



Empirical tsunami fragility modelling for hierarchical damage levels: An application to damage data of the 2009 South Pacific tsunami

Fatemeh Jalayer¹, Hossein Ebrahimian¹, Konstantinos Trevlopoulos¹, Brendon Bradley²

⁵ ¹Department of Structures for Engineering and Architecture, University of Naples Federico II, Naples 80125, Italy ²Department of Civil and Natural Resources Engineering, University of Canterbury, Private Bag 4800, Christchurch 8140, New Zealand

Correspondence to: Fatemeh Jalayer (fatemeh.jalayer@unina.it)

Abstract.

- 10 The present work proposes a simulation-based Bayesian method for parameter estimation and fragility model selection for mutually exclusive, and collectively exhaustive (MECE) damage states. This method uses adaptive Markov chain Monte Carlo simulation (MCMC) based on likelihood estimation using point-wise intensity values. It identifies the simplest model that fits the data best, among the set of viable fragility models considered. As a case-study, observed pairs of data for tsunami intensity and
- 15 corresponding damage level from the central South Pacific tsunami on September 29, 2009, are used. The tsunami was triggered by an unprecedented earthquake doublet (Mw 8.1 and Mw 8.0) and seriously impacted numerous locations in the central South Pacific. Damage data related to 120 brick masonry residential buildings in American Samoa and Samoa islands were utilized. A six-tier damage scale was considered, using tsunami flow depth as the intensity measure.
- 20

Keywords: probabilistic tsunami risk assessment, tsunami fragility, Bayesian inference, model class selection

1 Introduction

Fragility models express the probability of exceeding certain damage thresholds for a given level of intensity for a specific class of buildings or infrastructure. Empirical fragility curves are models derived based on observed pairs of damage and intensity data for buildings and infrastructures usually collected, acquired, and even partially simulated in the aftermath of disastrous events. Some examples of empirical fragility models are: seismic fragility (Rota et al. 2009, Rosti et al. 2021), tsunami fragility (Koshimura et al. 2009a, Reese et al. 2011; a comprehensive review can be found in Charvet et al. 2017), flooding

- 30 fragility (Wing et al. 2020), and debris flow fragility curves (Eidsvig et al. 2014). Empirical fragility modelling is greatly affected by how the damage and intensity parameters are defined. Mutually exclusive and collectively exhaustive (MECE, see next section for the definition) damage states are quite common in the literature as discrete physical damage states. The MECE condition is necessary for damage states in most probabilistic risk formulations leading to the mean rate of exceeding loss
- 35 (e.g., Behrens et al. 2021). Tsunami fragility curves usually employ the tsunami flow depth as the measure of intensity; although different studies use also other measures like current velocity (e.g., De Risi et al. 2017b, Charvet et al. 2015). Charvet et al. (2015) demonstrate that the flow depth may cease to be an appropriate measure of intensity for higher damage states and other parameters such as the current velocity, debris impact, and
- 40 scour can become increasingly more important. De Risi et al. (2017b) developed bivariate tsunami fragilities, which account for the interaction between the two intensity measures, tsunami flow depth





and current velocity.

Early procedures for empirical tsunami fragility curves used data binning for representing the intensity. For example, Koshimura et al. (2009b) binned the observations by the intensity measure, i.e., the flow

- 45 depth, however the latest procedures have mostly used point-wise intensity estimates instead. Fragility curves for MECE damage states are distinguished by their nicely "laminar" shape; in other words, the curves should not intersect. When fitting empirical fragility curves to observed damage data, this condition is not satisfied automatically. For example, fragility curves are usually fitted for individual damage states separately and they are filtered afterwards to remove the crossing fragility
- 50 curves (e.g., Miano et al. 2020) or ordered ("parallel") fragility models are used from the start (Charvet et al. 2014, Lahcene et al. 2021). Charvet et al. (2014) and De Risi (2017a) also used partially ordered models to derive fragility curves for MECE damage states. They used the multinomial probability distribution to model the probability of being in any of MECE damage states based on binned intensity representation. De Risi et al. (2017a) used Bayesian inference to derive the model parameters for an
- 55 ensemble of fragility curves. Empirical tsunami fragility curves are usually constructed using generalized linear models based on probit, logit, or the complementary loglog link functions (Charvet et al. 2014, Lahcene et al. 2021). As far as the assessment of the goodness of fit, model comparison and selection are concerned, approaches based on the likelihood ratio and Akaike Information Criterion, (e.g., Charvet et al. 2014, Lahcene et al.
- al. 2021) and on k-fold cross validation have also been used (Chua et al. 2021). For estimating confidence intervals for empirical tsunami fragility curves, bootstrap resampling has been commonly used (Charvet et al. 2014, Lahcene et al. 2021, Chua et al. 2021).
 The present paper presents a simulation-based Bayesian method for inference and model class selection for the ensemble modelling of the tsunami fragility curves for MECE damage states for a given class
- 65 of buildings. By fitting the (positive definite) fragility link function to the conditional probability of being in a certain damage state, given that building is not in any of the preceding states, the method ensures that the fragility curves do not cross (i.e., they are "hierarchical" as in De Risi et al. 2017a). The method uses adaptive Markov Chain Monte Carlo Simulation (MCMC, Beck and Au 2002), based on likelihood estimation using point-wise intensity values, to infer the ensemble of the fragility model
- 70 parameters. Alternative link functions are compared based on log evidence which considers both the average goodness of fit (based on log likelihood) and the model parsimony (based on relative entropy). This way, among the set of viable models considered, it identifies the simplest model that fits the data best. The main advantage is that the method provides, within the same set of runs, consistent parameter estimations for all the damage states, estimates confidence intervals, and identifies the best fragility
- 75 model class among the pool of models. Although the application is demonstrated for the observed damage and effects of the South Pacific 2009 Tsunami, the method is quite general and transferable to other contexts and hazards. The whole procedure is provided as an open-source software on the site of the European Tsunami Risk Service (<u>https://eurotsunamirisk.org/software/</u>) and is also available as a standalone docker application.
- 80

2 Methodology

2.1 Definitions of intensity and damage parameters

The intensity measure, IM, (or simply "intensity"; e.g., the tsunami flow depth) refers to a parameter used to convey information about an event from the hazard level to the fragility level –it is an intermediate variable. The damage parameter, D, is a discrete random variable and the vector of damage





levels is expressed as $\{D_j, j=0:N_{DS}\}$, where D_j as the j^{th} damage level (threshold) and N_{DS} as the total number of damage levels considered (depending on the damage scale being used and on the type of hazard, e.g., earthquake, tsunami, debris flow). Normally, D_0 denotes the *no-damage* threshold, while $D_{N_{DS}}$ defines the total collapse or being totally washed away. Let us assume that DS_j is the j^{th} damage

90 *state* defined by the logical statement that the damage *D* is between the two damage thresholds D_j and D_{j+1} ; i.e., *D* is equal to or greater than D_j and smaller than D_{j+1} as follows (see also Figure 1 for a graphical representation of the above expressions):

$$DS_{j} \equiv \left(D \ge D_{j}\right) \cdot \left(D < D_{j+1}\right) \tag{1}$$

where (·) denotes the logical product and is read as "AND". Obviously, for the last damage state, we have $DS_{N_{DS}} \equiv D \ge D_{N_{DS}}$.

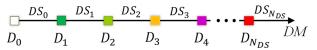


Figure 1: Graphical representation of damage levels *D_j* and damage states *DS_j*, where *j*=0:*N_{DS}*

Damage states $\{DS_0, DS_1, ..., DS_{N_{DS}}\}$ are mutually exclusive and collectively exhaustive (MECE) if an 100 only if $P(DS_i \cdot DS_j | IM) = 0$ (if $i \neq j, j = 0$: N_{DS}) and $\sum_{j=0}^{N_{DS}} P(DS_j | IM) = 1$; (·) denotes the logical product and is read as "AND". In simple words, the damage states are MECE if being in one damage state excludes all others and if all the damage states together cover the entire range of possibilities in terms of damage.

2.2 Fragility modelling using generalized regression models

105 The term $P(DS_j|IM)$ denotes the probability of being in damage state DS_j for a given intensity level *IM*. Based on N_{DS} damage thresholds, This conditional probability $P(DS_j|IM)$ can be read (see Equation 1) as the probability that $(D \ge D_j)$ and $(D < D_{j+1})$, and can be estimated as follows (see Appendix A for the derivation):

$$P(DS_{j}|IM) = P[(D \ge D_{j}) \cdot (D < D_{j+1})|IM]$$

$$= \begin{cases} P(D \ge D_{j}|IM) - P(D \ge D_{j+1}|IM) & \text{for } 0 \le j < N_{DS} \\ P(D \ge D_{j}|IM) & \text{for } j = N_{DS} \end{cases}$$
(2)

110 where P(D ≥ D_j|IM) is the fragility function for damage level D_j. For each damage threshold, fragility can be obtained for a desired building class considering that the damage data provides Bernoulli variables (binary values) of whether the considered damage level was exceeded or not for given IM levels. For damage threshold D_j, all buildings with an observed damage level D < D_j will have a probability equal to zero, while those with D ≥ D_j will have an assigned probability equal to one. In other words, for building i and damage state j, the Bernoulli variable Y_{ij}

indicates whether building *i* is in damage state *j*:

$$Y_{ij} = \begin{cases} 1 & \text{if building } i \text{ exceeds } D_j & \text{with probability } P(D \ge D_j | IM_i) \\ 0 & \text{if building } i \text{ does not exceed } D_j & \text{with probability } 1 - P(D < D_j | IM_i) \end{cases}$$
(3)





where IM_i is the intensity evaluated at the location of building *i*. A Bernoulli variable is defined by one parameter which is $P(D_j|IM_i)$ herein. This latter is usually linked to a linear logarithmic predictor in the form:

$$l_{ij} = \alpha_{0,j} + \alpha_{1,j} \ln IM_i \tag{4}$$

where $\alpha_{0,j}$ and $\alpha_{1,j}$ are regression constants for damage level *j*. We have employed generalized linear regression (e.g., Agresti 2012) with different link functions "logit", "probit", and "cloglog", to define probability function π_{ij} as following:

125
$$\pi_{ij} = \pi_j \left(IM_i \right) = \begin{cases} \left(1 + \exp(-l_{ij}) \right)^{-1} & \text{logit} \\ \Phi(l_{ij}) & \text{probit} \\ 1 - \exp(-\exp(l_{ij})) & \text{cloglog} \end{cases}$$
(5)

The *logit* link function is equivalent to presenting $\pi_j(IM)$ with a Logistic regression function. The *probit* link function is equivalent to a lognormal cumulative distribution function for $\pi_j(IM)$. In the *cloglog* (complementary log-log) transformation, the link function at the location of building *i* can be expressed as $l_{ij} = \ln[-\ln(1 - \pi_{ij})]$. It is noted that the generalized linear regression based on

- 130 maximum likelihood estimation (MLE) is available in many statistical software packages (e.g., MathWorks, Python, R). In the following, we have referred to the general methodology of fitting fragility model to data –one damage state at a time— the "*Basic method*". In the Basic method, the probability of exceeding damage level *j* is equal to the probability function defined in Equation 5; that is, $\pi_{ij} = P(D \ge D_j | IM_i)$. This method for empirical fragility curve parameter estimation is addressed
- in detail in the Section "*Results*", under "MLE-*Basic*" method. The fragility curves obtained under the "MLE-*Basic*" method could potentially cross, leading to the ill condition that $P(DS_j|IM) < 0$. To overcome this, a *hierarchical fragility modeling approach* has been adopted like that in De Risi et al. (2017a).

2.3 Hierarchical fragility modelling

Equation (2) for $0 \le j < N_{DS}$, and given IM_i , can also be written as follows using the product rule in probability:

$$P(DS_{j}|IM_{i}) = P[(D < D_{j+1}) \cdot (D \ge D_{j})|IM_{i}]$$

=
$$\left[1 - P(D \ge D_{j+1}|D \ge D_{j}, IM_{i})\right] \cdot P(D \ge D_{j}|IM_{i})$$
(6)

The term $P(D \ge D_{j+1} | D \ge D_j, IM_i)$ embedded in Equation (6) denotes the conditional probability that the damage exceeds the damage threshold D_{j+1} knowing that it has already exceeded the previous

145 damage level D_j given IM_i. By making $\pi_{ij} = P(D \ge D_{j+1}|D \ge D_j, IM_i)$ (see Equation 5, which is positive definite), we ensure that the fragility curve of a lower damage level will not fall below the fragility curve of the subsequent damage threshold (the ill condition of $P(DS_j|IM) < 0$ does not take place). Hence, Equation (6) can be expanded as follows (see Appendix B for derivation):

$$P(DS_{j}|IM_{i}) = (1 - \pi_{ij}) \cdot \left[1 - \sum_{k=0}^{j-1} P(DS_{k}|IM_{i})\right]$$
(7)





150 In this way, the fragility curves are constructed in a hierarchical manner by first constructing the "fragility increments" $P(DS_j|IM_i)$ starting from j=0. Note that for the last damage state $DS_{N_{DS}}$, the probability $P(DS_{N_{DS}}|IM)$, which is also equal to the fragility of the ultimate damage threshold $D_{N_{DS}}$, i.e. $P(D \ge D_{N_{DS}}|IM)$ (see Equation 2), can be estimated by satisfying the CE condition:

$$P(DS_{N_{DS}}|IM_{i}) = P(D \ge D_{N_{DS}}|IM_{i}) = 1 - \sum_{j=0}^{N_{DS}-1} P(DS_{j}|IM_{i})$$
(8)

155 Accordingly, the fragility for other damage levels $P(D \ge D_j | IM_i)$, where $0 < j < N_{DS}$, can be obtained from Equation (2) by starting from the fragility of the higher threshold $P(D \ge D_{j+1} | IM)$, and adding successively $P(DS_i | IM)$ (see Equation 7) as follows:

$$P\left(D \ge D_{j} \left| IM_{i} \right) = P\left(DS_{j} \left| IM_{i} \right) + P\left(D \ge D_{j+1} \left| IM_{i} \right) \quad \text{for } 0 \le j < N_{DS} \right)$$

$$\tag{9}$$

- As a result, the set of hierarchical fragility models based on Equation (9) has $2 \times N_{DS}$ model parameters with the vector $\mathbf{\theta} = [\{\alpha_{0,j}, \alpha_{1,j}\}, j = 0: N_{DS} - 1]$. Obviously, with reference to Equation (8), no model parameter is required for the last damage level which is derived by satisfying the CE condition. The vector $\mathbf{\theta}$ of the proposed hierarchical fragility models can be defined by two different approaches:
 - 1) *MLE method*: a generalized linear regression model (as explained in previous section) is used for the conditional fragility term $\pi_{ij} = P(D \ge D_{j+1} | D \ge D_j, IM)$ for the *j*th damage state DS_j
- 165 (see Equation 7, $0 \le j < N_{DS}$). Herein, we need to work with partial damage data so that all buildings in DS_j (with an observed damage $D_j \le D < D_{j+1}$) will be assigned a probability equal to zero, while those in higher damage states (with $D \ge D_{j+1}$) will be assigned a probability equal to one (i.e., in order to model the conditioning on $D \ge D_j$, the domain of possible damage levels is reduced to $D \ge D_j$).
- 170 2) *Bayesian model class selection (BMCS)*: employing the Bayesian inference for model updating to obtain the joint distribution of the model parameters.

Detailed discussion about these two approaches, namely MLE and BMCS, for parameters estimation of empirical fragility curves are provided in Section "*Results*".

2.4 Bayesian model class selection (BMCS) and parameter inference using adaptive 175 MCMC

We use the Bayesian model class selection (BMCS) herein to identify the best link model to use in the generalized linear regression scheme. However, the procedure is general and can be applied to a more diverse pool of candidate fragility models. BMCS (or model comparison) is essentially Bayesian updating at the model class level to make comparisons among candidate model classes given the observed data (e.g., Beck and Yuen 2004, Muto and Beck 2008). Given a set of $N_{\rm M}$ candidate model classes $\{\mathbb{M}_k, k = 1: N_{\rm M}\}$, and in the presence of the data **D**, the posterior probability of the $k^{\rm th}$ model class, denoted as $P(\mathbb{M}_k | \mathbf{D})$ can be written as follows:

$$P(\mathbf{M}_{k}|\mathbf{D}) = \frac{p(\mathbf{D}|\mathbf{M}_{k})P(\mathbf{M}_{k})}{\sum_{k=1}^{N_{\mathrm{M}}} p(\mathbf{D}|\mathbf{M}_{k})P(\mathbf{M}_{k})}$$
(10)

In lieu of any initial preferences about the prior $P(\mathbb{M}_k)$, one can assign equal weights to each model; 185 thus, $P(\mathbb{M}_k) = 1/N_{\mathbb{M}}$. Hence, the probability of a model class is dominated by the likelihood $p(\mathbf{D}|\mathbb{M}_k)$ (a.k.a. *evidence*). It is to note that *p* herein stands for the probability density function (PDF). Here data





vector $\mathbf{D} = \{(IM, DS)_i, i = 1: N_{CL}\}$ defines the observed intensity and damage data for N_{CL} buildings surveyed for class *CL*. In this paper, we are considering a mono-class portfolio of buildings. Let us define the vector of model parameters $\boldsymbol{\theta}_k$ for model class \mathbb{M}_k as $\boldsymbol{\theta}_k = \left[\left\{\alpha_{0,j}, \alpha_{1,j}\right\}_k, j = 0: N_{DS} - 1\right]$.

190 We use the Bayes theorem to write the "evidence" $p(\mathbf{D}|\mathbb{M}_k)$ provided by data **D** for model \mathbb{M}_k as follows:

$$p(\mathbf{D}|\mathbf{M}_{k}) = \frac{p(\mathbf{D}|\boldsymbol{\theta}_{k},\mathbf{M}_{k})p(\boldsymbol{\theta}_{k}|\mathbf{M}_{k})}{p(\boldsymbol{\theta}_{k}|\mathbf{D},\mathbf{M}_{k})}$$
(11)

It can be shown (see Appendix C, Muto and Beck 2008) that logarithm of the evidence (called *log-evidence*) $\ln[p(\mathbf{D}|\mathbb{M}_k)]$ can be written as:

$$\ln\left[p(\mathbf{D}|\mathbf{M}_{k})\right] = \int_{\Omega_{\boldsymbol{\theta}_{k}}} \ln\left[p(\mathbf{D}|\boldsymbol{\theta}_{k},\mathbf{M}_{k})\right] p(\boldsymbol{\theta}_{k}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}_{k}$$

$$- \int_{\Omega_{\boldsymbol{\theta}_{k}}} \ln\left[\frac{p(\boldsymbol{\theta}_{k}|\mathbf{D},\mathbf{M}_{k})}{p(\boldsymbol{\theta}_{k}|\mathbf{M}_{k})}\right] p(\boldsymbol{\theta}_{k}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}_{k}$$

$$(12)$$

$$Term 2$$

195

where Ω_{θ_k} is the domain of θ_k , and $p(\mathbf{D}|\theta_k, \mathbb{M}_k)$ is the likelihood function conditioned on model class \mathbb{M}_k . "*Term* 1" denotes the posterior mean of the log-likelihood, which is a measure of the average data fit to model \mathbb{M}_k ."*Term* 2" is the relative entropy (Kullback and Leibler 1959, Cover and Thomas 1991) between the prior $p(\theta_k|\mathbb{M}_k)$ and the posterior $p(\theta_k|\mathbf{D}, \mathbb{M}_k)$ of θ_k given model \mathbb{M}_k , which is a measure

of the distance between the two PDFs. The latter *Term* 2 measures quantitatively the amount of information (on average) that is "gained" about θ_k from the observed data **D**. It is interesting that *Term* 2 in the log-evidence expression penalizes for model complexity; i.e., if the model extracts more information from data (which is a sign of being a complex model with more model parameters), the log-evidence reduces. The exponential of the log-evidence, p(**D**|M_k), is going to be implemented directly in Equation (10), to provide the probability attributed to the model class M_k. More details on

how to estimate the two terms in Equation (12) are provided in the Section "*Results*". The likelihood $p(\mathbf{D}|\boldsymbol{\theta}_k, \mathbb{M}_k)$ can be derived, based on point-wise intensity information, as the likelihood of $n_{CL,j}$ buildings being in damage state DS_j (considering that $\sum_{j=0}^{N_{DS}} n_{CL,j} = N_{CL}$), according to data **D** defined before:

210
$$p(\mathbf{D}|\boldsymbol{\theta}_{k}, \mathbb{M}_{k}) = \prod_{j=0}^{N_{DS}} \prod_{i=1}^{n_{CL,j}} P(DS_{j}|IM_{i})$$
(13)

The posterior distribution $p(\boldsymbol{\theta}_k | \mathbf{D}, \mathbf{M}_k)$ can be found based on Bayesian inference:

$$\underbrace{p(\mathbf{\theta}_{k} | \mathbf{D}, \mathbb{M}_{k})}_{\text{posterior}} = \frac{p(\mathbf{D} | \mathbf{\theta}_{k}, \mathbb{M}_{k}) p(\mathbf{\theta}_{k} | \mathbb{M}_{k})}{\int_{\Omega_{\mathbf{\theta}_{k}}} p(\mathbf{D} | \mathbf{\theta}_{k}, \mathbb{M}_{k}) p(\mathbf{\theta}_{k} | \mathbb{M}_{k}) d\mathbf{\theta}_{k}} = C^{-1} \underbrace{p(\mathbf{D} | \mathbf{\theta}_{k}, \mathbb{M}_{k})}_{\text{likelihood}} \underbrace{p(\mathbf{\theta}_{k} | \mathbb{M}_{k})}_{\text{prior}}$$
(14)

where C^{-1} is a normalizing constant. In lieu of additional information (or preferences), the prior distribution, $p(\theta_k | \mathbb{M}_k)$, can be estimated as the product of marginal normal/lognormal PDFs for each model parameter, i.e., a multivariate normal/lognormal distribution with zero correlation between the pairs of model parameters θ_k (see Appendix D). More detail about an efficient prior joint PDF is provided in the Section "*Results*". To sample from the posterior distribution $p(\theta_k | \mathbf{D}, \mathbb{M}_k)$ in Equation (15), an *adaptive* MCMC simulation routine is employed. MCMC is particularly useful for drawing





samples from the target posterior, while it is known up to a scaling constant C^{-1} (see Beck and Au 2002); thus, in Equation (14), we only need un-normalized PDFs to feed the MCMC procedure. The MCMC routine herein employs the Metropolis-Hastings (MH) algorithm (Metropolis et al. 1953, Hasting 1970) to generate samples from the target joint posterior PDF $p(\theta_k | \mathbf{D}, \mathbb{M}_k)$.

2.5 Calculating the hierarchical fragilities and the corresponding confidence intervals based on the model parameters θ_k

- 225 For each realization of the vector of model parameters $\mathbf{\theta}_k$, the corresponding set of hierarchical fragility curves can be derived based on the procedure described in the previous sections. Since we have N_d realizations of the model parameters drawn from the joint PDF $p(\mathbf{\theta}_k | \mathbf{D}, \mathbf{M}_k)$ (where N_d is the number of distinct samples from adaptive MCMC procedure, see also Appendix E), we can use the concept of *Robust Fragility* (RF) proposed in Jalayer et al. 2017 (see also Jalayer et al. 2015, and Jalayer and
- Erahimian 2020) to derive confidence intervals for the fragility curves. RF is defined as the expected value for a prescribed fragility model considering the joint probability distribution for the fragility model parameters $\boldsymbol{\theta}_k$. The RF herein can be expressed as:

$$P(D \ge D_j | IM, \mathbf{D}, \mathbb{M}_k) = \int_{\Omega_{\mathbf{\theta}_k}} P(D \ge D_j | IM, \mathbf{\theta}_k) p(\mathbf{\theta}_k | \mathbf{D}, \mathbb{M}_k) d\mathbf{\theta} = \mathbb{E}_{\mathbf{\theta}_k | \mathbf{D}, \mathbb{M}_k} \left[P(D \ge D_j | IM, \mathbf{\theta}_k) \right]$$
(15)

where $P(D \ge D_j | IM, \mathbf{\theta}_k)$ is the fragility given the model parameters $\mathbf{\theta}_k$ associated with the model \mathbb{M}_k

235 (it has been assumed that once conditioned on fragility model parameters $\boldsymbol{\theta}$, the fragility becomes independent of data **D**); $\mathbb{E}_{\boldsymbol{\theta}_k | \mathbf{D}, \mathbb{M}_k}$ is the expected value over the vector of fragility parameters $\boldsymbol{\theta}_k$ for model \mathbb{M}_k . The integral in Equation (15) can be solved numerically by employing Monte Carlo simulation with N_d simulations of the vector $\boldsymbol{\theta}_k$ as follows:

$$P(D \ge D_j | IM, \mathbf{D}, \mathbb{M}_k) \cong \frac{1}{N_d} \sum_{l=1}^{N_d} P(D \ge D_j | IM, \mathbf{\theta}_{k,l})$$
(16)

240 where $P(D \ge D_j | IM, \mathbf{\theta}_{k,l})$ is the fragility given the l^{th} realization of the model parameters $\mathbf{\theta}_k$ for model \mathbb{M}_k . Based on the definition represented in Equation (15) and Equation (16), the variance $\sigma_{\mathbf{\theta}_k | \mathbf{D}, \mathbb{M}_k}^2$, which can be used to estimate a confidence interval for the fragility considering the uncertainty in the estimation of $\mathbf{\theta}_k$, is calculated as follows:

$$\sigma_{\boldsymbol{\theta}_{k}|\mathbf{D},\mathbb{M}_{k}}^{2}\left[P\left(D \ge D_{j}\left|IM,\boldsymbol{\theta}_{k}\right.\right)\right] = \underbrace{\mathbb{E}_{\boldsymbol{\theta}_{k}|\mathbf{D},\mathbb{M}_{k}}\left[P\left(D \ge D_{j}\left|IM,\boldsymbol{\theta}_{k}\right.\right)^{2}\right]}_{\stackrel{=}{=}\frac{1}{N_{d}}\sum_{i=1}^{N_{d}}P\left(D \ge D_{j}\left|IM,\boldsymbol{\theta}_{k}\right.\right)^{2}} - \underbrace{\left(\mathbb{E}_{\boldsymbol{\theta}_{k}|\mathbf{D},\mathbb{M}_{k}}\left[P\left(D \ge D_{j}\left|IM,\boldsymbol{\theta}_{k}\right.\right)\right]\right)^{2}}_{=P\left(D \ge D_{j}\left|IM,\mathbf{D},\mathbb{M}_{k}\right.\right)^{2} (Eq.16)}$$
(17)

- 245 The empirical fragilities derived through the hierarchical fragility procedure are not necessarily attributed to a lognormal distribution. Hence, we have derived equivalent lognormal statistics (i.e., the median and dispersion) for the resulting fragility curves. The median intensity, η_{IM_c} , for a given damage level, is calculated as the *IM* corresponding to 50% probability on the fragility curve. The logarithmic standard deviation (dispersion) of the equivalent lognormal fragility curve at the onset of damage
- 250 threshold, β_{IM_c} , is estimated as half of the logarithmic distance between the *IMs* corresponding to the probabilities of 16% (IM_c^{16}) and the 84% (IM_c^{84}) on the fragility curve; thus, the dispersion can be estimated as $\beta_{IM_c} = 0.50 \times \ln(IM_c^{84}/IM_c^{16})$. The overall effect of epistemic uncertainties (due to the uncertainty in the fragility model parameters) on the median of the empirical fragility curve is considered through (logarithmic) intensity-based standard deviation denoted as β_{UF} (see Jalayer et al.
- 255 2020). β_{UF} can be estimated as half of the (natural) logarithmic distance (along the *IM* axis) between





the RF curves derived with a 16% confidence level (denoted as IM⁸⁴) and 84% confidence level (IM¹⁶), respectively; i.e., $\beta_{UF} = 0.50 \times \ln(IM^{84}/IM^{16})$. The RF and its confidence band, the sample fragilities $\boldsymbol{\theta}_{k,l}$ (where $l = 1: N_d$), the equivalent lognormal parameters of the RF η_{IM_c} and β_{IM_c} , the epistemic uncertainty β_{UF} , and finally the intensities IM^{16} and IM^{84} are shown in Figure 2d to Figure 4d in the 260 following Section 3.

Results 3

3.1 Case Study: The 2009 South Pacific Tsunami

The central South Pacific region-wide tsunami was triggered by an unprecedented earthquake doublet (Mw 8.1 and Mw 8.0) on September 29, 2009, between about 17:48 and 17:50 UTC (Goff and 265 Dominey-Howes 2009). The tsunami seriously impacted numerous locations in the central South Pacific. Herein, the damage data related to the masonry residential buildings associated with the reconnaissance survey sites of American Samoa and Samoa islands were utilized as a proof of concept. Out of N_{CL} =120 surveyed buildings in the class of masonry residential, 84 were in American Samoa, and 36 in Samoa. Based on the observed damage regarding different indicators (see Reese et al. 2011

270 for more details on damage observation), each structure was assigned a damage state between (DS_0 and DS_5). The original data documented in Reese et al. (2011) reporting the tsunami flow depth and the attributed damage state to each surveyed building can be found on the site of the European Tsunami Risk Service (https://eurotsunamirisk.org/datasets/, reported as Class 1: brick masonry residential). The five damage thresholds ($N_{DS} = 5$) and a description of the indicators leading to the classification of the damage state are given in Table 1 based on Peece et al. (2011)

275	damage state are	given in	Table I	based on I	Reese et al.	(2011).
-----	------------------	----------	---------	------------	--------------	---------

Dama	ge threshold	Damage level description	Number of masonry buildings $n_{CL,i}$	Flow depth range (m)*
D_0	None	no damage	9	[0.01-0.50]
D_1	Light	non-structural damage	3	[0.30-0.80]
D_2	Minor	significant non-structural damage, minor structural damage	23	[0.40-2.00]
D_3	Moderate	significant structural and non-structural damage	24	[0.90-2.70]
D_4	Severe	irreparable structural damage, will require demolition	21	[0.96-3.07]
D_5	Collapse	complete structural collapse	40	[1.00-5.35]
	-	_	$N_{CL} = 120$	-

Table 1. The classification of damage thresholds used in this study and the observed damage data associated with residential masonry buildings (from Reese et al. 2011).

* [min-max] values (in meters) associated with each damage state.

The fourth column in Table 1 illustrates the distribution of data for masonry residential building class 280 surveyed based on the observed damage level $(n_{CL,j}, j = 0; N_{DS})$, with the total sum of $N_{CL} = 120$ buildings surveyed for this class). The last column shows the range of the flow depth associated with each damage state.

3.2 The different model classes

We have considered the set of candidate models consisting of the fragility models resulting from the three alternative link functions used in the generalized linear regression in Equation 5. That is, M_1 refers to hierarchical fragility modelling based on "probit"; M2 refers to hierarchical fragility modelling based on "logit"; M₃ refers to hierarchical fragility modelling based on "cloglog". For each model, both the





MLE method using the MATLAB generalized regression toolbox and the Bayesian inference using the procedure described in the previous section are implemented.

290 3.3 Fragility modelling using MLE

The first step towards calculating the fragilities by employing the MLE method is to define the vector of model parameters $\mathbf{\theta} = \{\alpha_{0,j}, \alpha_{1,j}\}$, where j = 0: $N_{DS} - 1 = 4$. To accomplish this, the *j*th pair of the model parameter $\{\alpha_{0,j}, \alpha_{1,j}\}$ is obtained by fitting the link functions in Equation (5) to conditional fragility $P(D \ge D_{j+1}|D \ge D_j, IM)$ according to Equation (7) where $0 \le j < 5$. Herein, we have used

- 295 MATLAB as a statistical software package (developed by MathWorks) to estimate the maximum likelihood of the *j*th pair of model parameter $\{\alpha_{0,j}, \alpha_{1,j}\}$ by using the following MATLAB command: glmfit(log(\mathbf{x}_j), \mathbf{y}_j , 'binomial', 'link', 'model'). The 'model' will be either 'logit', 'probit', or 'comploglog'. For each damage state D_{j+1} , $0 \le j < 5$, the vector \mathbf{x}_j is the *IM_i*'s for which the condition $D \ge D_j$ is satisfied (e.g., for *j*=0, all the 120 buildings are considered, for *j*=1, 111 buildings are
- 300 considered, see Table 1); **y**_j is the column vector containing one-to-one probability assignment to the *IM* data in **x**_j with zero (=0.0) assigned to those data corresponding to *DS*_j (*D*_j ≤ *D* < *D*_{j+1}) and one (=1.0) to those related to higher damage states (with *D* ≥ *D*_{j+1}). The vectors defining the MLE of the model parameters, **θ**_{MLE}, are presented in Table 2 for each of the three models M₁, M₂, and M₃ defined in Section 3.2. Given the model parameter **θ**_{MLE}, the damage
- state probability $P(DS_j|IM_i)$ can be estimated based on the recursive Equation (7). Then, the fragility for the ultimate damage level D_5 , i.e., $P(D \ge D_5|IM, \boldsymbol{\theta}_{MLE})$, is calculated first based on Equation (8). For the lower damage thresholds D_j , where 0 < j < 5, the empirical fragility $P(D \ge D_j|IM, \boldsymbol{\theta}_{MLE})$ is derived based on the Equation (9). The resulting hierarchical fragility curves by employing the direct fragility assessment given $\boldsymbol{\theta}_{MLE}$, i.e., $P(D \ge D_j|IM, \boldsymbol{\theta}_{MLE})$ for $1 \le j \le 5$ are shown later in the next section by comparison with those obtained from the BMCS method.

Table 2.	The model	parameters	$\theta_{\rm MLF}$.

Model	$\alpha_{0,0}$	$\alpha_{1,0}$	$\alpha_{0,1}$	$\alpha_{1,1}$	$\alpha_{0,2}$	$\alpha_{1,2}$	$\alpha_{0,3}$	$\alpha_{1,3}$	$\alpha_{0,4}$	$\alpha_{1,4}$
\mathbb{M}_1	5.242	4.190	3.900	4.255	-1.175	4.805	-1.345	2.887	-1.994	2.917
\mathbb{M}_2	2.742	2.190	2.007	2.221	-0.670	2.804	-0.803	1.745	-1.157	1.733
\mathbb{M}_3	2.079	2.011	1.322	1.850	-1.268	3.057	-1.366	1.961	-1.981	2.218

3.4 Fragility modelling using BMCS

In the first step, the model parameters are estimated for each model class separately. For each model class \mathbb{M}_k , the $2N_{DS} = 10$ model parameters $\mathbf{\theta}_k$ are estimated through the adaptive MCMC method described in detail in Appendix E which yields the posterior distribution in Equation (14). With reference to Equation (14), the prior joint PDF $p(\mathbf{\theta}_k | \mathbb{M}_k)$ should be assigned in advance. As noted previously, $p(\mathbf{\theta}_k | \mathbb{M}_k)$ can be a multivariate normal PDF with zero correlation between the pairs of model parameters $\mathbf{\theta}_{10\times 1}$ (see Appendix D). The vector of the mean values, $\mathbf{\mu}_{\mathbf{\theta}_{10\times 1}}$, is set to be the MLE tabulated in Table 2 (= $\mathbf{\theta}_{MLE}$ related to \mathbb{M}_k). We have attributed a high value for the coefficient of

- 320 variation (COV=1.60 herein) for each of the 10 model parameters. Appendix F illustrates the histograms representing the drawn samples from the joint posterior PDF $p(\theta_k | \mathbf{D}, \mathbb{M}_k)$ for each of the 10 model parameters. The marginal normal prior PDFs are also shown (with statistics defined previously). The robust fragility (RF) curves derived from the hierarchical fragility curves (see Section 2.5) and the corresponding \pm two standard deviation intervals from Equation (16) and Equation (17) are also plotted
- in Figure 2a to Figure 4a corresponding to M_1 to M_3 , and for the damage thresholds D_1 to D_5 . The





colors of the RF curves match closely those shown in Figure 1. The corresponding \pm two standard deviation confidence interval curve, which reflects the uncertainty in the model parameters, is shown as a light grey area with different color intensities. Figures 2b to Figure 4b compare the RF and its confidence interval, labeled as D_j -BMCS, with the result of the direct fragility assessment (FA, see

- 330 MLE method in Section 2.3), labeled as D_j -MLE, for $1 \le j \le 5$. The MLE-based curves are shown with similar colors (and darker intensity) and with the same line type (and half of the thickness) of the corresponding RF curves. The first observation is that the results of MLE-based fragilities and the BMCS-based fragilities are quite close in all damage thresholds (as expected, see Jalayer and Erahimian 2020). Moreover, the BMCS provides also the confidence bands for the fragility curves, which cannot
- be directly provided by the MLE method. To showcase an individual fragility curve, Figure 2c to Figure 4c illustrate the empirical fragility curves associated with the l^{th} realization of the vector of model parameters $\boldsymbol{\theta}_{k,l}$ for model class \mathbb{M}_k (where l is defined on each figure separately), i.e., $P(D \ge D_j | IM, \boldsymbol{\theta}_{k,l})$ where $1 \le j \le 5$ (see Section 2.5). Figures 2d to Figure 4d illustrate the RF curve associated with the damage threshold D_5 , together with all the N_d sample fragilities (N_d =903 for \mathbb{M}_1 ,
- 340 N_d =882 for \mathbb{M}_2 , and N_d =951 for \mathbb{M}_3) shown with thin gray lines. N_d is the number of distinct samples as discussed in Appendix E and Appendix F of this manuscript. The total number of samples generated by adaptive MCMC in its last chain is N_{seed} =1000 ($N_d \le N_{seed}$). Figure 2d to 4d illustrate IM^{84} and IM^{16} as IM values at the median (50% probability) from the RF minus/plus one standard deviation, respectively (see Section 2.5). The equivalent lognormal parameters η_{IM_c} and β_{IM_c} , as well as the
- 345 epistemic uncertainty in the empirical fragility assessment β_{UF} are tabulated in Table 3 for damage thresholds D_1 to D_5 associated to model classes \mathbb{M}_1 to \mathbb{M}_3 .

Table 3. The equivalent lognormal parameters and the epistemic uncertainty in the RF assessment for damage thresholds D_1 to D_5 and for the model classes \mathbb{M}_1 to \mathbb{M}_3

Damage	Model 1 (1	\mathbb{M}_1)		Model 2 (N	$\mathbb{M}_2)$	Model 3 (I	Model 3 (\mathbb{M}_3)		
threshold	$\eta_{IM_c}[m]$	β_{IM_C}	β_{UF}	η_{IM_c} [m]	β_{IM_C}	β_{UF}	$\eta_{IM_c}[m]$	β_{IM_C}	β_{UF}
D_1	0.29	0.42	0.21	0.30	0.45	0.21	0.33	0.52	0.21
D_2	0.44	0.34	0.14	0.45	0.38	0.15	0.49	0.40	0.15
D_3	1.29	0.35	0.07	1.27	0.35	0.07	1.37	0.37	0.07
D_4	1.82	0.42	0.06	1.79	0.44	0.06	1.90	0.37	0.06
D_5	2.49	0.46	0.07	2.46	0.45	0.07	2.51	0.34	0.06





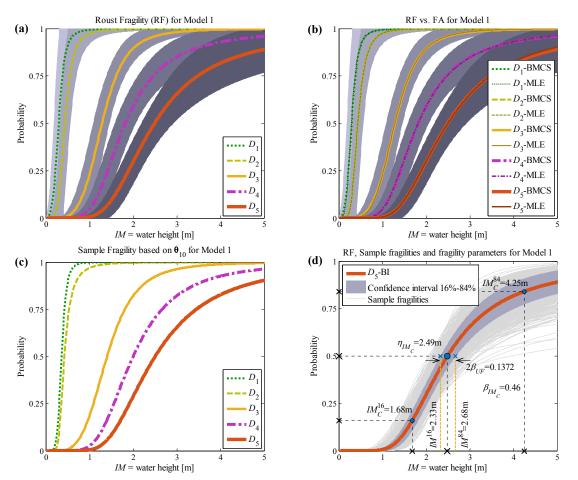


Figure 2: Model class \mathbb{M}_1 (a) Robust fragility curves (RF) and their \pm two standard deviation confidence intervals; (b) comparison between RF and its confidence band (based on BMCS method) and FA (based on MLE method); (c) the fragility curves $P(D \ge D_j | IM, \theta_{1,10})$ where $1 \le j \le 5$ associated with the 10th realization of the model parameters, $\theta_{1,10}$ (*k*=1 associated to model \mathbb{M}_1 , *l*=10); (d) RF associated with the damage threshold D_5 , together with all the N_d =903 sample fragilities, and the equivalent lognormal fragility

parameters.





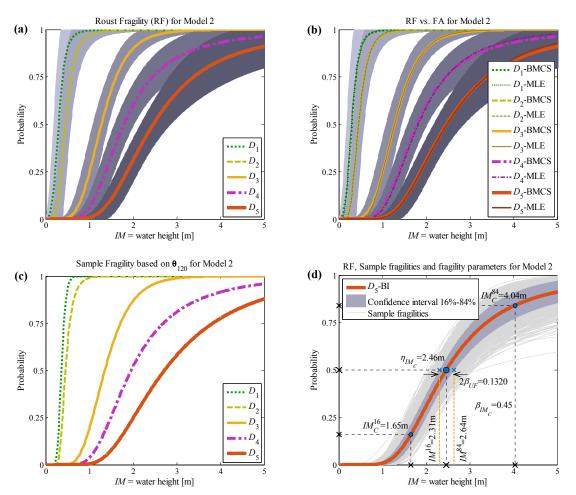


Figure 3: Model class \mathbb{M}_2 (a) Robust fragility curves (RF) and their \pm two standard deviation confidence intervals; (b) comparison between RF and its confidence band (based on BMCS method) and FA (based on MLE method); (c) the fragility curves $P(D \ge D_j | IM, \theta_{2,120})$ where $1 \le j \le 5$ associated with the 120th realization of the model parameters, $\theta_{2,120}$ (k=2 associated to model \mathbb{M}_2 , l=120); (d) RF associated with the damage threshold D_5 , together with all the $N_d=882$ sample fragilities, and the equivalent lognormal fragility parameters.

365





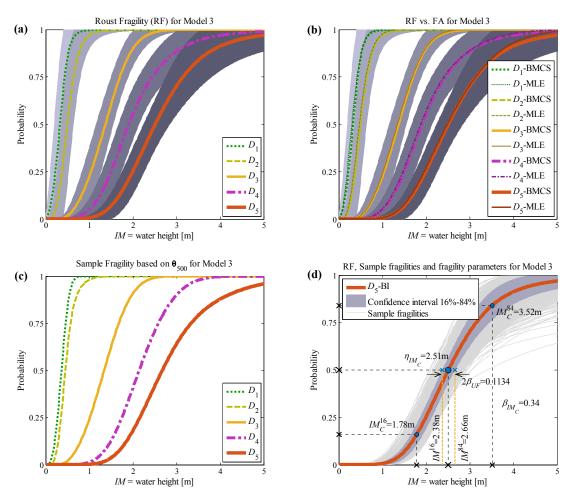


Figure 4: Model class \mathbb{M}_3 (a) Robust fragility curves (RF) and their \pm two standard deviation confidence intervals; (b) comparison between RF and its confidence band (based on BMCS method) and FA (based on MLE method); (c) the fragility curves $P(D \ge D_j | IM, \theta_{3,500})$ where $1 \le j \le 5$ associated with the 500th realization of the model parameters, $\theta_{3,500}$ (k=3 associated to model \mathbb{M}_3 , l=500); (d) RF associated with the damage threshold D_5 , together with all the $N_d=951$ sample fragilities, and the equivalent lognormal fragility parameters.

3.5 Model selection

With reference to Equation (12), the *log-evidence* $\ln[p(\mathbf{D}|\mathbf{M}_k)]$, can be estimated by subtracting *Term* 1 and *Term* 2. The former denotes the posterior mean of the log-likelihood, and the latter is the relative entropy between the prior and the posterior. Within the BMCS method, these two terms are readily computable.

Given the samples generated from the joint posterior PDF's $\boldsymbol{\theta}_k$, *Term* 1 (=*Average Data Fit*) can be seen as the expected value of the log-likelihood over the vector of fragility parameters $\boldsymbol{\theta}$ given the model \mathbb{M}_k , i.e., $\mathbb{E}_{\boldsymbol{\theta}_k | \mathbf{D}, \mathbb{M}_k}(\ln[p(\mathbf{D}|\mathbb{M}_k)])$. *Term* 2 (=*Information Gain*) is calculated as the expected value of

information gain or entropy between the two PDF's posterior and prior over the vector $\boldsymbol{\theta}$ given the model \mathbb{M}_k , i.e., $\mathbb{E}_{\boldsymbol{\theta}_k | \mathbf{D}, \mathbb{M}_k}(\ln[p(\boldsymbol{\theta} | \mathbf{D}, \mathbb{M}_k)/p(\boldsymbol{\theta} | \mathbb{M}_k)])$. It is noted that based on Jensen's inequality, the





mean information gain of posterior compared to the prior is always non-negative (see e.g., Jalayer et al. 2012, Ebrahimian and Jalayer 2021). Hence, Term 2 should always be positive. Herein, $p(\theta|\mathbf{D}, \mathbb{M}_k)$ is constructed by an adaptive kernel density function (see Equation E5, Appendix E) as the weighted sum (average) of 10-dimensional Gaussian PDFs centered among the samples θ_k given model \mathbb{M}_k (*k*=1:3). The prior $p(\theta|\mathbb{M}_k)$ is a multivariate normal PDF, respectively with the mean and covariance described previously for each model (see Equation D1 in Appendix D). Table 4 shows the results for model class

selection. The last column illustrates the posterior probability (weight) of the model $P(\mathbb{M}_k | \mathbf{D})$ according to Equation (10) assuming that the prior $P(\mathbb{M}_k) = \frac{1}{3}$ (where k=1:3).

390	Table 4. Bayesian model class selection results for empirical fragility models

		-	•••			
Model	Term 1: Average	Term 2: Information	Log Evidence	Posterior Probability of		
Class	Data Fit	Gain	Log-Evidence	each model		
\mathbb{M}_1	-124.2898	17.3825	-141.6723	0.058		
M_2	-123.1298	17.9314	-141.0612	0.107		
\mathbb{M}_3	-120.6051	18.4015	-139.0066	0.835		

Model class \mathbb{M}_3 (using a complementary log-log "cloglog" transformation of π_{ij} to the linear logarithmic space, see Equation 5) is preferred, since it has a better data fit, which makes the log-evidence greater. In terms of the information gain, all the three models perform similarly with higher

395 value attributed to \mathbb{M}_3 (and obviously being more penalized for it). After \mathbb{M}_3 , the model \mathbb{M}_2 with a lognormal distribution ("probit") is preferred compared to \mathbb{M}_2 with a logistic regression model ("logit") It is noteworthy that as new samples $\boldsymbol{\theta}_k$ become available through the BMCS method by performing new MCMC sampling, the posterior model probabilities will change; however, the whole procedure seems to be stable; i.e., the evidence that \mathbb{M}_3 is preferable among the models using the BMCS holds.

400 The posterior weights (last column of Table 4, see also Equation 10) of 6%, 11% and 83% is stabilized through different runs of the BMCS method with around 2% changes. It is noted that based on Jensen's inequality, the mean information gain of posterior compared to the prior is always non-negative (see e.g., Jalayer et al. 2012). Hence, Term 2 should always be positive.

3.6 The "Basic" (MLE-basic) method: fitting data to one damage state at a time

- 405 In the traditional method, the fragility $P(D \ge D_j | IM)$ is obtained by using a generalized linear regression model according to Equation (5) with "logit", "probit" or "cloglog" link function fitted to the damage data (\mathbb{M}_k where k = 1:3). With reference to the MLE method described previously, the vector \mathbf{x}_j herein is the *IM* associated to all damage data (and not partial, as in the hierarchical fragility method described in Section 3.3), and \mathbf{y}_j is the column vector of one-to-one probability assignment to
- 410 the *IM* data in \mathbf{x}_j with zero (=0) assigned to those data with an observed damage threshold $D < D_j$, and one (=1) to those with $D \ge D_j$. Thus, for the empirical fragility associated with the damage threshold D_j , and based on the model \mathbb{M}_k , there are two model parameters to be defined, namely $\mathbf{\theta}_{MLE-Basic} = \{\alpha_0, \alpha_1\}_k$. As noted previously, there might be conditions (depending on the quantity of the observed damage data), where a part of the fragility of damage threshold D_j lies below the fragility of the higher
- 415 damage level D_{j+1} , indicating that $P(DS_j|IM) < 0$. This is due to the fact that in the traditional method, there is no explicit requirement to satisfy $P(DS_j|IM) > 0$ as compared to the proposed method. The MLE of model parameters { α_0, α_1 } for damage levels D_1 to D_5 are presented in Table 5. Figure 5 compares the fragility assessment obtained based on MLE-based hierarchical fragility modeling (see also the MLE-based curves in Figure 2b to Figure 4b) with the result of FA by employing
- 420 the MLE-Basic method for the three considered Model Classes \mathbb{M}_k (k=1:3). It is noted that the fragility





function is different between the two methods. MLE-based FA given M_k uses Equation (7) to Equation (9) to construct hierarchical fragility curve given that the conditional fragility term π_{ij} = P(D ≥ D_{j+1}|D ≥ D_j, IM_i) has one of the functional forms in Equation (5). However, the FA using MLE-Basic method employs directly one of the expressions in Equation (5) (corresponding to M_k, 425 k=1:3) to derive the fragility curve (on the whole damage data). This difference manifests itself in the deviation between the two fragility models in Figure 5, especially for higher damage thresholds D₄ and D₅. The deviations between the fragility curves are particularly noticeable at higher IM values (with exceedance probability >50%); however, their medians are quite similar. Strictly speaking, the fragilities are closer in the case of M₂ and M₃.

430 Table 5. The Model parameters $\theta_{MLE-Basic}$.

Model	$D \ge D_1$		$D \ge D_2$	$D \ge D_2$		$D \ge D_3$		$D \ge D_4$		
	α_0	α_1	α_0	α_1	α_0	α_1	α_0	α_1	α_0	α_1
\mathbb{M}_1	5.242	4.190	3.655	4.556	-1.221	4.884	-2.666	2.213	-4.271	4.651
\mathbb{M}_2	2.742	2.190	1.946	2.486	-0.695	2.846	-1.506	2.425	-2.293	2.515
\mathbb{M}_3	2.079	2.011	1.347	2.361	-1.319	3.139	-2.390	3.009	-3.919	3.806

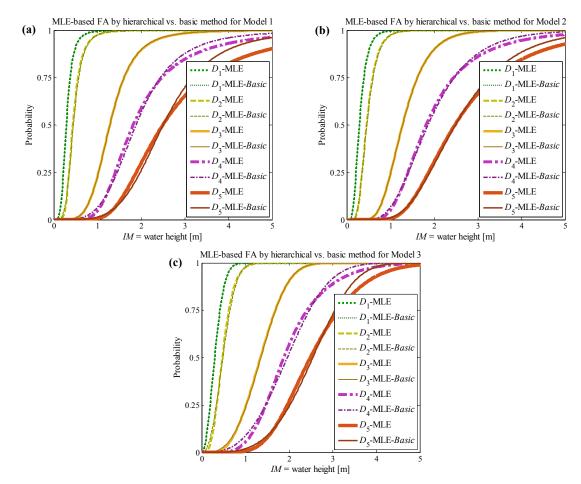






Figure 5: (a) Comparison between the FA by MLE-based hierarchical fragility modeling and direct FA based on MLE-*Basic* method given (a) M_1 , (b) M_2 and (c) M_3 models

To have a better comparison among the fragility curves in Figure 5, Table 6 reports the FA parameters of the MLE and MLE-*Basic* methods for the damage thresholds D_1 to D_5 with the equivalent lognormal parameters η_{IM_c} and β_{IM_c} (explained in Section 2.5) for \mathbb{M}_1 to \mathbb{M}_3 . The medians are almost identical among the four models while there are higher dispersion estimates for MLE method derived by hierarchical fragility modelling. It is noteworthy that the fragility curves derived based on the MLE-*Basic* method do not intersect here; however this condition was not explicitly enforced --as it was in the hierarchical method.

Table 6: Comparison between FA based on MLE method (by hierarchical fragility modelling) and the MLE-*Basic* method for damage thresholds D_1 to D_5 .

e		Mode	$\mathfrak{l} \mid (\mathbb{M}_1)$			Model	$2(\mathbb{M}_2)$		Model 3 (\mathbb{M}_3)			
Damage Level	MLE method		MLE-Basic method		MLE method		MLE-Basic method		MLE method		MLE-Basic method	
Д	$\eta_{IM_c}[m]$	β_{IM_C}	$\eta_{IM_c}[m]$	β_{IM_C}	$\eta_{IM_c}[m]$	β_{IM_C}	$\eta_{IM_c}[m]$	β_{IM_C}	$\eta_{IM_c}[m]$	β_{IM_C}	$\eta_{IM_c}[m]$	β_{IM_C}
D_1	0.29	0.40	0.29	0.40	0.29	0.46	0.29	0.46	0.30	0.59	0.30	0.59
D_2	0.43	0.35	0.45	0.37	0.45	0.38	0.46	0.40	0.47	0.44	0.48	0.50
D_3	1.28	0.34	1.28	0.34	1.27	0.35	1.28	0.35	1.34	0.38	1.35	0.38
D_4	1.82	0.43	1.88	0.40	1.82	0.42	1.86	0.41	1.88	0.38	1.96	0.39
D_5	2.50	0.46	2.50	0.36	2.47	0.44	2.49	0.40	2.49	0.34	2.54	0.31

Conclusion

455

- 445 An integrated procedure based on Bayesian model class selection (BMCS) for empirical fragility modeling for a class of buildings or infrastructure is discussed. This procedure relies on efficient simulation techniques to: 1) perform hierarchical fragility modeling for mutually exclusive and collectively exhaustive damage states; 2) estimate the confidence interval for the resulting fragility curves; 3) select the simplest model that fits the data best amongst a suite of candidate fragility models
- 450 (herein, alternative link functions for generalized linear regression are considered). The proposed procedure is demonstrated for empirical fragility assessment based on observed damage data to masonry buildings due to the 2009 South Pacific Tsunami in the American Samoa and Samoa Islands. It is observed that:
 - For each model class, the same set of simulation realizations is used to estimate the fragility parameters, the confidence band, and the log evidence. The latter, which consists of two terms depicting the goodness of fit and the information gain resulting from the observed data, is used to compare the candidate fragility models to identify the model that maximizes evidence.
- Hierarchical fragility assessment can be done also based the maximum likelihood estimation (MLE) and the available statistical toolboxes (e.g., MATLAB's generalized linear model). For each damage level, the reference domain should be the subset of data that exceeds the consecutive lower damage level, instead of taking the entire set of data points as reference domain. Note that the basic fragility estimation (MLE-Basic) fits the damage data for each damage level at a time. In other words, the reference domain is set to all damage data.
- Although the resulting fragility curves are not lognormal (strictly speaking), equivalent statistics are used to show the fragility curves (median and logarithmic dispersion) and the corresponding epistemic uncertainty (logarithmic dispersion).



470



- The results show that the fragility curves built based on "cloglog" link function lead to the highest evidence compared to the fragility curves obtained based on the other two link functions "logit" and "probit".
- Moreover, the proposed method BMCS and the one based on maximum likelihood estimation (MLE) lead to essentially the same set of parameters' estimates for hierarchical fragility estimation. However, the latter does not readily lead to the confidence band and log evidence.
 - Using the basic method for fragility estimation ("MLE-Basic", non-hierarchical fragility model) leads to results that are slightly different from the hierarchical fragility curves. The
- 475 difference grows for higher damage levels. It is to note that following the basic method "MLE-Basic" did not lead to ill-conditioned results (i.e., fragility curves crossing) in the case-study presented herein. Nevertheless, it is not guaranteed that, through following the basic method, the crossing fragility curves are going to be avoided.
- 480 The proposed method is quite general with respect for empirical fragility modelling and is transferable also to other types of hazards. This procedure is based on the assumption that given the intensity values, the set of observed damage data are independent.





Appendix A: The derivation of Equation (2)

485 The probability of being in damage state DS_j for a given intensity measure *IM* can be estimated as follows:

$$P(DS_{j}|IM) = P[(D \ge D_{j}) \cdot (D < D_{j+1})|IM] = 1 - P[\overline{(D \ge D_{j}) \cdot (D < D_{j+1})}|IM]$$

$$= 1 - P[\overline{(D \ge D_{j})} + \overline{(D < D_{j+1})}|IM] = 1 - P[(D < D_{j}) + (D \ge D_{j+1})|IM]$$

$$= 1 - P(D < D_{j}|IM) - P(D \ge D_{j+1}|IM) = P(D \ge D_{j}|IM) - P(D \ge D_{j+1}|IM)$$
(A1)

where the upper-bar sign stands for the logical negation and is read as "NOT", and (+) defines the logical sum and is read as "OR". The above derivation is based on the *rule of sum* in probability and considering the fact that the two statements $D < D_j$ and $D \ge D_{j+1}$ are mutually exclusive (ME); thus, the probability of their logical sum is the sum of their probabilities.

Appendix B: The derivation of Equation (7)

The probability of being in damage state DS_i given the intensity measure evaluated at the location of building *i*, denoted as IM_i , based on Equation (6) can be expanded in a recursive format as follows:

$$P(DS_{j}|IM_{i}) = \underbrace{\left[1 - P(D \ge D_{j+1}|D \ge D_{j}, IM_{i})\right]}_{1-\pi_{ij}} \cdot \left[1 - P(D < D_{j}|IM_{i})\right]$$

$$= (1 - \pi_{ij}) \cdot \left[1 - P((D < D_{j}) \cdot (D \ge D_{j-1}) + \dots + (D < D_{1}) \cdot (D \ge D_{0})|IM_{i})\right]$$

$$= (1 - \pi_{ij}) \cdot \left[1 - \sum_{k=0}^{j-1} P((D < D_{k+1}) \cdot (D \ge D_{k})|IM_{i})\right]$$

$$= (1 - \pi_{ij}) \cdot \left[1 - \sum_{k=0}^{j-1} P(DS_{k}|IM)\right]$$
(B1)

495

where (+) defines the logical sum and is read as "OR". The above derivation is based on the rule of sum in probability and considering the fact that the recursive statements in the second term expressed generally as $(D < D_{k+1}) \cdot (D \ge D_k)$, where $0 \le k \le j - 1$, are ME; hence, the probability of their logical sum is the sum of their probabilities.

500 Appendix C: The derivation of log-evidence in Equation (13)

From an information-based point of view, the logarithm of the evidence (*log-evidence*), denoted as $\ln[p(\mathbf{D}|\mathbb{M}_k)]$, can provide a quantitative measure of the amount of information as evidence of model \mathbb{M}_k . Moreover, the posterior PDF $p(\boldsymbol{\theta}|\mathbf{D},\mathbb{M}_k)$ (see Equation 14) over the domain of the model parameters $\Omega_{\boldsymbol{\theta}}$ given the k^{th} model is equal to unity. Thus, $\ln[p(\mathbf{D}|\mathbb{M}_k)]$ can be written as follows:

505
$$\ln\left[p(\mathbf{D}|\mathbf{M}_{k})\right] = \ln\left[p(\mathbf{D}|\mathbf{M}_{k})\right] \cdot \underbrace{\int_{\Omega\theta_{k}} p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}}_{=1.0}$$
 (C1)

Since the log-evidence is independent of θ , we can bring it inside the integral, and do some simple manipulation (also using the relation in Equation 11) as follows:





$$\ln\left[p(\mathbf{D}|\mathbf{M}_{k})\right] = \int_{\Omega_{\theta_{k}}} \ln\left[p(\mathbf{D}|\mathbf{M}_{k})\right] \cdot p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta} = \int_{\Omega_{\theta}} \ln\left[\frac{p(\mathbf{D}|\boldsymbol{\theta},\mathbf{M}_{k})p(\boldsymbol{\theta}|\mathbf{M}_{k})}{p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k})}\right] \cdot p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}$$
$$= \int_{\Omega_{\theta}} \ln\left[\frac{p(\mathbf{D}|\boldsymbol{\theta},\mathbf{M}_{k})}{p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k})/p(\boldsymbol{\theta}|\mathbf{M}_{k})}\right] \cdot p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}$$
$$= \int_{\Omega_{\theta}} \ln\left[p(\mathbf{D}|\boldsymbol{\theta},\mathbf{M}_{k})\right] \cdot p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta} - \int_{\Omega_{\theta}} \ln\left[\frac{p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k})}{p(\boldsymbol{\theta}|\mathbf{M}_{k})}\right] \cdot p(\boldsymbol{\theta}|\mathbf{D},\mathbf{M}_{k}) d\boldsymbol{\theta}$$
$$(C2)$$

510 Appendix D: Multivariate normal distribution and generating dependent Gaussian variables

A multivariate normal PDF can be expressed as follows:

$$p(\boldsymbol{\theta}) = \frac{1}{\sqrt{(2\pi)^{n} |\mathbf{S}|}} \exp\left(-\frac{1}{2} (\boldsymbol{\theta} - \boldsymbol{\mu}_{\boldsymbol{\theta}})^{\mathrm{T}} \mathbf{S}^{-1} (\boldsymbol{\theta} - \boldsymbol{\mu}_{\boldsymbol{\theta}})\right)$$
(D1)

where *n* is the number of components (uncertain parameters) of vector $\mathbf{\theta} = \{\theta_i, i = 1: n\}$; $\mu_{\mathbf{\theta}}$ is the 515 vector of the mean value of $\mathbf{\theta}$; **S** is the covariance matrix. The positive definite matrix $\mathbf{S}_{n \times n}$ can be factorized based on Cholesky decomposition as $\mathbf{S}=\mathbf{LL}^T$, where $\mathbf{L}_{n \times n}$ is a lower triangular matrix (i.e., for all j > i, $L_{ij} = 0$ where L_{ij} denotes the (i, j)-entry of the matrix **L**). A Gaussian vector $\mathbf{\theta}_{n \times 1}$ with mean $\mu_{\mathbf{\theta}}$ and covariance **S** can be generated as follows:

$$\boldsymbol{\theta} = \boldsymbol{\mu}_{\boldsymbol{\theta}} + \mathbf{L} \mathbf{Z} \tag{D2}$$

520 where $Z_{n\times 1}$ is a vector of standard Gaussian *i.i.d.* random variables with zero mean $\mathbf{0}_{n\times n}$, and covariance equal to the identity matrix $\mathbf{I}_{n\times n}$. To verify the properties of $\mathbf{0}$, we know that with reference to Equation (D2), it should have a mean equal to $\mu_{\mathbf{0}}$ and a covariance matrix equal to \mathbf{S} . The expectation of $\mathbf{0}$, denoted as $E(\mathbf{0})$, can be estimated as:

$$E(\theta) = E(\mu_{\theta} + LZ) = E(\mu_{\theta}) + L\underbrace{E(Z)}_{=0_{md}} = \mu_{\theta}$$
(D3)

525 The covariance matrix of $\boldsymbol{\theta}$ can be written as:

$$\mathbf{E}\left[\left(\boldsymbol{\theta}-\boldsymbol{\mu}_{\boldsymbol{\theta}}\right)\left(\boldsymbol{\theta}-\boldsymbol{\mu}_{\boldsymbol{\theta}}\right)^{T}\right]=\mathbf{E}\left(\mathbf{L}\mathbf{Z}\mathbf{Z}^{T}\mathbf{L}^{T}\right)=\mathbf{L}\underbrace{\mathbf{E}\left(\mathbf{Z}\mathbf{Z}^{T}\right)}_{=\mathbf{I}_{max}}\mathbf{L}^{T}=\mathbf{L}\mathbf{L}^{T}=\mathbf{S}$$
(D4)

Thus, the vector $\boldsymbol{\theta}$ can be written according to Equation (D2).

Appendix E: Adaptive MCMC scheme

MCMC procedure

530 The MCMC simulation scheme has a Markovian nature where the transition from current state to a new state is done by using a conditional transition function that is conditioned on the current (last) state. To generate $(i+1)^{\text{th}}$ sample $\boldsymbol{\theta}_{i+1}$ from the current i^{th} sample $\boldsymbol{\theta}_i$ based on MH routine, the following procedure





is adopted herein:

- Simulate a *candidate* sample θ^{*} from a *proposal* distribution q(θ|θ_i). It is important to note that there are no specific restrictions about the choice of q(·) apart from the fact that it should be possible to calculate both q(θ_{i+1}|θ_i) and q(θ_i|θ_{i+1}).
 - Calculate the acceptance probability min(1,*r*), where *r* is defined as follows (it is to note that the following Equation E1 is written in the general format for brevity compared to Equation 14 of the manuscript, and we have used θ instead of θ_k , and dropped the conditioning on \mathbb{M}_k ; hence when we write the *i*th sample θ_i , it is actually the *i*th sample drawn from θ_k and "*k*" is dropped for brevity):

540

$$r = \frac{p(\boldsymbol{\theta}^*|\mathbf{D})}{p(\boldsymbol{\theta}_i|\mathbf{D})} \cdot \frac{q(\boldsymbol{\theta}_i|\boldsymbol{\theta}^*)}{q(\boldsymbol{\theta}^*|\boldsymbol{\theta}_i)} = \left(\frac{p(\mathbf{D}|\boldsymbol{\theta}^*)}{\underbrace{p(\mathbf{D}|\boldsymbol{\theta}_i)}_{\text{likelihood ratio}}} \cdot \frac{p(\boldsymbol{\theta}^*)}{\underbrace{p(\mathbf{O}_i)}_{\text{prior ratio}}} \right) \cdot \frac{q(\boldsymbol{\theta}_i|\boldsymbol{\theta}^*)}{\underbrace{q(\boldsymbol{\theta}^*|\boldsymbol{\theta}_i)}_{\text{proposal ratio}}}$$
(E1)

- Generate *u* from a Uniform distribution between (0, 1), *u* ~ Uniform (0, 1).
- if $u \le \min(1, r) \to \operatorname{set} \mathbf{\theta}_{i+1} = \mathbf{\theta}^*$ (*accept* the *candidate* state to be taken as the *next* state of the Markov chain); else set $\mathbf{\theta}_{i+1} = \mathbf{\theta}_i$ (the *current* state is taken as the *next* state).
- 545 Estimating the likelihood in the arithmetic scale based on Equation (E1) may encounter instability as $p(\mathbf{D}|\mathbf{\theta})$ may become very small; thus, the likelihood ratio becomes indeterminate. To avoid this numerical instability, it is desirable to substitute the likelihood ratio in Equation (E1) with $\exp(\ln(p(\mathbf{D}|\mathbf{\theta}^*)) \ln(p(\mathbf{D}|\mathbf{\theta}_i)))$ if the ratio becomes indeterminate or zero.

With reference to Equation (E1), samples from the posterior can be drawn based on MH algorithm 550 without any need to define the normalizing C^{-1} coefficient according to Equation (14). Equation (E1) always accepts a candidate if the new proposal is more likely under the target posterior distribution than the old state. Therefore, the sampler will move towards the regions of the state space where the target posterior function has high density.

The choice of the proposal distribution q is very important. The ratio $q(\theta_i|\theta_i)/q(\theta^*|\theta_i)$ corrects for any asymmetries in the proposal distribution. Intuitively, if $q(\theta^*|\theta_i)=p(\theta^*|\mathbf{D})$, the candidate state is always accepted (with r=1); thus the closer q is to the target posterior PDF, the better the acceptance rate and the faster the convergence. This is not a trivial task as information about the important region $p(\theta|\mathbf{D})$ is not available. If the proposal distribution q is *non-adaptive*, it means that the information of the current

- sample $\boldsymbol{\theta}_i$ is not used to explore the important region of the target posterior distribution $p(\boldsymbol{\theta}|\mathbf{D})$; thus, we can say that $q(\boldsymbol{\theta}^*|\boldsymbol{\theta}_i) = q(\boldsymbol{\theta}^*)$. Therefore, it is more desirable to choose an *adaptive* proposal distribution
- which depends on the current sample (Beck and Au 2002). Having the proposal PDF q centered around the current sample renders the MH algorithm like a local random walk that adaptively leads to the generation of the target PDF. However, if the Markov chain starts from a point that is not close to region of the significant probability density of $p(\theta|\mathbf{D})$, the chance of generating a candidate state θ^* will become
- 565 extremely small (and we will face high rejection of candidate samples). Therefore, most of the samples will be repeated. To solve this problem, Beck and Au (2002) introduce a sequence of PDFs that bridge the gap between the prior PDF and the target posterior PDF. This issue will be more explored hereafter under the adaptive MCMC. Finally, it can mathematically be shown that (see Beck and Au 2002) if the current sample θ_i is distributed as $p(\cdot|\mathbf{D})$, the next sample θ_{i+1} is also distributed as $p(\cdot|\mathbf{D})$.

570 Adaptive Metropolis-Hastings algorithm (adaptive MCMC)

The adaptive MH algorithm (Beck and Au 2002) introduces a sequence of intermediate candidate





evolutionary PDF's that resemble more and more the target PDF. Let $\{p_1, p_2, ..., p_{Nchain}\}$ be the sequence *(chain)* of PDF's leading to $p(\boldsymbol{\theta}|\mathbf{D})=p_{Nchain}$, where *Nchain* is the number of chains and each chain contains *Nseed* samples (as indicated subsequently). The following adaptive simulation-based procedure is employed:

- **Step 1:** Simulate *Nseed* samples $\{\mathbf{\theta}_1, \mathbf{\theta}_2, ..., \mathbf{\theta}_{Nseed}\}^{(1)}$, where the superscript (1) denotes the first simulation level or the first chain (nc=1 where nc denotes the chain number/simulation level), with the target PDF p_1 as the first sequence of samples. Instead of accepting or rejecting a proposal for $\mathbf{\theta}$ involving all its components simultaneously (called *block-wise* updating scheme), it might be
- 580 computationally simpler and more efficient for the first chain to make proposals for individual components of $\boldsymbol{\theta}$, one at a time (called *component-wise* updating approach). In the *block-wise* updating, the proposal distribution has the same dimension as the target distribution. For instance, if the model parameters involve *n* uncertain parameters (e.g., the vector of model parameters $\boldsymbol{\theta}_{n\times 1}$ in this paper has $n = 2(N_{DS} 1) = 8$ variables for each of the three models M_1 , M_2 , and M_3), we design an *n*-
- 585 dimensional proposal distribution, and either accept or reject the candidate state (with all *n* variables) as a block. The block-wise updating approach can be associated with high rejection rates. This may cause problem when we want to generate the first sequence of samples (first chain). Therefore, we have utilized the more stable component-wise updating for the first chain. We start from the first variable and generate a candidate state based on a proposal distribution for this individual component, and finally
- 590 accept or reject it based on MH algorithm. Note that in this stage, we have varied the current component and kept the other variables in vector $\boldsymbol{\theta}$ constant. Then, we move to the next components one by one and do the same procedure while taking into account the previous (updated) components. Therefore, what happens in the current step is conditional on the updated parameters in the previous steps.
- **Step 2:** Construct a kernel density function $\kappa^{(1)}$ as the weighted sum (average) of *n*-dimensional Gaussian PDFs centered among the samples $\{\theta_1, \theta_2, ..., \theta_{Nseed}\}^{(1)}$, with the covariance matrix $\mathbf{S}^{(1)}$ of the samples $\theta_i^{(1)}$ and the weights associated to each sample as w_i where *i*=1:*Nseed* as follows (see Ang et al. 1992, Au and Beck 2002):

$$\kappa^{(1)}(\boldsymbol{\theta}) = \frac{1}{Nseed} \sum_{i=1}^{Nseed} \frac{1}{w_i^n \sqrt{(2\pi)^n |\mathbf{S}^{(1)}|}} \exp\left(-\frac{1}{2w_i^2} (\boldsymbol{\theta} - \boldsymbol{\theta}_i^{(1)})^T (\mathbf{S}^{(1)})^{-1} (\boldsymbol{\theta} - \boldsymbol{\theta}_i^{(1)})\right)$$
(E2)

The kernel density $\kappa^{(1)}$ constructed in Equation (E2) approximates p_1 . The kernel function κ can be viewed as a PDF consisting of bumps at θ_i , where width w_i controls the common size of the bumps. Therefore, a large value of w_i tends to over-smooth the kernel density, while a small value may cause noise-shaped bumps. In view of this, w_i can be assumed to have a fixed width (= w), or alternatively the *adaptive kernel* estimate can be employed (Ang et al. 1992, Au and Beck 1999) that is defined for each sample θ_i , *i*=1:*Nseed*. The adaptive kernel has better convergence and smoothing properties over the fixed-width kernel estimate. The fixed width w is estimated as follows (Epanechnikov 1969):

$$w = \left(\frac{4}{(n+2)N_d}\right)^{\frac{1}{n+4}}$$
(E3)

where N_d is the number of distinct samples ($N_d \le Nseed$). For one-dimensional problems (n=1), this leads to the well-known fixed-width value of $[(4/3)/Nseed]^{1/5}$. The reason for using N_d is due to the fact that for the next simulation levels, where we are going to use a block-wise updating approach in the MCMC scheme, one may be fixed with rejection of candidate states within the Markov chain. Thus,

610 the MCMC scheme, one may be faced with rejection of candidate states within the Markov chain. Thus, we need to count the distinct samples. In the adaptive kernel method, the idea is to vary the shape of each bump so that a larger width (flatter bump) is used in regions of lower probability density. Following the general strategy used in the past (see Ang et al. 1992, Au and Beck 1999), the adaptive





band width w_i for the *i*th sample $\mathbf{\theta}_i$ can be written as $w_i = w\lambda_i$, where the local bandwidth factor λ_i can be estimated as follows:

$$\lambda_{i} = \left(\kappa(\boldsymbol{\theta}_{i}) \middle/ \left(\prod_{j=1}^{N_{seed}} \kappa(\boldsymbol{\theta}_{j}) \right)^{\frac{1}{N_{seed}}} \right)^{-\omega}$$
(E4)

where $0 \le \omega \le 1.0$ is the sensitivity factor, and $\kappa(\mathbf{\theta}_i)$ is calculated based on Equation (E2) where $\mathbf{\theta} = \mathbf{\theta}_i$, with the choice of fixed-width w in Equation (E3). The denominator in Equation (E4) is a geometric mean of the kernel estimator at all *Nseed* points. The value of $\omega = 0.50$ is employed herein as also suggested by other research endeavors (Abramson 1982, Ang et al. 1992, Au and Beck 1999). It is

numerically more stable to estimate the denominator in Equation (E4) as $\prod_{j=1}^{Nseed} \left[\kappa(\mathbf{\theta}_j)^{1/Nseed} \right]$.

Step 3: Simulate *Nseed* Markov chain samples $\{\theta_1, \theta_2, ..., \theta_{Nseed}\}^{(2)}$ with the target PDF p_2 as the second simulation level (*nc*=2). We use $\kappa^{(1)}$ as the proposal distribution $q(\cdot)$ in Equation (E1) in this stage to generate the second chain of samples. To generally simulate sample θ from the kernel $\kappa^{(nc)}$ (where *nc*=1:*Nchain*), we generate a discrete random index from the vector [1, 2, ..., *Nseed*] with the corresponding weights [$w_1, w_2, \cdots, w_{Nseed}$] using an inverse transformation sampling; if index=*j*, then generate θ from the Gaussian PDF κ_j , where:

$$\kappa_{j}(\boldsymbol{\theta}) = \frac{1}{\left(w\lambda_{j}\right)^{n}\sqrt{\left(2\pi\right)^{n}\left|\mathbf{S}^{(nc)}\right|}} \cdot \exp\left(-\frac{1}{2\left(w\lambda_{j}\right)^{2}}\left(\boldsymbol{\theta}-\boldsymbol{\theta}_{j}\right)^{\mathrm{T}}\left(\mathbf{S}^{(nc)}\right)^{-1}\left(\boldsymbol{\theta}-\boldsymbol{\theta}_{j}\right)\right)$$

$$= \frac{1}{\sqrt{\left(2\pi\right)^{n}\left|\mathbf{S}_{j}^{(nc)}\right|}} \cdot \exp\left(-\frac{1}{2}\left(\boldsymbol{\theta}-\boldsymbol{\theta}_{j}\right)^{\mathrm{T}}\left(\mathbf{S}_{j}^{(nc)}\right)^{-1}\left(\boldsymbol{\theta}-\boldsymbol{\theta}_{j}\right)\right)$$
(E5)

where $\mathbf{S}_{j}^{(nc)} = w_{j}^{2} \mathbf{S}^{(nc)}$, where $\mathbf{S}^{(nc)}$ is the covariance matrix of the samples $\{\mathbf{\theta}_{1}, \mathbf{\theta}_{2}, ..., \mathbf{\theta}_{Nseed}\}^{(nc)}$.

630 Appendix D shows how a sample $\boldsymbol{\theta}$ can be drawn from the Gaussian PDF κ_i . From this sequence on, the MCMC updating is done in a block-wise manner as we generate a candidate $\boldsymbol{\theta}$ and accept/reject it as a block. The second chain of samples $\{\boldsymbol{\theta}_1, \boldsymbol{\theta}_2, ..., \boldsymbol{\theta}_{Nseed}\}^{(2)}$ are then used to construct the kernel density $\kappa^{(2)}$ based on Equation (E2).

Step 4: In general, $\kappa^{(nc)}$ is used as the proposal distribution in order to move from the *nc*th simulation level (which approximates p_{nc}) into $(nc+1)^{\text{th}}$ chain (with target PDF p_{nc+1}). This will continue until the *Nchain*th simulation level where Markov chain samples are simulated for the target updated $p(\boldsymbol{\theta}|\mathbf{D})=p_{Nchain}$.

Appendix F: MCMC samples for each model

The adaptive MCMC procedure for drawing samples from the model parameters from the joint posterior PDF $p(\theta_k | \mathbf{D}, \mathbf{M}_k)$ is carried out by considering *Nchain*=5 chains (simulation levels), and *Nseed*=1000 samples per each chain (see Appendix E). In the first simulation level (first chain, *nc*=1), for which a component-wise updating approach is employed (see Appendix E, **Step 1** for the description of component-wise and block-wise updating), the first 20 samples are not considered in order to reduce the initial transient effect of the Markov chain. The proposal distribution (see Equation E1) for each

645 component is assumed to be a normal distribution with a COV=1.60 herein. In addition, the prior ratio according to Equation (E1), will become the ratio of two normal distributions, for each component one at a time. In the next simulation levels (i.e., nc=2 to 5), the adaptive kernel estimate (Equation E2) is



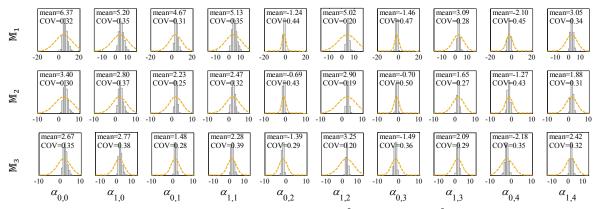


employed, i.e., the MCMC updating is performed in a block-wise manner. Since this updating approach can be associated with high rejection rates (i.e., there are similar samples indicating the rejection of the candidate states within the Markov chain), there will be N_d distinct (not considering the repeats) Markov

chain samples generated within the each chain, denoted as $\{\boldsymbol{\theta}_{k,1}, \boldsymbol{\theta}_{k,2}, \cdots, \boldsymbol{\theta}_{k,N_d}\}^{(nc)}$, where $N_d \leq Nseed$ and nc=2:Nchain (=5). The samples of the last chain (nc=5) will be used as the fragility model parameters, as discussed in Section 3.4. It is to note that the likelihood $p(\mathbf{D}|\boldsymbol{\theta}_k, \mathbf{M}_k)$ (used in calculating the acceptance probability within the MCMC procedure in Equation E1) is estimated according to 655 Equation (13).

Figure 2 illustrates the histograms representing the drawn samples from the joint posterior PDF's corresponding to the sampled model parameters $\{\boldsymbol{\theta}_{k,1}, \boldsymbol{\theta}_{k,2}, \dots, \boldsymbol{\theta}_{k,N_d}\}^{(5)}$ related to \mathbb{M}_k (where k = 1:3). For model \mathbb{M}_1 , N_d =903< Nseed=1000; for model \mathbb{M}_2 , N_d =882; finally, for \mathbb{M}_3 , we have N_d =951. The marginal normal prior PDFs are also shown with orange-coloured dashed lines. The statistics of

660 the samples (mean and COV of the posterior) of model parameters $\boldsymbol{\theta}_k$ are shown on the figures associated to each parameter. It is expected to have the mean values of the marginal posterior samples close to and comparable with those obtained by the MLE in Table 2 (first row) that are also the mean values of prior joint PDF.



665 Figure F1: Distribution of the fragility model parameters $\theta_k = \left[\left\{ \alpha_{0,j}, \alpha_{1,j} \right\}_k, j = 0; 4 \right]$ based on model class \mathbb{M}_k (where k = 1; 3) by employing an adaptive MCMC procedure including samples drawn from the joint posterior PDF with their statistics (mean and COV), and the marginal normal priors (subfigures show the posterior statistics).





670 Code availability

The code implementing the methodology in this article is available at the following URL: https://github.com/eurotsunamirisk/computeFrag

Data availability

The data used to produce the results in this article are available at the following URL:

675 <u>https://github.com/eurotsunamirisk/computeFrag</u>

Author contribution

FJ designed and coordinated this research. HE performed the simulations and developed the fragility functions. KT cured the availability of codes and software on the European Tsunami Risk Service (ETRiS). BB provided precious insights on the damage data gathered for American Samoa and Samoa

680 Islands in the aftermath of the 2009 South Pacific Tsunami (documented in Reese et al 2011). All authors have contributed to the drafting of the manuscript. The first two authors contributed in an equal manner to the drafting of the manuscript.

Competing interests:

The authors declare that they have no conflict of interest.

685 References

Abramson, I. S.: On bandwidth variation in kernel estimates-a square root law, *Ann. Stat.*, 10(4), 1217-1223, 1982.

Agresti, A.: Categorical Data Analysis, 3rd ed., Wiley, New York, 2012.

Ang, G. L., Ang, A. H.-S., and Tang, W. H.: Optimal importance sampling density estimator, *J. Eng.* 690 *Mech.*, 118(6), 1146–1163, 1992.

Au, S. K., and Beck, J. L.: A new adaptive importance sampling scheme, *Struct. Saf.*, 21, 135–158, 1999.

Beck, J. L., and Yuen K.-V.: Model selection using response measurements: Bayesian probabilistic approach, *J. Eng. Mech.*, 130,192–203, 2004.

695 Behrens, J., Løvholt, F., Jalayer, F., Lorito, S., Salgado-Gálvez, M. A., Sørensen, M., ..., and Vyhmeister, E.: Probabilistic Tsunami Hazard and Risk Analysis: A Review of Research Gaps, *Front. Earth Sci.*, 9, 114, 2021.

Cover, T. M., Thomas J. A.: Elements of information theory, Wiley, New York, 1991.

Charvet, I., Ioannou, I., Rossetto, T., Suppasri, A., and Imamura, F.: Empirical fragility assessment of
 buildings affected by the 2011 Great East Japan tsunami using improved statistical models, *Nat. Hazards*, 73, 951–973, 2014.

Charvet, I., Suppasri, A., Kimura, H., Sugawara, D., and Imamura, F.: A multivariate generalized linear tsunami fragility model for Kesennuma City based on maximum flow depths, velocities and debris impact, with evaluation of predictive accuracy, *Nat Hazards*, 79(3), 2073-2099, 2015.





705 Charvet, I., Macabuag, J., and Rossetto, T.: Estimating tsunami-induced building damage through fragility functions: critical review and research needs, *Front. Built Environment*, 3, 36, 2017.

Chua, C. T., Switzer, A. D., Suppasri, A., Li, L., Pakoksung, K., Lallemant, D., ..., and Winspear, N.: Tsunami damage to ports: Cataloguing damage to create fragility functions from the 2011 Tohoku event, *Nat Hazards Earth Sys*, 21(6), 1887-1908, 2021.

710 De Risi, R., Goda, K., Mori, N., and Yasuda T.: Bayesian tsunami fragility modelling considering input data uncertainty, *Stoch. Env. Res. Risk A.*, 31, 1253–1269, 2017a.

De Risi, R., Goda, K., Yasuda, T., and Mori, N.: Is flow velocity important in tsunami empirical fragility modeling?, *Earth-Sci. Rev.*, 166, 64-82, 2017b.

Ebrahimian, H., and Jalayer, F: Selection of seismic intensity measures for prescribed limit states using alternative nonlinear dynamic analysis methods, *Earthq. Eng. Struct. D.*, 50(5), 1235-1250, 2021.

Eidsvig, U. M. K., Papathoma-Köhle, M., Du, J., Glade, T., and Vangelsten, B. V.: Quantification of model uncertainty in debris flow vulnerability assessment, *Eng. Geol.*, 181, 15-26, 2014.

Epanechnikov, V.A.: Nonparametric estimation of a multidimensional probability density. *Theor. Probab. Appl.*, 14, 153-1588, 1969.

720 Goff, J., and Dominey-Howes, D.: The 2009 South Pacific Tsunami, *Earth-Sci. Rev.*, 107(1–2), v-vii, 2011

Hastings, W. K.: Monte-Carlo sampling methods using Markov chains and their applications, *Biometrika*, 57(1), 97-109, 1970.

Jalayer, F., Beck, J., and Zareian, F.: Analyzing the sufficiency of alternative scalar and vector intensity measures of ground shaking based on information theory, *J Eng Mech*, 138(3), 307-316, 2012.

Jalayer, F., De Risi, R., and Manfredi, G. Bayesian Cloud Analysis: efficient structural fragility assessment using linear regression, *B. Earthq. Eng.*, 13(4), 1183-1203, 2015.

Jalayer, F., Ebrahimian, H., Miano, A., Manfredi, G., and Sezen, H.: Analytical fragility assessment using unscaled ground motion records, *Earthq. Eng. Struct. D.*, 46(15), 2639-2663, 2017.

730 Jalayer, F., and Ebrahimian H.: Seismic reliability assessment and the non-ergodicity in the modelling parameter uncertainties, *Earthq. Eng. Struct. D.*, 49(5),434-457, 2020.

Jalayer, F., Ebrahimian, H., and Miano, A.: Intensity-based demand and capacity factor design: a visual format for safety-checking, *Earthq. Spectra*, 36(4):1952-1975. 2020

Koshimura, S., Namegaya, Y., and Yanagisawa, H.: Tsunami Fragility—A New Measure to Identify 735 Tsunami Damage, *J. Disaster Research*, 4(6), 479-488, 2009a

Koshimura, S., Oie, T., Yanagisawa, H., and Imamura, F. (2009b). Developing fragility functions for tsunami damage estimation using numerical model and post-tsunami data from Banda Aceh, Indonesia, *Coas. Eng. J.*, 51(3), 243-273, 2009b.

Lahcene, E., Ioannou, I., Suppasri, A., Pakoksung, K., Paulik, R., Syamsidik, S., ..., and Imamura, F.:
Characteristics of building fragility curves for seismic and non-seismic tsunamis: case studies of the 2018 Sunda Strait, 2018 Sulawesi–Palu, and 2004 Indian Ocean tsunamis, *Nat. Hazards Earth Sys.*, 21(8), 2313-2344, 2021.

Miano, A., Jalayer, F., Forte, G., and Santo, A.: Empirical fragility assessment using conditional GMPE-based ground shaking fields: Application to damage data for 2016 Amatrice Earthquake, *B. Earthq. Eng.*, 18(15), 6629-6659, 2020.

Metropolis, N., Rosenbluth, A. W., Rosenbluth, M. N., Teller, A. H., and Teller E.: Equations of state calculations by fast computing machines, *J. Chem. Phys.* 21(6), 1087-1092, 1953.

Muto, M., and Beck, J. L.: Bayesian updating and model class selection for hysteretic structural models using stochastic simulation, *J. Vib. Control*, 14(1-2),7-34, 2008.





Reese, S., Bradley, B. A., Bind, J., Smart, G., Power, W., and Sturman, J.: Empirical building fragilities from observed damage in the 2009 South Pacific tsunami, *Earth-Sci. Rev.*, 107(1-2), 156-173, 2011.
Rosti, A., Del Gaudio, C., Rota, M., Ricci, P., Di Ludovico, M., Penna, A., and Verderame, G. M. Empirical fragility curves for Italian residential RC buildings. *B. Earthq. Eng.*, 19(8), 3165-3183, 2021.

Wing, O. E., Pinter, N., Bates, P. D., and Kousky, C.: New insights into US flood vulnerability revealed
from flood insurance big data, *Nat. Commun.*, 11(1), 1-10, 2020.