup>; and  $\rho_w$  is the water density,  $\text{g/cm}^3$ <sup>222</sup>.

216 The infinite slope stability model in this studywork provides insight into the stress variation resulting from  
217 changes in the soil suction and water content during infiltration (Lu and Likos, 2006):

218

$$F_s = \frac{\tan \varphi'}{\tan \beta} + \frac{2c'}{\gamma z \sin 2\beta} - \frac{\sigma^s}{\gamma z} (\tan \beta + \cot \beta) \tan \varphi' \quad (9)$$

219 where  $\varphi'$  is the effective friction angle, °;  $\beta$  is the topographic slope angle, °;  $c'$  is the effective cohesion, kPa;  $\gamma$   
220 is the unit weight of the soil, KN/m<sup>3</sup>; and  $\sigma^s$  is the suction stress (kPa)<sup>221</sup>, and expressed as:

221

$$\sigma^s = -\frac{S_e}{\alpha} (S_e^{n/(1-n)} - 1)^{1/n} \quad (10)$$

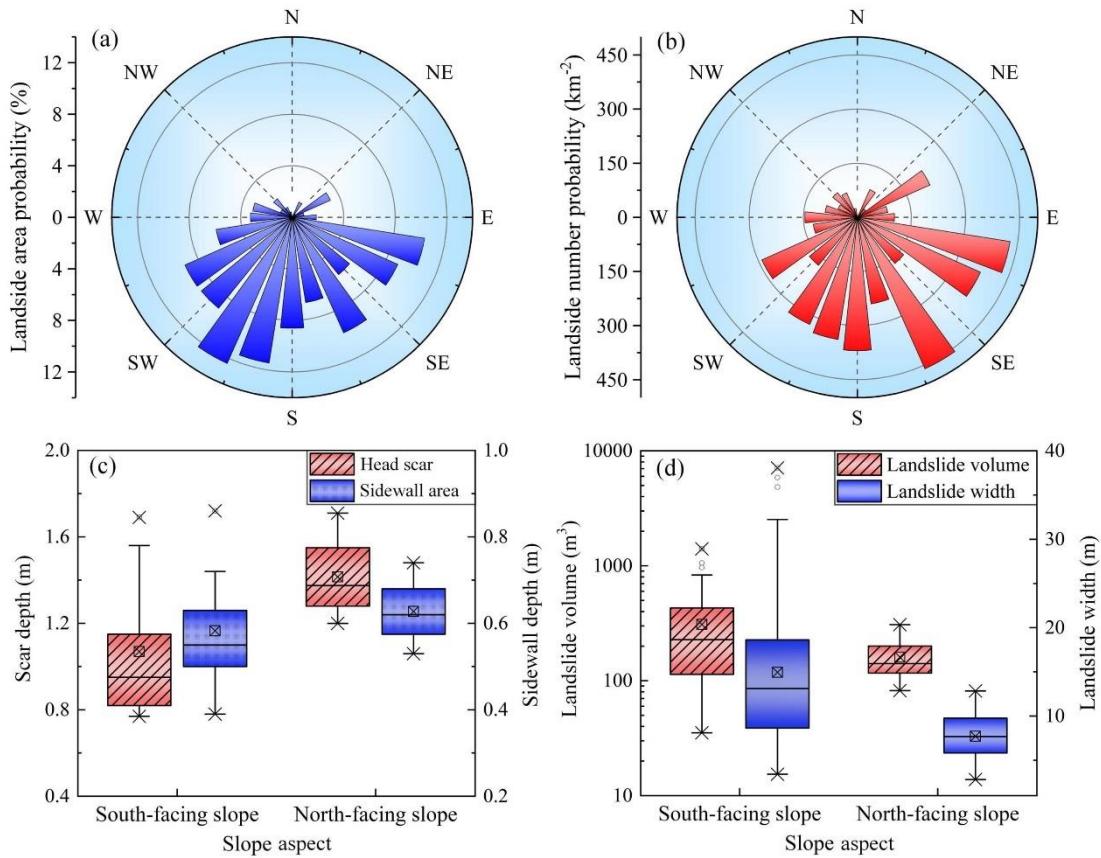
## 222 4 Results

### 223 4.1 Shallow landslides on south- and north-facing slope

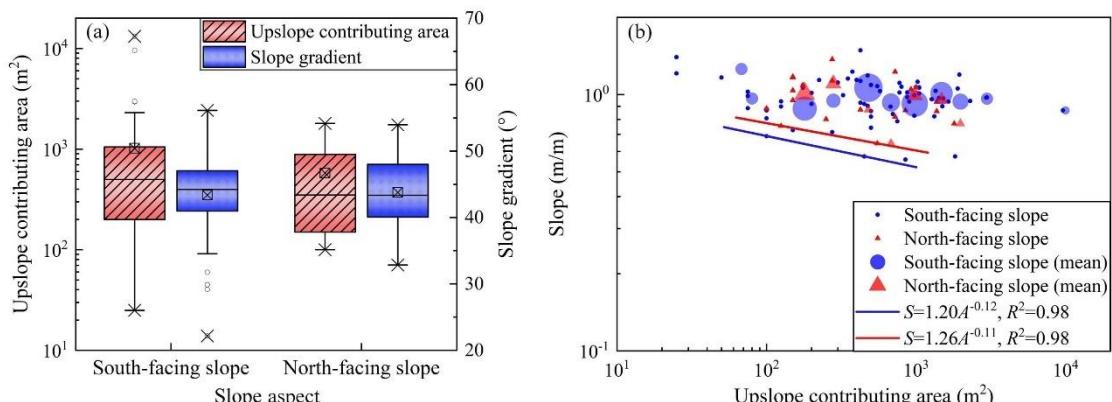
224 In the study area, the south-facing slope was between 157.5 ° and 247.5 ° and the north-facing slope ranged<sup>225</sup> from 0 ° to 67.5 °, and 292.5 ° to 360 ° (0 ° is the due north). There were 71 shallow landslides on the south-facing  
226 slope, and while on<sup>227</sup> merely 20 landslides on the north-facing slope. Figure 2a shows<sup>228</sup> indicates that the shallow  
227 landslides on south-facing slopes have exhibited<sup>229</sup> larger<sup>230</sup> spatial areas than those on north-facing slopes. The Meanwhile,  
228 majority of<sup>231</sup> Most of the shallow landslides occurred<sup>232</sup> on the south-facing slope (Fig. 2b). Furthermore, The  
229 volume of landslides on the south-facing slope was<sup>233</sup> greater<sup>234</sup> than those on the north-facing slope. For  
230 landslides on the south-facing slope, the basal area was<sup>235</sup> 372.64 m<sup>2</sup> and the width was<sup>236</sup> 14.9 m on average. For

231 landslides on the north-facing slope, the averaged basal area was is merely 157.28 m<sup>2</sup> and the width was 7.7 m  
232 (Fig. 2c). Altough the landslides on the south-facing slope had a larger volume and greaterwider width, the  
233 depth of the head-scar and the sidewall area are no greater than those on the more than the landslides on north-facing  
234 slope. Field studies showed investigation reveals that the averaged depth for landslides on the north-facing slope  
235 was 1.02 m, which was deeper than the depth of 0.83 m for landslides on south-facing slope (Fig. 2d). The LL  
236 all, landslides on the south-facing slope exhibited an overwhelming propensity for occurrence in terms of number  
237 and area. Meanwhile, the failure depth was no more than that of the landslides on the north-facing slope.

238 Shallow landslides can be modeled as occurring when sufficient through-flow converges from the upslope  
239 contribution area to the hollow area and triggers slope instability (Montgomery and Dietrich, 1994). Their  
240 topographic initiation conditions are condition is controlled by the spatial competition between the slope and upslope  
241 contribution being area dependent (Stock and Dietrich 2003 and 2006; Horton et al., 2008). For the shallow  
242 landslides in the study area, the averaged upslope contributing area and the slope gradient did not differ significantly  
243 differdo not differentiate a lot (Fig. 3a). Meanwhile, while the lower limit line representing the minimum initiation  
244 condition fore landslides on south-facing slopes was lower than that on the north-facing slopes (Fig. 3b). This  
245 indicates that a higher upslope contributioncontributing area was required to provide sufficient through-flow  
246 conditions and trigger slope failures on the north-facing slope. Given thatAs the landslides in the study area were  
247 triggered by the prolonged antecedent precipitation and intensive rainfall (Li et al., 2021), sufficient rainfall  
248 infiltration could result in a high soil water content within the displaced mass, leading to a decrease inof the matrix  
249 suction and soil strength. The generation of pore pressure generation in response to intense rainfall also plays an  
250 important role in shallow landslides. Therefore, we have proposed two assumptions to elucidate the distribution and  
251 scale of the aspect-dependent landslides distribution and scale. The first assumption is that the basal area of the  
252 landslide may be related to the soil strength and the high pore-water pressure. This assumption can be tested by the  
253 pore water properties, including the pore water generation potential and dissipation ratio, during the failure process.  
254 The second assumption is that the south-facing slope may have a relatively higher failure potential than the  
255 north-facing slope in a given rainfall process, whichThis can be determined elucidated fromby the stability  
256 comparison using the methods of equations (8) and (9).



257  
258 **Fig. 2.** Spatial distribution and geometric characteristics of the landslide: (a) Landslide area probability vs slope aspect; (b) landslide number probability vs slope aspect; (c) landslide volume and width vs slope aspect; (d) scar depth and sidewall depth vs slope aspect. The edge line of the box shows the 75<sup>th</sup> quantile, median and 25<sup>th</sup> quantile from top to bottom. The length of the box is the interquartile distance. The crossed square inside the box is the average value. The whiskers extend to the maximum and minimum values except the outliers. The circles are the outliers, and the cross symbol is the maximum and minimum values for all the data).



265  
266 **Fig. 3.** Upslope contributing area and slope gradient condition: (a) Upslope contribution area and mean slope vs slope aspect; and (b) the upslope contributing area vs mean slope gradient above the landslide area. (The large icons are the average value with the radius size proportional to the number of landslides. The small icons represent all the individual data values).

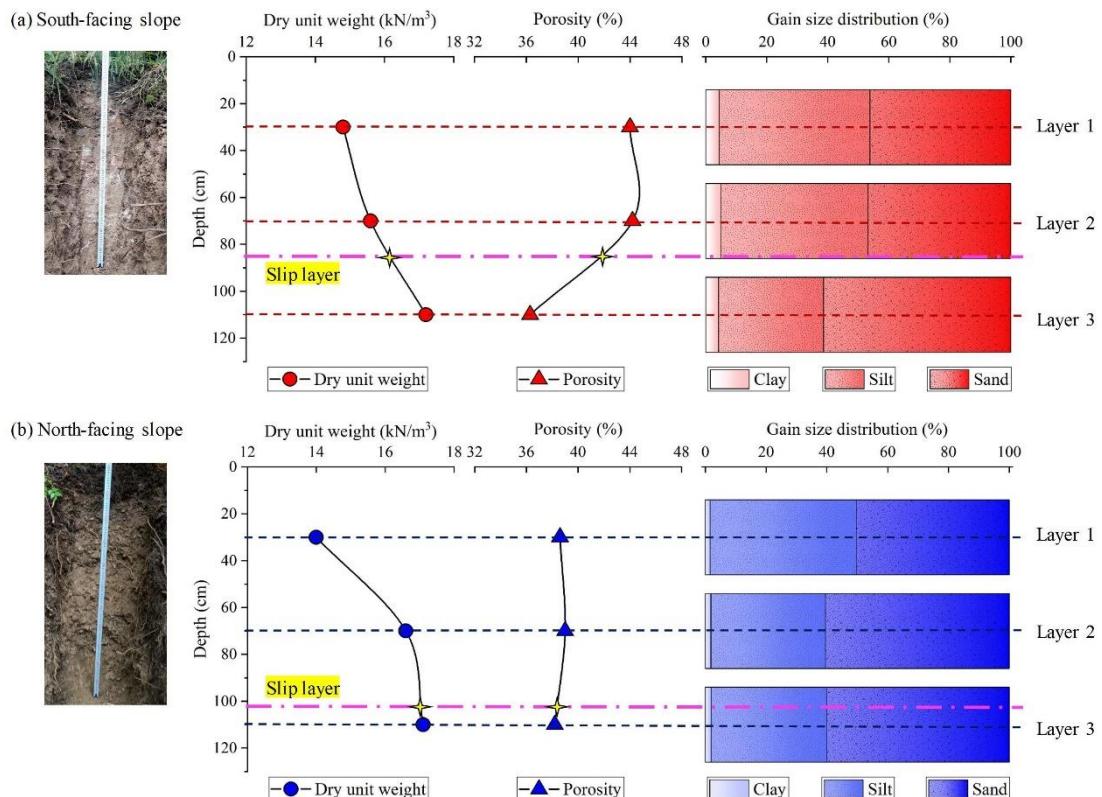
#### 4.2 Differences in soil physical properties

270 To show the differences in the physical properties of the hillslope materials, the dry unit weights, porosity, 271

272 and grain size distribution of the soil mass in the three layers of each slope were firstly compared (Fig. 4). Then,  
 273 the effective cohesion and inner friction angle were then examined with respect to the particle component (Table 1  
 274 and Fig. 5).

275  
 276 Table 1 Physical properties and strength parameters of the soil mass

Parameters	South-facing slope			North-facing slope		
	Layer 1	Layer 2	Layer 3	Layer 1	Layer 2	Layer 3
Unit weight of soil (kN/m <sup>3</sup> )	14.8	15.6	17.2	14	16.6	17.1
Porosity (%)	43.0	43.1	36.2	42.5	37.3	36.4
Effective cohesion (kPa)	6.5	17.5	21.2	5.3	9.1	7.9
Effective inner friction angle (°)	29.8	25	31	27.1	35.2	41
Saturated hydraulic conductivity (cm/s)	$6.4 \times 10^{-3}$	$6.2 \times 10^{-4}$	$4.4 \times 10^{-4}$	$8.8 \times 10^{-3}$	$1.2 \times 10^{-3}$	$4.3 \times 10^{-3}$

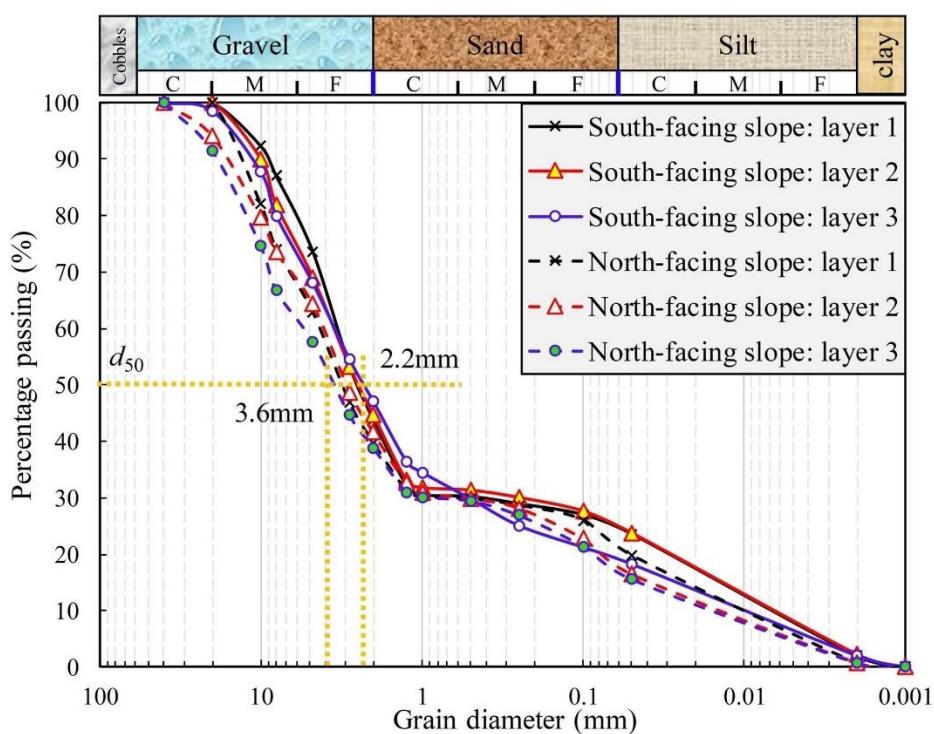


277  
 278 Fig. 4. Differences in the soil properties, including dry unit weights, porosity, and grain size in sand, silt, and clay.  
 279 (a) Physical properties of soil mass on the south-facing slope; and, (b) physical properties of soil mass on the  
 280 north-facing slope. (The two-soil profile photos were taken by Yanglin Guo during field studies investigations.)

281  
 282 For the soil mass on the south-facing slope, the dry unit weights increased with soil depth, whereas the  
 283 porosity and saturated hydraulic conductivity decreased (Fig. 4a and Table 1). For the soil layers No. 1 and 2, the  
 284 soil textures were similar, because the proportions of sand, silt, and clay did not differ significantly (Fig. 4a).  
 285 However, the proportion of silt in the soil layer No. 3 was no more than that in layers No. 1 and 2, and the sand proportion was higher. In addition, the average failure depth was above the soil  
 286 layer No. 3 and is below the soil layer No. 2. For the soil mass on the north-facing slope, the dry unit weights  
 287

288 also increased with soil depth. Unlike the south-facing slope, the porosity of the soil mass for the three soil layers  
 289 was approximately about 38% and does not differentiate among them. For the soil texture, the proportion of  
 290 sand in Soil Layer No. 1 was no more than that in Soil Layers No. 2 and 3 (Fig. 4b). MoreoverBesides, The  
 291 depth of the failure plane was close to that of Soil Layer No. 3.

292 In comparison, one of the main noticeable differences was the higher saturated hydraulic conductivity for the  
 293 soil mass above the failure plane on the north-facing slope. Thiswhich may have resulted from the high porosity and  
 294 sand proportion. This indicates that the rainfall infiltration on of the north-facing slope could penetrate faster than  
 295 that of the south-facing slope. Indeed, The soil mass of the three layers on the south-facing slope had a relatively  
 296 higher proportion of fine particles proportion than those on the north-facing slope; if the gravel was considered (Fig.  
 297 5). As noted above, The saturated hydraulic conductivity for the soil masses es from Soil of Layers No. 2 and 3 on the  
 298 south-facing slope was lower than that on the north-facing slope. This is expected reasonable because the porosity  
 299 and proportions of fines on the south-facing slope were relatively higher.



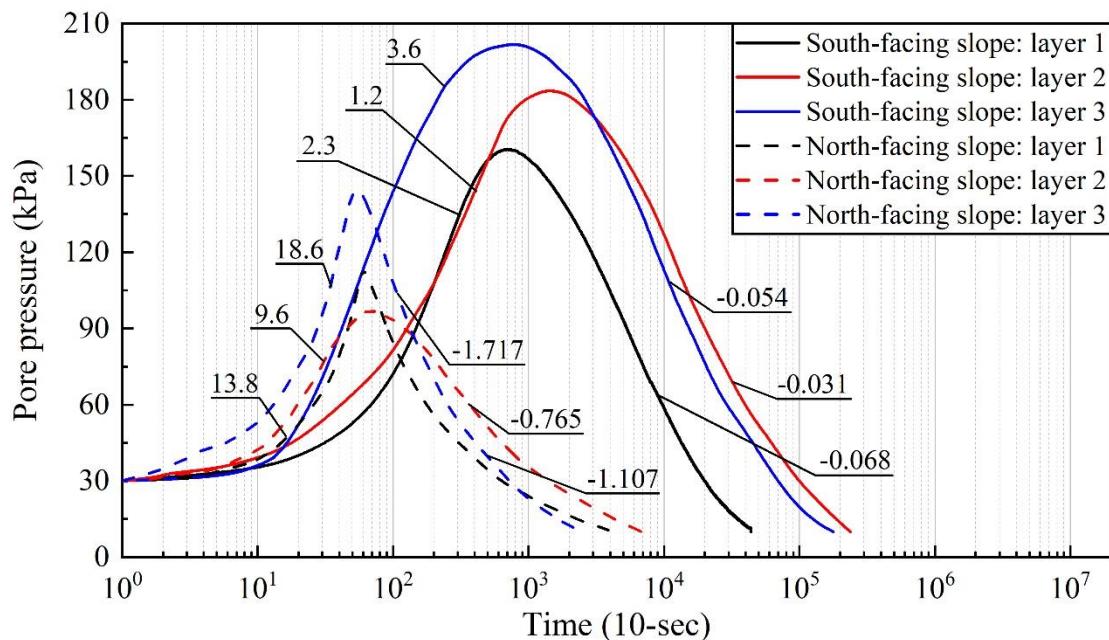
300  
 301 **Fig. 5.** Soil particle component curves

302 According to the results of the triaxial shear test (Table 1), the soil mass in each layer on the nNorth-facing  
 303 slope had a smaller effective cohesion than that one comparing to the south-facing slope. In particular, The effective  
 304 cohesion on the failure plane for landslides on the the south-facing slope s may be twicetwo times of that on the  
 305 north-facing slope s. However, the effective inner friction angles for the soil masses es of Soil Layers No. 2 and 3 on  
 306 the north-facing slope were far greatermore than those on the south-facing slope. TheseSuch differences in effective  
 307 cohesion and inner frictional angle may be attributed to the higher clay and silt and fewerless coarse grains within  
 308 the soil mass on the south-facing slope.

#### 309 **4.3 Pore-water pressure properties**

310 The consolidation module of the triaxial shear test was used to measure the generation and dissipation process  
 311 of the pore water pressure. The principle is to consolidate and drain the soil from the initial saturated state. It was  
 312 found that Under the same confining pressure, there are pronounced obvious differences in the consolidation rate,  
 313 consolidation time, and peak rise in of pore water pressure for of different soil properties of soil. The results of the  
 314 pore water pressure during the consolidation process under 200 kPa effective confining pressure were taken here

(Fig. 6). ~~The~~ It was found that the peak value of pore water pressure within the soil mass on the south-facing slope was higher than that on the north-facing slope. The peak value of ~~the~~ pore water pressure within the soil mass on the south-facing slope ~~increased~~ could rise to 150–200 kPa. However, the peak value of pore water pressure within the soil mass on the north-facing slope was below 150 kPa. Importantly, ~~B~~oth of the rising and decaying rates of pore water pressure for ~~Soil M~~ass layers No. 1 and 2 on the south-facing slope were lower than ~~those~~ on the north-facing slope. ~~In detail, T~~he rising rate and decaying rates for ~~the~~ ~~S~~oil ~~M~~ass layer No. 2 on the south-facing slope were 1.2 kPa/10 s and ~~–~~0.031 kPa/10 s, respectively. However, they ~~are~~ ~~were~~ ~~were~~ 9.6 kPa/10 s and ~~–~~0.765 kPa/10 s for the soil mass on the north-facing slope.



323  
324 **Fig. 6.** Variation ~~in~~ of pore water pressure under effective confining pressure of 200 kPa by GDS triaxial shear tests.  
325

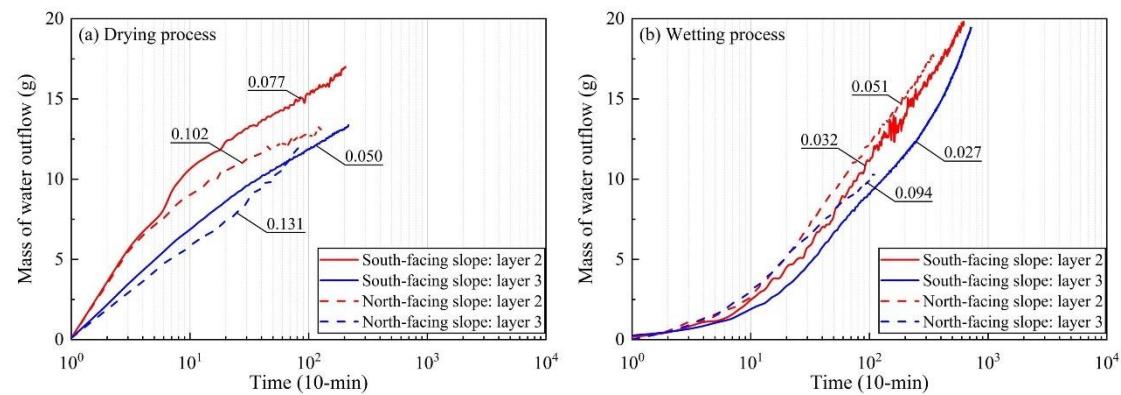
326 ~~In fact, T~~he ~~relatively~~ lower peak pore water pressure ~~demonstrates~~ ~~an~~ ~~illustrates~~ the effect of fine particles  
327 on the pore water pressure, which directly affects ~~the~~ landslide mobility and ~~the~~ scale. ~~It is generally believed that~~  
328 ~~the~~ ~~R~~ainfall-induced landslides result~~s~~ from ~~an~~ increase ~~in~~ positive pore water pressure within the failure plane,  
329 which reduces the effective stress ~~and~~ in the soil and the shear strength of the soil (Terzaghi, 1950). This often occurs  
330 in the undrained soil layer, which ~~can easily~~ is easy to cause slope liquefaction (Sassa, 1984). The increase ~~in~~ pore  
331 water pressure ~~predominantly~~ mainly depends ~~mainly~~ on the speed of landslide movement, soil deformation, and  
332 soil permeability. If the shear rate is ~~the~~ given, the dissipation rate of pore water pressure for high ~~permeability~~ soil  
333 is faster, ~~and therefore thus~~ so the increase ~~in~~ pore pressure is smaller (Iverson and LaHusen, 1989; Iverson et al.,  
334 1997). As shown in ~~T~~able 1, the saturated hydraulic conductivity for ~~S~~oil ~~M~~ass ~~L~~ayers No. 2 and 3 on ~~the~~ ~~north-~~  
335 ~~facing slope was~~ ~~north facing slope are~~ ~~commonly~~ 10 times ~~that~~ of ~~the~~ ~~at~~ ~~on~~ south-facing slope. Therefore, the  
336 measured peak pore water pressure ~~measured~~ during the test for ~~the~~ soil mass on ~~the~~ south-facing slope ~~was~~ ~~would~~  
337 ~~be~~ smaller. ~~In addition~~ ~~B~~esides, ~~T~~he soil mass on the north-facing slope ~~had~~ ~~relatively~~ higher sand and gravels  
338 ~~contents~~ than that on the south-facing slope (Fig. 5). ~~A~~ ~~h~~igh clay content on the south-facing slope ~~would~~ ~~filled~~ the  
339 macropores within ~~the~~ soil mass and reduced ~~the~~ pore water discharge rate ~~of~~ ~~pore~~ ~~water~~. Wang and Sassa (2003)  
340 found that fine particles play the most important role in the dissipation of pore pressure. The pore water pressure  
341 within the saturated sand ~~will~~ increased with ~~the~~ shear rate. The soil mass with high coarse particles ~~will~~ produced  
342 less pore water pressure than the soil with high fine particles ~~during~~ the shear process. Therefore, ~~the~~ high  
343 permeability ~~offer~~ the soil mass on ~~the~~ south-facing slope may result in ~~relatively~~ low peak pore water pressure. The

344 relatively higher fine particles may result in a slow increasing and dissipation of the pore water pressure.  
345 This slow pore water pressure dissipation could result in the liquefaction failure of the sliding mass and a  
346 relatively larger landslide area.

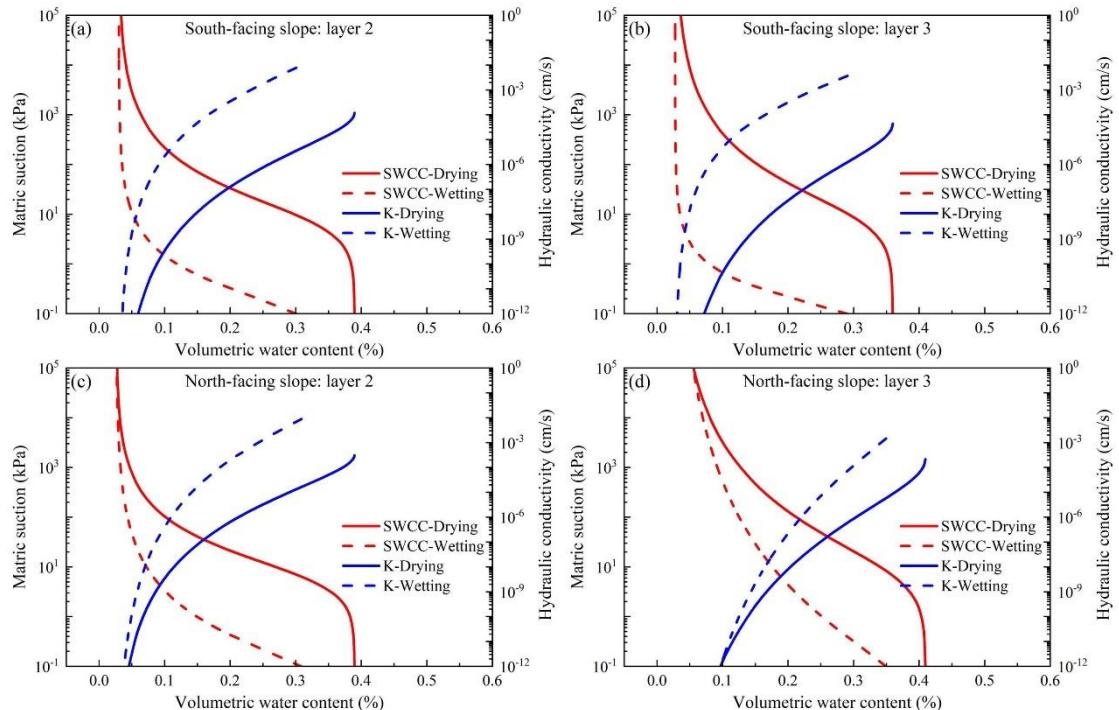
#### 347 4.4 Unsaturated hydraulic conductivity

##### 348 4.4.1 Measured water outflow mass

349 Figure 7 shows the measured water outflow mass measured for a given 10 min period during the  
350 drying and wetting processes. The measured water outflow masses measured for Soil Layers No. 2 and 3 on the  
351 north-facing slope were generally higher than those on the south-facing slope. For the drying tests using the soil  
352 mass of Soil Layers No. 2 and 3 on the north-facing slope, the given water outflow masses were 0.102 g/10 min  
353 and 0.131 g/10 min, respectively. However, the measured water outflow masses measured for the soil mass of Soil  
354 Layers No. 2 and 3 were 0.077 g/10 min and 0.050 g/10 min, respectively, on the south-facing slope, respectively  
355 (Fig. 7a). For tests using the same layers of the soil mass in the wetting process, the measured water outflow masses  
356 measured were 0.051 g/10 min and 0.094 g/10 min on the north-facing slope, respectively, and while those are  
357 0.032 g/10 min and 0.027 g/10 min, respectively, on the south-facing slope (Fig. 7b). Overall As a whole, the  
358 permeability of the soil mass on the north-facing slope was higher than that on the south-facing slope. The  
359 same results were also obtained when the saturated hydraulic conductivities of the soil layers were measured using by  
360 the constant water head method (Table 1).



361  
362 **Fig. 7.** Mass of water outflow during the drying and wetting process: (a) drying tests, (b) wetting tests.



363

364 **Fig. 8.** Soil water characteristic curve obtained using the TRIM test: (a) Layer No. 2 on the south-facing slope,  
 365 (b) Layer No. 3 on the south-facing slope, (c) Layer No. 2 on the north-facing slope, and (d) Layer No. 3 on  
 366 the north-facing slope.

367

#### 368 4.4.2 SWCC and HCF curves

369 Hudraulic properties, such as the Soil Water Characteristic Curve (SWCC) and Hydraulic Conductivity  
 370 Function (HCF), are critical for the analysis of water flow movement and mechanical behavior of unsaturated soil  
 371 material. In this study, the unsaturated hydraulic property measurement adopted the Transient Release and Imbibition  
 372 Method (TRIM) for unsaturated hydraulic property measurement. The intelligent advantage of the TRIM method  
 373 is TRIM method lies in that it combines physical and numerical experiments. In detail, it employs the relatively  
 374 simple and reliable measurement of transient water content using an electronic balance to record the signature of  
 375 transient unsaturated flow. It also takes advantage of the robust inverse modeling capability to simulate the  
 376 physical process. The apparatus could accommodate both undisturbed and remolded samples. The results of this  
 377 study were obtained by using the Hydrus-1D code with the reverse modeling option, which implemented and the  
 378 Levenberg–Marquardt non-linear optimization algorithm. This and minimized the error between the results of the  
 379 test and the simulation (Wayllace and Lu, 2012). Meanwhile, in order to ensure the uniqueness of the parameters,  
 380 the aforementioned algorithm repeatedly runs with different initial parameter estimates, until it always converges  
 381 to obtain the same or similar results. The compare the prediction results are then compared with the function curves  
 382 of water flow and time obtained from the actual experiment, so that they can be basically combined to meet certain  
 383 accuracy requirements. In this experiment, the R square of the regression between the optimized predicted value and  
 384 the observed value was greater than 0.99. In addition, the model constraint effect of the TRIM under two  
 385 suction increment steps was better, and the parameters obtained by the inversion calculation were more accurate  
 386 (Lu and Godt, 2013). Table 2 shows the soil characteristic parameters obtained using the Hydrus 1-D inversion.

387 Using these parameters, the SWCC and HCF curves of the soil mass at Soil Layers No. 2 and 3 on the north-  
 388 and south-facing slopes can be drawn (Fig. 8). Air-entry pressures and residual water content are two important  
 389 parameters that describe the hydrological and mechanical characteristics of the hillslope materials. The air-entry

390 pressures represents the critical value at which air enters the saturated soil and starts to drain. For the Soil Layer  
 391 No. 2, the difference between the air-entry values of the north- and south-facing slopes can reach 14.03 kPa (Figs.  
 392 8a and 8c). In addition Besides, The residual water contents and air-entry pressures of the south-facing slope were  
 393 higher than those of the north-facing slope. For the Soil Layer No. 3, the soil mass on the north-facing slope has  
 394 the smallest air-entry pressure, which is 0.51 times that of the air-entry pressure of the south-facing slope (Figs. 8b  
 395 and 8d). The saturated hydraulic conductivities of Soil Layers No. 2 and 3 on the south-facing slope were lower  
 396 than those on the north-facing slope in both the drying and wetting processes. In particular, The saturated hydraulic  
 397 conductivity of the soil mass on the north-facing slope in the wetting test was one order of magnitude higher than  
 398 that on the south-facing slope. These results suggest simply that it is more difficult for the soil mass the soil mass on  
 399 south facing slope is more difficult to absorb water and drain water than the soil mass on the north-facing slope.

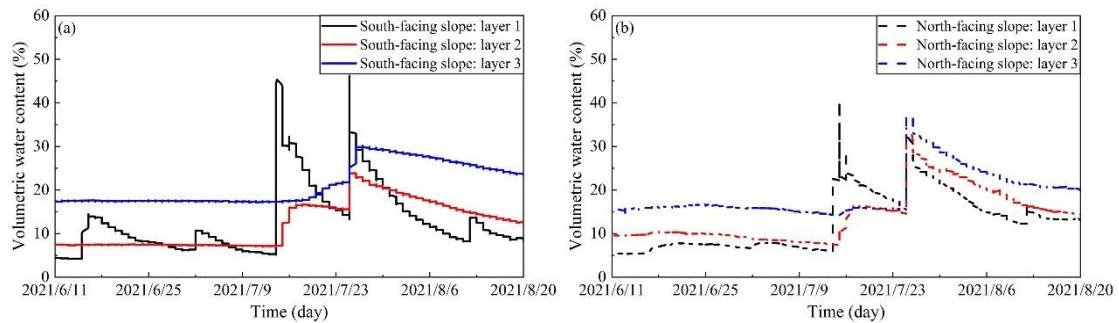
400 Table 2 Parameters describing the Soil and Water Characteristic Curve (SWCC) and the Hydraulic Conductivity  
 401 Function (HCF) from by Hydrus 1-D

Parameters	Definition	South-facing slope		North-facing slope	
		Layer 2	Layer 3	Layer 2	Layer 3
$\theta_r$	Residual moisture	0.0302	0.0278	0.0262	0.0268
$\theta_s^d$	Saturated moisture	0.39	0.36	0.39	0.41
$\theta_s^w$		0.36	0.38	0.39	0.42
$\alpha^d$ (kPa <sup>-1</sup> )	The inverse of the air-entry pressure head	0.0128	0.0117	0.0156	0.0141
$\alpha^w$ (kPa <sup>-1</sup> )		0.78	0.94	1.21	1.86
$n^d$	The pore size distribution parameter	1.49	1.39	1.57	1.27
$n^w$		1.63	1.85	1.43	1.18
$K_s^d$ (cm/s)	Saturated hydraulic conductivity	$1.52 \times 10^{-4}$	$0.64 \times 10^{-4}$	$3.76 \times 10^{-4}$	$4.56 \times 10^{-4}$
$K_s^w$ (cm/s)		$9.58 \times 10^{-2}$	$4.93 \times 10^{-2}$	$4.10 \times 10^{-1}$	$4.68 \times 10^{-1}$

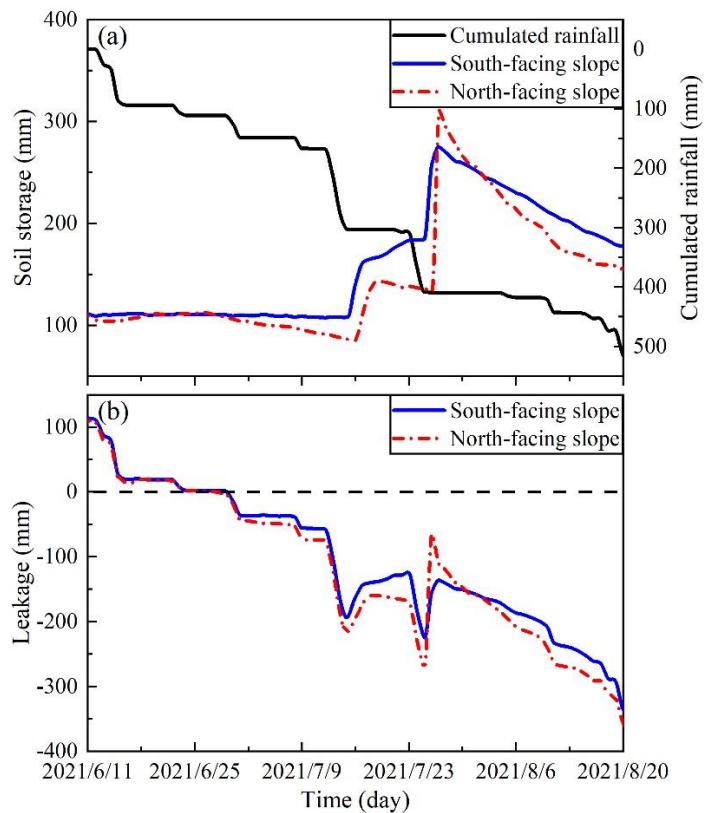
402 Notes: the superscript *d* and *w* indicate drying and wetting states.

#### 403 4.5 Water storage and drainage

404 To show exhibit the water storage during the rainfall process and the water drainage after the rainfall, the timely  
 405 recorded soil moisture at various soil layers and the rainfall process during June 11 and August 20 were used  
 406 (Figs. 9a and 9b). In comparison, this is likely the most important significant finding, as it shows that the soil  
 407 becomes nearly saturated on the south slope, but while not on the north slope. This implies that the soil water on the  
 408 south-facing slope has difficulty in to draining because of the presence of more fine grains and the slow pore water  
 409 pressure dissipation. In addition Besides, The stable soil moisture from of Soil Layers No. 2 and 3 for both slopes  
 410 may be attributed to the long dry seasons in the study area, Tand the daily rainfall amount > 30 mm on July 9 and  
 411 23 resulted in an increase in soil moisture increase for all the slope layers.



414  
415 **Fig. 9.** Field monitored volumetric water content: (a) Soil moisture on the south-facing slope, and (b) soil moisture  
416 on the north-facing slope.

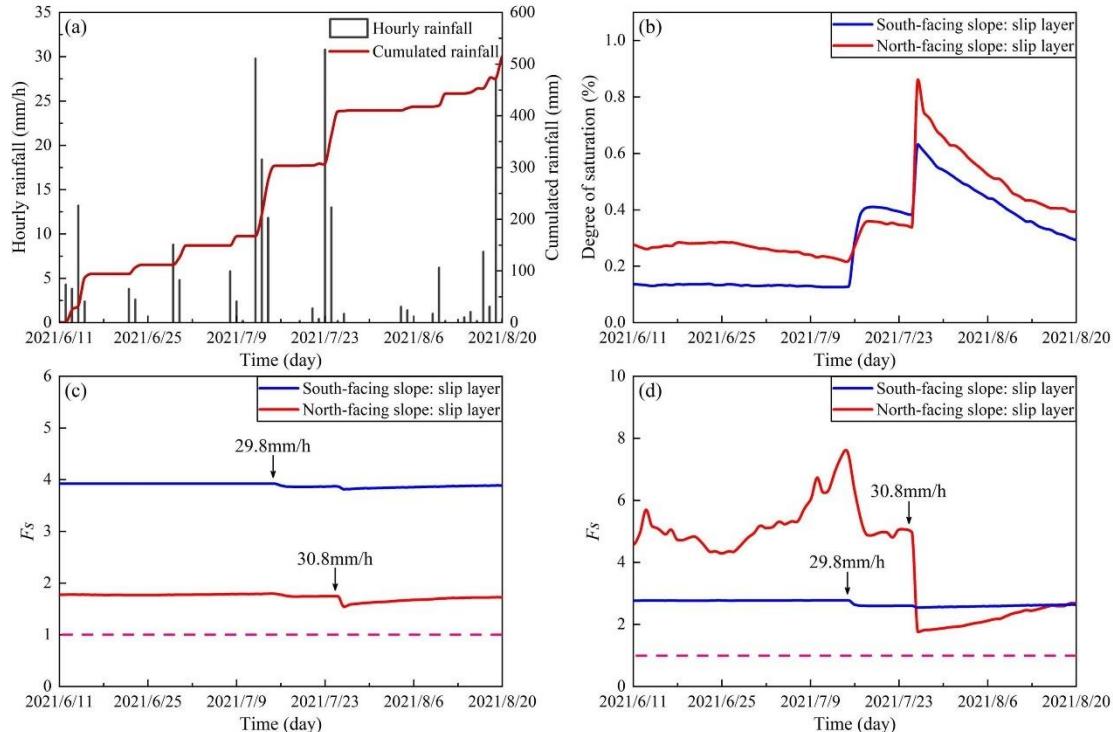


417  
418 **Fig. 10.** Seepage model of slope water storage and drainage. (a) soil water storage, (b) soil water drainage  
419

420 It can be seen from Fig. 10a shows that the storied water of the north- and south-facing slopes diddoes not  
421 synchronously increase with the-accumulated precipitation. When the storied water rapidly increasedds rapidly, the  
422 increase inof the soil water storage of the north-facing slope wais greaterlarger than that of the south-facing slope.  
423 On July 26, a rainfall of 30.8 mm/h was recordedoccurred, and the water storage of the slope reached the peak. It  
424 can be seen that Tthe peak of the water storage onof the north-facing slope was higher than that of the  
425 south-facing slope. However, when the accumulated rainfall tends to be stable, that is, when the rainfall stops for a period  
426 of time, the decline rate of the soil water storage on the north-facing slope is substantiallymuch higher than that on  
427 the south-facing slope. In general, Tthe soil water storage of the south-facing slope wais always higher than that of the  
428 north-facing slope during the rainfall process. DuringIn the process of drainage process, the seepage rate of the  
429 north-facing slope wais greater than that of the south-facing slope (Fig. 10b). Therefore, the south-facing slope hads a better water storage performance, and the north-facing slope hads a higher drainage performance.

#### 431 4.6 Stability fluctuation

432 In this study, the infinite slope model and the finite slope model ~~were~~ used to characterize the sensitivity of  
 433 landslide triggering. ~~in so order as~~ to determine the main mechanism of ~~high overwhelming~~ landslide probability on  
 434 south-facing slopes. The infinite slope model ~~can be used to examine~~ studies the transient stress changes caused by  
 435 water entering the soil, emphasizing the differences ~~in~~ soil permeability (Lu and Likos, 2006; Lu and Godt, 2013).  
 436 The finite slope model focuses on the cohesion of the base surface and lateral periphery of the ground landslide  
 437 source body, as well as the influence ~~of~~ the ~~lateral~~ additional ~~lateral~~ cohesion provided by the vegetation root  
 438 system for the landslide (Schmidt et al., 2001; Dai et al., 2022).



439  
 440 **Fig. 11.** Change ~~in~~ slope stability fluctuation: (a) rainfall records, (b) degree of saturation, (c) stability of finite  
 441 model, ~~and~~ (d) stability of infinite slope model.

442  
 443 Figure 11a shows the rainfall records from June 11 to August 20, 2021. In general, ~~the~~ ~~degree of~~ saturation  
 444 ~~degree~~ of the sliding layer ~~on~~ ~~the~~ south-facing slope was higher than that on the north-facing slope (Fig. 11b). In  
 445 the finite model, the stability of ~~the~~ south-facing slope was always higher than that of ~~the~~ north-facing slope (Fig.  
 446 11c). In the infinite model, the stability of the north-facing slope was generally higher than that of ~~the~~ south-facing  
 447 slope, and the stability of the north-facing slope fluctuated ~~substantially significantly greatly~~ (Fig. 11d). On July 26,  
 448 a rainfall event with ~~a~~ maximum intensity of 30.8 mm/h resulted in ~~a~~ sudden decrease ~~in~~ stability. ~~More importantly~~,  
 449 ~~T~~he estimated stability index of the north-facing slope decreased ~~ds~~ to ~~become~~ lower than that of the south-facing  
 450 slope, ~~and then while~~ increased afterwards. Although the soil moisture of ~~the~~ south-facing slope ~~increased~~  
 451 ~~substantially significantly increased during~~ the rainfall event on July 16, the stability fluctuation was ~~relatively very~~  
 452 ~~small~~. ~~This may which might~~ be related to the relatively strong effective cohesion and smaller pore structure. ~~In~~  
 453 ~~Overall~~, the results of ~~the~~ finite slope model ~~have shown reveal~~ that the south-facing slope has ~~a~~ relatively high  
 454 stability. ~~This which is predominantly mainly attributed~~ to the ~~fact that the~~ effective cohesion of hillslope materials  
 455 on the south-facing slope ~~being is~~ stronger than that of ~~in~~ the north-facing slope ~~although even though~~ the basal area  
 456 of the landslide is more than ~~double twice~~. However, this result is inconsistent with the ~~high overwhelming~~ landslide  
 457 density on ~~the~~ south-facing slopes. ~~The~~ ~~Results of the infinite slope model, Considering the soil characteristic~~  
 458 parameters of the soil moisture ~~characteristic~~ curve, ~~the results of the infinite slope model have shown revealed~~ that

459 the north-facing slope showed a higher level of stability. In the analysis of finite and infinite models, the stability  
460 fluctuation amplitude of the south-facing hillslope was smaller than that of the north-facing hillslope. This indicated,  
461 indicating that the water movement on the south-facing slope was less active than that of the north-facing slope.  
462 Therefore, in the study area, the change in soil stress was more sensitive to the slope stability than the change  
463 in root soil cohesion. It was verified that the change in soil permeability caused by the differential weathering  
464 of the bedrock could be responsible for the aspect-dependent landslide initiation in the study area.

## 465 5 Discussion

466 The landslide strong overwhelming propensity for landslides in some arid environments in the Northern  
467 Hemisphere is are scientifically interesting, and some researchersscholars have highlighted the contribution of plant  
468 roots. ThisSucha finding is to would be expected in the future in other mountain regions, where water is a limitinged  
469 factor for local system sustainability. In the Colorado Frontal range, McGuire et al. (2016) found that the apparent  
470 cohesion supplied by roots was responsible for the observed-connection observed between landslide distribution and  
471 slope aspect (Ebel, 2015; Rengers et al., 2016). In the study area, Li et al. (2021) also found that the plant roots may  
472 explain the observed-connection observed between vegetation cover and landslide probability for the entirewhole  
473 study area. Furthermore, Dai et al. (2022) found that a strong root network and high saturated hydraulic conductivity  
474 may promote the A-S condition of shallow landslides. OnIn the the Loess Plateau in China, some scholars  
475 researchers have observed that the strong overwhelming propensity for shallow landslide initiation is closely relatedds  
476 to the present-day tree density, and plant roots do not penetrate over the failure plane (Guo et al., 2020; Deng et al.,  
477 2022). However, the strong overwhelming propensity for shallow landslides on north- and south-facing slopes  
478 could not't be attributed to the plant roots, becauses the artificial man-made vegetation on both slopes is the  
479 same. Conversely, thesesuch observations could be the result of from the soil hydraulic and mechanical properties  
480 from owing to differential weathering.

481 This study haswork contributeds to the knowledge of know about the effect of differential weathering on the  
482 aspect-dependent landslide initiation fromby the perspective of soil hydraulic properties, in addition to other than  
483 from the mechanical and hydrological effects of plant roots. Except for the strong overwhelming propensity for a  
484 high number ofin landslides number, the shallow landslides on south-facing slopes have exhibited ed relatively larger  
485 areas and greater wider widths than those on the north-facing slopes (Fig. 2). TheIn comparison, the effective  
486 cohesion of the failure zone on the south-facing slope was stronger than that on the north-facing slope. It seems  
487 that The basal area of shallow landslides in the study area may be attributed to the effective cohesion, becauses  
488 some statistical results have shown reveal that incoherent materials favor shallow landslides with no limitation in  
489 size. Meanwhile, cohesive materials favor deep landslides and show a limitation for small sizes (Larsen et al., 2010;  
490 Frattini and Crosta, 2013; Milledge et al., 2014). However, a stronger effective cohesion tends to promote the A-S  
491 conditions of shallow landslides. In other words, A relatively larger up-slope contributing area or steeper gradient  
492 is required to trigger slope failures. In fact, Fig. 3 shows illustrates that some shallow landslides on south-facing  
493 slopes fail on relatively lower upslope contributing areas. Therefore, the soil hydraulic property-related factors,  
494 such as the rising or dissipation of pore water pressure, water storage, and drainage, may contribute to the observed  
495 phenomena observed.

496 The saturated hydraulic conductivities obtained by the by constant water head method and TRIM methods  
497 coincide with each other, which together demonstrates proves that the hillslope material on the north-facing slope has  
498 a relatively larger water infiltration (Tables 1 and 2). However, the results of the stability analysis using the finite  
499 and infinite models imply that the failure potential of slides on a north-facing slope is relatively lower than that on  
500 a south-facing slope, although the stability index fluctuates more heavily than the north-facing slope. Thesesuch  
501 differences imply that slope failures on a north-facing slope may only occur only under intensive rainfall

502 ~~conditionson condition of intensive rainfall merely,~~ or by a combination of prolonged antecedent precipitation and  
503 short-duration intensive rainfall. For potential failures on south-facing slopes, the combination of prolonged  
504 antecedent precipitation and short-duration intensive rainfall should be ~~a potential the possible~~ trigger ~~owing~~<sup>due</sup> to  
505 the low hydraulic conductivity and pore water pressure dissipation. ~~Additionally, This~~ ~~study~~ ~~work~~ ~~mainly~~ highlights  
506 the role of hydraulic properties ~~on the~~ landslide occurrence. ~~Alt~~<sup>T</sup>hough the south- and north-facing slopes are ~~merely~~  
507 underlain by granite, the physical properties of hillslope materials, such as ~~the~~ excessive pore water pressure, strength  
508 of sliding mass, soil water storage, and leakage, ~~are significantly different~~<sup>differentiates a lot</sup>. ~~Such a~~ ~~This~~ finding  
509 cannot be random because the study area ~~has been~~<sup>is</sup> selected on ~~the~~ condition that it is ~~relatively~~ far from the northern  
510 and eastern areas, where local soils are ~~predominantly~~<sup>mainly</sup> from Loess deposits, and the study areas of Li et al.  
511 (2021) and Dai (2022), where the bedrock underneath differs ~~substantially~~<sup>greatly</sup>. ~~Finally, This~~ main purpose of  
512 this work ~~is merely to~~ elucidates the reason ~~for~~ ~~the~~ aspect-dependent landslide initiation ~~from~~<sup>by</sup> the perspective of  
513 soil hydraulic properties. ~~These~~ ~~Such~~ differences ~~essentially~~ results from ~~the~~ differential weathering owing to the  
514 amount of direct sunlight. Other mechanics, such as numerical or relative dating methods ~~and~~, preferential flow in  
515 ~~the~~ macro-pore distribution, could provide new evidence ~~for~~ such observations.

## 516 6 Conclusion

517 Previous researches ~~on about~~ the ~~strong overwhelming~~ propensity ~~for~~ shallow landslides on south-facing  
518 slopes over north-facing slopes ~~has~~ highlighted the role of plant roots. In a localized area with ~~the~~ same vegetation,  
519 ~~including especially the~~ plant roots, ~~they do does~~ not penetrate ~~the~~ over failure layer. ~~These~~ ~~such~~ observations  
520 ~~cannot could~~ ~~be~~ attributed to plant roots and may result from the differential weathering of bedrock under the  
521 influence of hydrothermal conditions. In this ~~study~~ ~~work~~, we jointly explained the influence of bedrock weathering  
522 on soil hydraulic properties from physical and mechanical properties, pore water pressure, unsaturated hydraulic  
523 conductivity, water storage and drainage, and slope stability fluctuation during monitoring, and studied ~~the~~ landslide  
524 initiation related to slope direction. The following conclusions ~~were an~~ be drawn:

525 (1) In terms of soil physical and mechanical properties on both slopes, the soil masses on ~~the~~ south-facing slope  
526 ~~were~~ rich in clay content, ~~whereas while~~ the soil mass on ~~the~~ north-facing slope ~~had~~ ~~relatively~~ high sand ~~content~~.  
527 The effective cohesion of ~~in~~ ~~the~~ soil mass on ~~the~~ south-facing slope ~~was~~ is higher than that on ~~the~~ north-facing slope,  
528 while the effective frictional angle ~~was~~ is smaller.

529 (2) ~~The r~~Results of the GDS tests ~~showed revealed~~ that the dissipation rate of pore water pressure for soil mass  
530 on the south-facing slope ~~was~~ ~~substantially~~<sup>much</sup> lower than that ~~on~~ in the north-facing slope. Higher effective  
531 cohesion and ~~the~~ slower pore water pressure dissipation may result in ~~at the~~ larger basal area ~~for~~ shallow landslides  
532 on south-facing slopes.

533 (3) The soil mass on ~~the~~ south-facing slope ~~had~~ ~~a~~ higher residual water content and air-entry pressure, and ~~a~~  
534 lower saturated hydraulic conductivity than that ~~of~~ on the north-facing slope. For ~~the~~ water storage and drainage  
535 performance, the storied water ~~from~~ of the south-facing slope ~~was~~ is higher than that of the north-facing slope, while  
536 the north-facing slope ~~had~~ ~~a~~ higher leakage rate. ~~The r~~Results of the stability analysis ~~based on the~~ ~~on~~ basis of finite  
537 and infinite models ~~show illustrate~~ that the infinite slope model may be suitable for elucidating ~~the~~ aspect-dependent  
538 landslide distribution in the study area.

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544 **Code/Data availability**

545 The raw/processed data in this work cannot be shared at this time, ~~becauseas~~ the data also forms part of an ongoing  
546 study.

547 **Author contributions**

548 Professor Ma Chao found ~~a strong the overwhelming~~ propensity ~~for~~ shallow landslide initiation on south-facing  
549 hillslopes in the study area and launched ~~at~~ the research proposal. Miss Yanglin Guo ~~completed~~~~finished~~ the sampling  
550 collection and indoor tests.

551 **Competing interests**

552 All authors have declared that there were no conflicts of interests and competing interests.

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